

REQUEST FOR PROPOSALS
Consulting Services for Developing a 2D Watershed-wide Model for Minnehaha Creek Watershed District

PART 1: BACKGROUND AND PROJECT OVERVIEW

General

The Minnehaha Creek Watershed District (MCWD or District) is seeking a qualified CONSULTANT team to support MCWD staff in building a two-dimensional (2D) coupled surface water-groundwater model for the Minnehaha Creek Watershed District. This work will directly support the District’s need to understand and predict the impacts of climate change, which represents the first phase of the District’s Climate Adaptation Framework.

The work described in this request for proposals (RFP) will build upon findings, processes, and datasets that were developed in three preceding, related District efforts: 1) the Pilot 2D Model Project (“Pilot Model”), 2) the 2D Model Data Standardization Project, and 3) the Model Input Refinement Project (“MIR Project”). More details on the Pilot Model Project can be found in Exhibit B.

The overarching aim of these projects has been to minimize technical risks inherent in constructing a large-scale, high-resolution hydrology and hydraulic (H&H) model. The District has tackled the challenge of devising a streamlined approach to process and assimilate unique stormwater infrastructure data from the cities and agencies within the District. Through these efforts, the District has developed an automated framework and a baseline Python script package, which together serve to process stormwater infrastructure datasets, address critical data gaps for the watershed-wide climate model, and unify each municipal/agency dataset within the watershed into a single, standardized geodatabase.

The final phase of the watershed-wide build project is to leverage learning from past projects to build a watershed-wide 2D H&H model that can help answer critical questions surrounding climate change adaptation for MCWD. This project is unique since it will require a strong partnership between the CONSULTANT and MCWD to build a watershed model that encompasses the Minnehaha Creek Watershed District (Exhibit A). At a high level, the roles for this project are as follows:

- 1) **Data Processing and Python Scripting:** The CONSULTANT will lead data review and data processing to produce baseline datasets that will be used in the watershed models.
- 2) **Upper Watershed Model Development:** MCWD staff will lead the development of the Upper Watershed model that will be reviewed by the CONSULTANT.
- 3) **Lower Watershed Model Development:** The CONSULTANT will lead the development of the Lower Watershed model.
- 4) **Data Collection:** The CONSULTANT will lead field data collection and MCWD staff will support field data collection where necessary.

This collaboration between the MCWD and CONSULTANT aims to leverage the distinct expertise and insights of both parties to the project to create a comprehensive and effective watershed model. Project success will require a strong background working with large-scale hydrologic and hydraulic ICPR4 models, stormwater infrastructure data, Python scripting, and the ability to work in a highly collaborative environment.

Project Background

Climate change is measurably changing the distribution, frequency and intensity of rainfall in Minnesota. Between 2013 and 2019, the MCWD experienced the wettest seven years ever recorded. Over the past 10 years, Minnesota has experienced both record flood conditions and statewide drought that has negatively impacted aquatic ecology, stressed stormwater infrastructure and cost billions in property damage. To successfully adapt to the increasingly volatile extremes in weather, MCWD and communities must be able to identify what landscape interventions are needed, where they are needed, and how much investment is needed. Recognizing these challenges, the MCWD has initiated an effort to develop a more robust understanding of how water moves through the Minnehaha Creek Watershed District and how climate change will impact the built and natural environment through the Climate Action Framework.

The first stage of the MCWD's Climate Action Framework is to "Understand and Predict" the impacts of climate change using new data sets and modeling to forecast scenarios, evaluate vulnerabilities, and make decisions about adaptation strategies. To accomplish this goal the District identified the need to develop a model that better represents 1) stormwater infrastructure, 2) surficial groundwater, and 3) storage. In 2023, the District was awarded a grant from Legislative Citizen Commission of Natural Resources (LCCMR) to fund the watershed-wide model development.

Project Objectives

The project has three primary objectives:

- 1) Processing datasets into a format that can be loaded into ICPR4 based on the model input requirements using repeatable processes developed in past projects (Exhibit B)
- 2) Identifying data gaps or data needs for the watershed-wide model that need to be filled through data collection for the final watershed-wide model to support the District's Climate Action Framework
- 3) Build, calibrate, and document the watershed-wide model to support climate planning based on data developed in previous projects or during this project

Project Elements

The project comprises seven key elements to achieve the project's objectives:

Task 1: Data Discovery and Data Review

The CONSULTANT will perform an initial review of the available datasets (Exhibit C and Supplemental Materials) that MCWD has obtained or developed to determine if additional data processing is necessary to build the ICPR4 model. This includes, but is not limited to, evaluating stormwater infrastructure data, stream channel cross-sections, USDA soils data, geologic information, and LiDAR. The goal is to identify critical issues or gaps that could impede a preliminary model build, focusing on ensuring data integrity and readiness for model development. The CONSULTANT will be tasked with reviewing the data on a watershed-wide scale.

Task 2: Model Build Approach

The CONSULTANT will conduct a series of workshops with District staff to ensure there is alignment on the critical decisions for the model build. These decisions include 2D mesh resolution, groundwater model approach, model calibration and validation approach, and other vital decision points for the model. The workshops aim to develop a shared understanding of the model-build approach and document the decisions in a technical memorandum that will aid in model-build consistency between the Upper Watershed Model (built by District staff) and the Lower Watershed Model (built by CONSULTANT).

Task 3: Non-pipe Stormwater Dataset

The CONSULTANT will be responsible for developing a comprehensive geodatabase of storage, such as ponds and BMPs, and hydraulic control structures critical for climate change modeling and planning across the entire watershed. They will develop a methodology to locate critical data gaps and criteria for filling gaps, which includes reviewing partner agency geospatial data and potentially collecting field data.

Task 4: Review and Refinement of Programmatic Data Processing Scripts

The CONSULTANT will review and refine the District's existing Python stormwater infrastructure scripts to integrate new datasets (layers, fields, and/or attributes) developed in Task 3. This includes developing new Python scripts to create a model-ready stormwater dataset, incorporating storage and hydraulic control structures.

Task 5: Preliminary Model Build

The preliminary model build will be based on currently available geographic data, aiming to identify key data gaps for further refinement. The CONSULTANT will lead the Lower Watershed Model and the MCWD will lead the Upper Watershed Model preliminary build. This task includes running initial model simulations, documenting data gaps or issues, and developing a prioritized list of data gaps.

Task 6: Data Collection and Data Processing

The focus of this task is to strategically address data gaps crucial to the hydrologic and hydraulic model's accuracy. The CONSULTANT, in partnership with the District, will prioritize data gaps based on their impact on model quality and budget considerations. A detailed plan will be outlined in a technical memorandum developed by the CONSULTANT, specifying prioritized data for collection, data collection methods, and documentation strategies. This task also includes the data collection effort itself, which will be led by the CONSULTANT with support from District staff if necessary.

The CONSULTANT will process the collected data using Python scripts developed in Task 4. These data will then be used to create a watershed-wide model-ready dataset. The focus is on integrating all processed datasets into the ICPR4 model, ensuring functional and cohesive model components.

Task 7: Model Build

The model build involves integrating processed datasets into the ICPR4 models, conducting initial test runs, and applying a calibration strategy. The Upper Watershed Model will be built by District staff, and the Lower Watershed Model will be built by the CONSULTANT. The CONSULTANT will adjust the Lower Watershed Model to accurately reflect the watershed's hydrologic behavior and document the calibration process. Model reports will be developed for the Upper Watershed Model by the District and Lower Watershed by the CONSULTANT. These reports will characterize critical aspects of the model

build, data collection, and calibration results based on standardized model documentation developed by the CONSULTANT.

PART 2: SCOPE OF SERVICES

The CONSULTANT will work in coordination with the District to complete tasks 1-7, below. The expected completion date for the scope of services is May 31, 2025. The District estimates a project budget of \$580,000-\$620,000.

For the purpose of the RFP, the scope of services is as follows:

Task 1: Data Discovery and Data Review

Task 1a. Data Discovery

The CONSULTANT will first conduct an initial review of the datasets available and needed to develop the ICPR4 model. The CONSULTANT will review each dataset to identify any critical issues or gaps with the datasets based on its professional judgment. The goal of this task is to ensure that there are no critical errors, however, it is only meant to identify the issues that will prevent a preliminary model build. The datasets MCWD curated prior to this project are summarized in Exhibit C.

Task 1b. Data Review Documentation

The CONSULTANT will create a technical memorandum that documents the current state of each dataset and potential gaps that would prevent a successful preliminary model build. The document will cover three elements including:

- Identifying critical issues or gaps within the datasets provided to develop the ICPR4 model
- A description of the issues or gaps within the datasets and why they need to be filled prior to model development
- The data sources, methods, or assumptions that will be used to fill the data gaps or erroneous data

Task 1 Deliverables:

- Data review assessment technical memorandum that includes:
 - o Specific gaps or erroneous data across the watershed-wide datasets
 - o A description of the issues or gaps within the datasets and why they need to be filled prior to model development
 - o The data sources, methods, or assumptions that will be used to fill the data gaps or erroneous data

Task 2: Model Build Approach

Task 2a. Model Build Approach

After reviewing the data, the CONSULTANT and the District will collaborate to arrange a series of four meetings to discuss the approach for building the model. The goal of this task is to develop a shared understanding between the CONSULTANT and District staff about critical model build decisions such as 2D surface mesh resolution, groundwater model approach, use and resolution of 1D components, calibration approach, and other critical model decisions that will set the direction for the model build.

The CONSULTANT will collect notes from each meeting, which will be used to document the agreed-upon model build approach through a technical memorandum.

Task 2 Deliverables:

- 4 in-person 1-hour (more or less) workshops
- Workshop summaries for each meeting describing the decisions made during the meetings
- A technical memorandum that summarizes the model approach developed by the CONSULTANT in collaboration with District staff

Task 3: Non-pipe Stormwater Dataset

Task 3a. Non-pipe Stormwater Dataset Strategy Development

One of the most important aspects of developing a watershed-scale 2D H&H model for climate change planning is characterizing the amount of storage that currently exists in ponds, lakes, wetlands, best management practices, and soil pore space. Characterizing the storage currently on the landscape will be an important first step to help the District, and its partner communities, understand where additional storage may be needed to adapt to increased precipitation. However, the District's initial review of the stormwater infrastructure datasets revealed that very limited data exist about storage and outlet control structures making it difficult to characterize the current extent of storage within MCWD. Therefore, the goal of this task is to:

- 1) Characterize a methodology for locating critical data to fill gaps in:
 - a. Wetland/pond/lake storage volumes
 - b. Wetland/pond/lake outlet control structures
- 2) Develop criteria for when and how assumptions will be relied on to fill data gaps for storage (ponds, wetlands and BMPs) and hydraulic control structures
- 3) Identify a strategy for filling critical data gaps in storage and hydraulic control structures through mining existing data sources or collecting data in the field.

Task 3b. Initial Non-Pipe Stormwater Data Mining and Data Collection

The findings from Tasks 2a and Task 3a will be used to guide data collection for storage volumes, hydraulic control structures, and other datasets identified as critical for model accuracy. This task will focus on reviewing data from existing geospatial data sources or field data collection of critical model features identified in task 3a to support the preliminary model build.

Task 3 Deliverables:

- A non-storm pipe data assessment technical memorandum that includes:
 - o A method for locating critical storage and hydraulic control structure data gaps
 - o Develop criteria for when assumptions will be made about storage and hydraulic control structures
 - o Articulate the strategy for filling critical data gaps in storage and hydraulic control structures
- A second technical memorandum summarizing the storage and hydraulic dataset produced for the preliminary model build
- Populated geodatabase, in an agreed-upon structure, that contains the recently developed storage data

Task 4: Review and Refinement of Programmatic Data Processing Scripts

Task 4a. Existing Stormwater Infrastructure Script Review

It's important that the CONSULTANT first develop an understanding of the overarching framework and the two associated script packages ("Raw to MGIS" and "MGIS to Model Ready") that were developed in previous projects to (1) convert each municipality's infrastructure dataset into the Metro GIS Stormwater Geodata Standard (MGIS) and (2) address topology issues, gaps, erroneous values. District Staff will provide all the scripts and associated documentation to the CONSULTANT for its review to serve as the baseline for subsequent tasks.

Task 4b. Existing Stormwater Infrastructure Script Refinement

The CONSULTANT will focus on refining or creating Python scripts to develop a comprehensive stormwater infrastructure, storage, and hydraulic control structure dataset, aligning with model requirements developed in Task 2. This task is expected to be an iterative process, involving continuous spatial analysis to evaluate the impact of script modifications on data quality and to determine the need for incorporating additional script functions. Furthermore, the CONSULTANT will leverage other spatial datasets, like LiDAR, to effectively address and rectify dataset deficiencies. Throughout this process, it is crucial to distinctly mark any assumed values within the dataset, ensuring future modelers can easily differentiate between field collected and estimated data. The ultimate objective is to prepare a robust, model-ready dataset that will serve as a key component in the District's forthcoming climate planning model development.

Task 4c. Script Documentation

Once scripts are finalized in task 4b and a model-ready dataset has been generated, the CONSULTANT will thoroughly document each script that outlines the purpose, required setup and source datasets, and pseudo code descriptions. Existing documentation from past projects can be built upon (Exhibit B) or new documentation can be developed as part of this project. Documentation must clearly identify where gaps are being filled or assumptions are being made.

Task 4 Deliverables:

- Revised package of stormwater infrastructure scripts with associated documentation
- Geodatabase(s) of watershed-wide model-ready datasets based on the model approach Task 2

Task 5: Preliminary Model Build

Task 5a. Model Build

The CONSULTANT will build a preliminary 2D H&H ICPR4 integrated surface water-groundwater model for MCWD's Lower Watershed (Exhibit A) using the approach guidelines developed in Task 2 and datasets developed in Task 4. District staff will develop the 2D H&H ICPR4 model for the upper watershed (Exhibit A), which will be reviewed by the CONSULTANT. The focus of this task is to create a functional ICPR4 model that can be used in subsequent tasks to identify critical data gaps and is not intended to serve as the final model for the project.

Task 5b. Initial Model Run and Review of Model Issues.

One of the key insights from the 2022 Pilot Watershed Model Build (Exhibit B) was that geographic data gaps have the greatest impact on model quality and should be filled before manipulating model parameters for calibration. Therefore, the purpose of the preliminary model build is to create a working model version based on geographic data that are currently available or data that require little effort to obtain. The preliminary model will help the CONSULTANT and District identify key geographic data gaps based on model runs that will help refine the key data gaps.

The CONSULTANT will conduct event-based and continuous model runs to characterize and document three critical elements:

- Erroneous data that cause issues with model stability or poor model results
- Data gaps that cause issues within the model such as poor stability or model errors
- Data gaps that reduce the accuracy of the model based on measured streamflow, stream level, or groundwater level.

Task 5c. Document Data Gaps or Issues

The CONSULTANT will work collaboratively to document data gaps and issues that were identified in Task 5b. Once model runs are finalized in Task 5b the CONSULTANT will thoroughly document the identified data errors and gaps. Documentation must clearly identify:

- The location of the data gap or erroneous data
- The nature of the data gap or erroneous data
- The relative magnitude or importance of the data gap or erroneous data on model performance or accuracy
- A prioritized list of data gaps that need to be filled via further review of existing data sources or field data collection

Task 5 Deliverables:

- A ICPR4 model that has been developed based on datasets created in Task 4 and modeling guidelines in Task 2
- A technical memorandum for the Lower Watershed Model that summarizes:
 - o The location of the data gap or erroneous data
 - o The nature of the data gap or erroneous data
 - o The relative magnitude or importance of the data gap or erroneous data
 - o A prioritized list of data gaps that need to be filled via requesting data from partner agencies or field data collection
- A second technical memorandum that summarizes the CONSULTANT's review and recommendations of the Upper Watershed Model built by MCWD staff

Task 6: Data Collection and Processing Planning

Task 6a. Data Collection Prioritization

The extent of data gaps that must be filled is currently unknown, which makes providing a prescriptive data collection request difficult to define. However, the District understands that the model built for this project cannot be a comprehensive warehouse of all geographic data that pertain to hydrologic and hydraulic models. Therefore, the District will work collaboratively with the CONSULTANT to develop a

prioritized list of data gaps that have the greatest impact on model quality, which must balance the level of importance to the model relative to the budget estimated by the CONSULTANT. The CONSULTANT will provide a written plan in a technical memorandum that will serve as an agreed-upon approach for filling data gaps through data collection or further geospatial data requests for the upper and lower watershed.

The CONSULTANT will work with District Staff to develop several items prior to data collection including:

- 1) A list of data that will be prioritized for data collection or mined through other methods.
- 2) Data collection forms based on prioritized data to be collected that will serve as long term data collection documentation.
- 3) A data request plan for data sources the District hasn't obtained from partner agencies or other data sources.
- 4) A collection plan that outlines the data collection locations, approach for collecting or obtaining each data type, property access for field data collection type, collection methods, CONSULTANT or District role, and documentation approach.

Task 6b. Data Collection and Data Entry

The CONSULTANT will execute any further data collection and geospatial data mining in close collaboration with District staff. District staff will notify private property owners for access, as-needed, prior to field work based on the collection plan developed in Task 6a. The CONSULTANT will then commence field work and collect data outlined within the collection plan.

Following data collection, the CONSULTANT will process and organize the data collected within a geodatabase format. Data should utilize NAD 1983 UTM Zone 15N as the horizontal spatial reference and NAVD88 as the vertical coordinate system.

Task 6c. Data Processing

The CONSULTANT will use the refined Python scripts developed in Task 4 to process data collected in Task 6b. The output of this data processing step will be the model-ready data used for the watershed-wide model to support climate planning.

Task 6 Deliverables:

- Data field form templates
- A technical memorandum summarizing the data collection plan and data request plan
- Geodatabase(s) of watershed-wide model-ready datasets based on the model approach Task 2
- Metadata associated with field-collected data

Task 7: Model Build

Task 7a. Model Build

Task 7 represents a culmination of the previous work since it will integrate all processed datasets developed in Task 6 into the ICPR4 model. This integration is pivotal for the development of a cohesive and functional hydrologic and hydraulic model. In addition, the CONSULTANT will need to configure the model parameters and settings that were outlined in Task 2 (Model Build Approach) for the Lower Watershed Model. Initial test runs will be conducted to verify the basic functioning and integration of the model components. These test runs are essential for ensuring that all elements of the ICPR are working correctly and creating model output that can be used for calibration.

Task 7b. Model Calibration and Validation

In this phase, the focus is on applying the calibration strategy outlined in Task 2 to the model. This involves using water level, flow, and precipitation data collected by MCWD and its partners (Exhibit D), with the goal of adjusting the model to accurately reflect the watershed's hydrologic behavior. This step is critical for ensuring that the model's outputs are not only consistent with historical observations but also follow a methodical and scientifically sound calibration and validation process. Documenting the calibration process will include, but is not limited to, a description of final adjustments to the model parameters, the justification for model adjustments, and a comparison of calibrated model results relative to the uncalibrated model and measured data results.

Task 7c. Model Build Report

Task 7c entails the preparation of a detailed Model Build Report. This report is a comprehensive documentation of:

- Introduction and background
- A summary of the datasets utilized for the model
- Detailed documentation of the Python Scripts used to develop the model-ready datasets
- A summary of the preliminary model build and the prioritized list of data gaps
- Detailed documentation of data collection and data mining to serve as a record of datasets created during this project
- Assessment of the accuracy of each model based on calibration and validation results

Task 7 Deliverables:

- A calibrated ICPR4 model for the Lower Watershed of MCWD
- Standardized watershed modeling report outline that can be used for the Upper and Lower Watershed Model
- A technical memorandum that summarizes the CONSULTANT's review and recommendations of the Upper Watershed Model built by MCWD staff
- Draft modeling report for the Lower Watershed
- Final Draft of modeling report for the Lower Watershed

Project Area

The project area for this RFP process is the entire Minnehaha Creek Watershed District that stretches 178-square miles and includes all or part of 27 cities and two townships (cities/municipalities) in Hennepin and Carver counties (Exhibit A)

Project Team

Brian Beck (Primary Contact)
R&M Program Manager, MCWD
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952-471-8306

Alex Steele (Secondary Contact)
GIS Coordinator, MCWD
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PART 3: INSTRUCTION TO PROPOSERS

Submittal Requirements

Responses to the RFP should be submitted to Brian Beck via email (bbeck@minnehahacreek.org) no later than 4:00 pm on Wednesday January 24, 2024.

Please visit the RFP webpage located on the District's website:

<https://www.minnehahacreek.org/>

No page limit is imposed; however, respondents will be evaluated on clarity and conciseness. Each proposal should include the following items:

- Cover Letter – Please provide a primary point of contact through the transmission of a cover letter.
- Project understanding – Describe your understanding of the scope of work, the approach to be taken, and your vision for the project. Identify any additional information the District will need to supply or obtain to enhance your understanding of the project and to complete the work, and/or any issues you might anticipate in performing the work.
- Approach and methodology – Describe your detailed approach and methodology for completing the project, organized by task as outlined in Part 2: Scope of Services. For each task, provide a clear outline including your understanding of the task, proposed methodology, the team assigned, anticipated deliverables, and any supplemental tasks not described in the RFP. This structure aims to ensure clarity and facilitate the evaluation process.
- Critical Aspects and Questions: Address the following critical aspects in your proposal:
 - o Describe your approach to identifying and filling critical data gaps in the watershed-wide model.
 - o What areas of the model build do you anticipate being most challenging? What is your approach or strategy for ensuring a successful model build in the face of these challenges?
 - o What are the areas of ambiguity that may cause the greatest risk in this project and what are your strategies to mitigate that risk?
 - o How will your team engage with the District staff to ensure alignment on critical model build decisions, such as 2D surface mesh resolution, groundwater model approach, calibration approach, and other vital decision points for the model? Please detail your methodology for facilitating effective communication, decision-making processes, and ensuring consensus on these key aspects of the model build.
- Proposal Structure: Proposals should be organized by task as outlined in Part 2: Scope of Services. For each task, provide a specific outline including understanding of the task, proposed approach and methodology, team assigned to it, and anticipated deliverables. This structure will help ensure clarity and facilitate the evaluation process.
- Cost Proposals Guidance: The proposal should include a detailed spreadsheet showing budgets for each task and subtask. This should break down the costs for labor hours, materials, subcontracting (if applicable), and any other direct expenses. Clearly indicate any assumptions that impact the cost and how those assumptions will impact your project cost.
- Project Assumptions:
 - o The District expects that the CONSULTANT has an ICPR4 Expert license to build the watershed model due to the incorporation of 2D surface flow and the incorporation of groundwater into the project models.
 - o The District will provide remote access to a 64-core workstation for model runs or model development. It is not a requirement to build the model on the District's

workstation, however, the District does want to make the resource available if the selected CONSULTANT would benefit from using a workstation optimized for ICPR4.

- Milestones and Deliverables: Include a detailed schedule of milestones and deliverables for each task and subtask. Indicate key dates and dependencies to illustrate how your team will manage the project timeline effectively. This should align with the task based NTE budgets and provide a clear roadmap for project completion.
- Qualifications and experience – Provide an overview of the firm(s) and project team member’s qualifications. Include descriptions of projects undertaken by the firm(s) and team members that demonstrate a strong understanding of municipal stormwater datasets, large-scale watershed 2D H&H models, ICPR4 modeling software, collaborating with clients on complex projects, and knowledge of the MCWD geography. Speak to the team’s ability to deliver the project on time and on budget.
- References – Provide three recent references for your proposed firm or team, including names, addresses, and phone numbers, along with a description of the project and your role. References preferably pertain to work described in this project.
- District Resources – Include a list of resources, expectations, or requirements that the CONSULTANT expects from the District in order to complete the project as proposed.
- Subcontracting – If the CONSULTANT intends to subcontract, identify and describe the subcontractor, describe the intended scope and role of the subcontractor, identify the team members proposed from the firm, and provide the qualifications and experience information requested above for those team members.

Request for Proposal Timeline

A review committee led by the project manager, Brian Beck, along with other select District staff will evaluate proposals. The District staff team will host one information meeting to answer questions about the RFP. Interviews are anticipated as part of the selection process. Following a comprehensive review, the review committee will recommend a CONSULTANT to the MCWD Board of Managers.

The anticipated timeline for the proposal review process, which is subject to change, is as follows:

- **RFP issue date:** Tuesday, January 2, 2024
- **Informational Meeting:** Thursday, January 11, 2024
- **Deadline for receipt of proposals:** Wednesday, January 24, 2024 at 4:00pm
 - o **Expected dates for Interviews:** Monday, February 5, 2024 to Tuesday, February 6, 2024
- **Expected dates for follow-up questions, as needed:** Thursday, February 8, 2024 to Friday, February 9, 2024
- **Anticipated date for CONSULTANT selection:** February 22, 2024 (District Board of Managers meeting)

Proposer's Budget for the Project

The requested services under this RFP will be funded through awarded LCCMR grant funds. Services will be compensated on an hourly basis with a specified not-to-exceed for the entire project. The Contract Maximum, to be set after the determination of the scope of work, is the cap for contractual services including both professional fees and expenses.

Addenda/Clarifications

Any changes to this RFP will be made by the District through a written addendum. No verbal modification will be binding.

Contract

Issuance of this RFP and receipt of proposals do not commit the MCWD to entering into a contract. The MCWD reserves the right to postpone the proposal deadline for its own convenience, to accept or reject any or all proposals received in response to this RFP, to negotiate with other than the selected CONSULTANT should negotiations with the selected CONSULTANT be terminated, to negotiate with more than one CONSULTANT simultaneously, or to cancel all or part of this RFP.

Joint Offers

Where two or more proposers desire to submit a single proposal in response to this RFP, they should do so on a prime-subconsultant basis rather than as a joint venture. The MCWD intends to contract with a single firm and not with multiple firms doing business as a joint venture.

Proposal Evaluation Procedure

Methodology

- *Project Understanding*: Does the proposal make it clear that the CONSULTANT fully understands the scope, goals, and technical requirements of the project?
- *Completeness and Specificity*: How fully does the proposal explain what the CONSULTANT will do to develop the required deliverables?
- *Identification of Needs*: Does the proposal carefully consider what resources will be required to complete the tasks, including staff time, additional technical information, etc.?

Experience

- *Company Experience*: What other projects has the CONSULTANT performed that have developed, used and demonstrated the expertise and capacity required for the proposed work (evaluated via the proposer's submittal materials)?
- *Staff Experience*: What qualifications and work experience do the proposed staff members or sub-CONSULTANTS bring to the project?
- *Area Knowledge*: Does the company or any of the project team have specific knowledge about the geographic project area that would aid in the study?
- *Collaboration Experience*: Does the company have experience collaborating with clients to deliver on complex projects?

Cost

- *Fee*: The proposal must clearly outline the fees and costs to complete all aspects of this project. Include hourly rates for each project team member along with hours for each task and subtask. The final fee, payment structure and not-to-exceed price are subject to negotiation.

Contract

Enclosed with this RFP is the form of contract that CONSULTANT and MCWD will execute. The MCWD may agree to non-substantive document revisions, but CONSULTANT's proposal should be based on the contract form. The proposal should identify any terms of the form of contract that are unacceptable. The MCWD will negotiate a term where it can preserve the

substantive intent of the term but reserves the right to reject a proposal that is conditioned on a material alteration of the contract form.

The proposal also must identify any data or methods of the proposer that would be used in performing the work, and that the proposer considers to be instruments of service that should be excepted from the intellectual property terms of the contract form.

Payments will be based on hourly rates on certification of completion of identified tasks. The payment schedule can be negotiated and finalized through the contract after the selection of a CONSULTANT by MCWD.

Contact

Any questions should be directed to Brian Beck at 952-471-8306 or bbeck@minnehahacreek.org.

PART 4: DISCLOSURES

Non-Binding

The District reserves the right to accept or reject any or all responses, in part or in whole, and to waive any minor irregularities, as deemed in the District's best interests. In determining the most advantageous proposal, the District reserves the right to consider matters such as, but not limited to, consistency with the District's watershed management plan goals, and the quality and completeness of the CONSULTANT's completed projects similar to the proposed project.

This RFP does not obligate the respondent to enter into a contract with the District, nor does it obligate the District to enter into a relationship with any entity that responds, or limit the District's right to enter into a contract with any entity that does not respond, to this RFP. The District also reserves the right, in its sole discretion, to cancel this RFP at any time for any reason.

Each respondent is solely responsible for all costs that it incurs to respond to this RFP and, if selected, to engage in the process including, but not limited to, costs associated with preparing a response or participating in any interviews, presentations or negotiations related to this RFP.

Right to Modify, Suspend, and Waive

The District reserves the right to:

- Modify and/or suspend any or all elements of this RFP;
- Request additional information or clarification from any or all respondents;
- Allow one or more respondents to correct errors or omissions or otherwise alter or supplement a proposal;
- Waive any unintentional defects as to form or content of the RFP or any response submitted.

Any substantial change in a requirement of the RFP will be disseminated in writing to all parties that have given written notice to the District, by email to Brian Beck, of an interest in preparing a response.

Disclosure and Disclaimer

This RFP is for informational purposes only. Any action taken by the District in response to proposals made pursuant to this RFP, or in making any selection or failing or refusing to make any selection, is without liability or obligation on the part of the District or any of its officers, employees or advisors. This RFP is being provided by the District without any warranty or representation, expressed or implied, as to its content, accuracy or completeness. Any reliance on the information contained in this RFP, or on any communications with District officials, employees or advisors, is at the CONSULTANT's own risk. Prospective CONSULTANTS must rely exclusively on their own investigations, interpretations and analysis in connection with this matter. This RFP is made subject to correction of errors, omissions, or withdrawal without notice.

The District will handle proposals and related submittals in accordance with the Minnesota Data Practices Act, Minnesota Statutes §13.591, subdivision 3(b).

Exhibits

Exhibit A: Map of the Upper and Lower Watershed Boundary

Exhibit B: Minnehaha Creek Watershed District Pilot 2D Model Project

Exhibit C: List of curated datasets currently available for this project

Exhibit D: Map of monitoring locations

Exhibit E: LCCMR Grant Agreement

Exhibit F: Contract Template

Supplemental Materials

[https://mcwdistrict-my.sharepoint.com/:f:/g/personal/asteele_minnehahacreek_org/](https://mcwdistrict-my.sharepoint.com/:f:/g/personal/asteele_minnehahacreek_org/EkvjEupljgZAtIQ3yujB_5gB38WK-fak5Q7B_gJZ4-FziA?e=PVuWN7)

[EkvjEupljgZAtIQ3yujB_5gB38WK-fak5Q7B_gJZ4-FziA?e=PVuWN7](https://mcwdistrict-my.sharepoint.com/:f:/g/personal/asteele_minnehahacreek_org/EkvjEupljgZAtIQ3yujB_5gB38WK-fak5Q7B_gJZ4-FziA?e=PVuWN7)

Exhibit A: Map of the Upper and Lower Watershed Boundary

MINNEHAHA CREEK WATERSHED DISTRICT

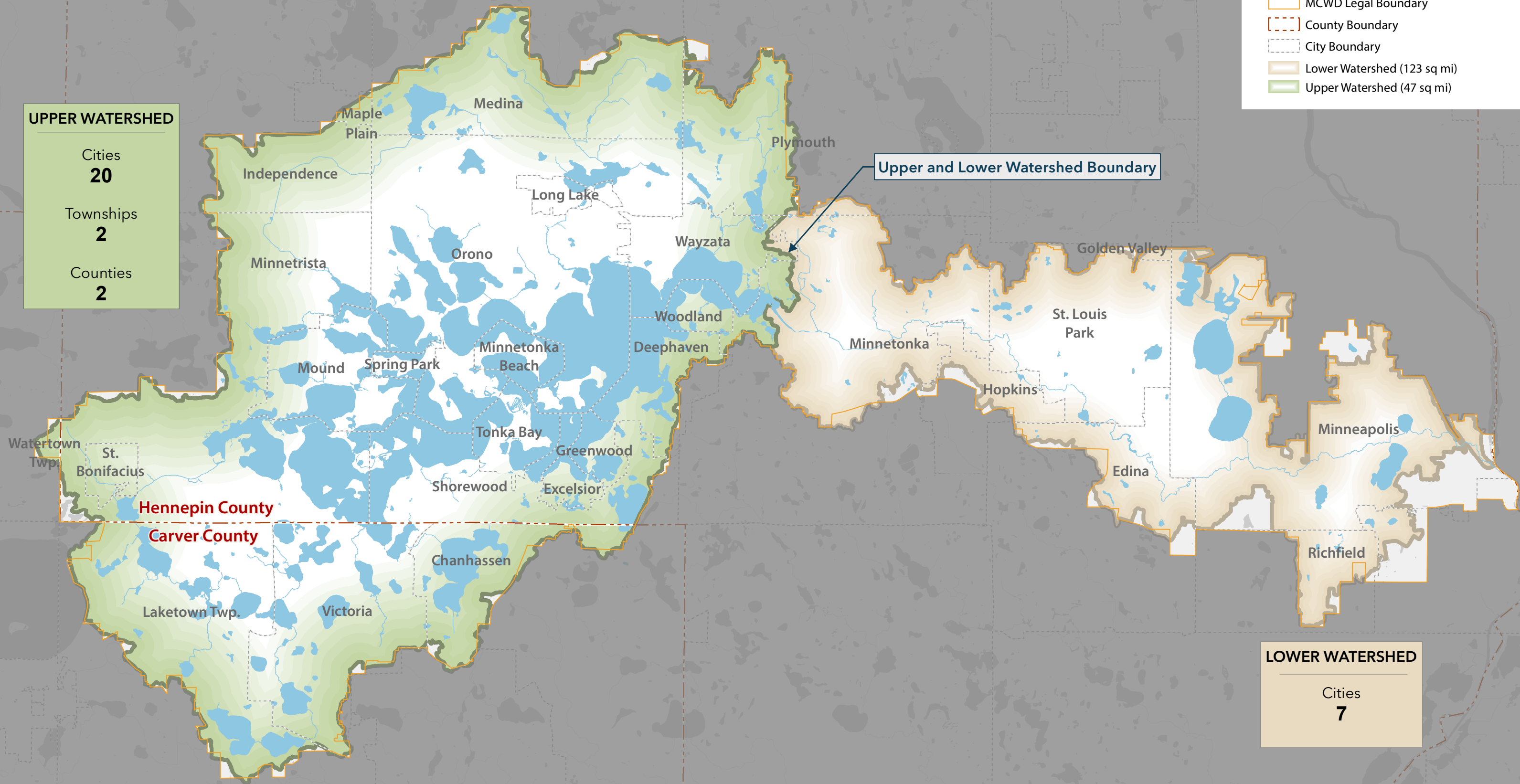
- LEGEND**
- MCWD Legal Boundary
 - County Boundary
 - City Boundary
 - Lower Watershed (123 sq mi)
 - Upper Watershed (47 sq mi)

UPPER WATERSHED

Cities
20

Townships
2

Counties
2



Upper and Lower Watershed Boundary

LOWER WATERSHED

Cities
7

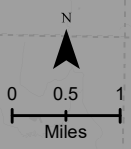


Exhibit B: Minnehaha Creek Watershed District Pilot 2D Model Project

2D Pilot Model Build

Project Summary Report

August 11, 2023

Prepared for:



MINNEHAHA CREEK
WATERSHED DISTRICT
QUALITY OF WATER, QUALITY OF LIFE

Prepared by:

Kimley»»Horn

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EXECUTIVE SUMMARY

The Minnehaha Creek Watershed District (District or MCWD) identified the need for a watershed-scale, two-dimensional (2D) modeling tool to support its goal of characterizing present and future climate change impacts and evaluating adaptation strategies. The District first pursued a pilot model project to manage technical risks and guide model platform selection. The pilot aimed to:

- (1) Develop a repeatable automated process for integrating diverse model input datasets, necessary for incorporating stormwater infrastructure datasets from the various communities within the District; and
- (2) Assess the capabilities and alignment of two modeling platforms (ICM and ICPR4) with the District's needs for climate planning.

In the course of the project, a semi-automated data processing framework (framework) for model inputs was developed to streamline the model build process, successfully accomplishing a core project objective. This work included establishing a method that effectively addressed the challenge of standardizing unique stormwater infrastructure datasets. The framework output format is largely model-agnostic, meaning the work undertaken during the pilot will serve the District beyond the climate model. Refinement to the automated processing steps will need to occur next to support its use at a watershed scale.

Among the two platforms evaluated, ICPR emerged as a model that is particularly aligned with the District's needs for climate planning. While both models were found capable to characterize flood risk and evaluate project and policy adaptation strategies, ICPR's standout feature is its ability to model surface water and groundwater in one integrated platform, offering the District an opportunity to grow its understanding in surface water-groundwater interactions and characterize its influence on flood risk. However, this benefit comes with its challenges; ICPR is inherently more complex, making it more difficult and time-consuming to build, calibrate, and operate.

ICM's strengths are ICPR's weaknesses. ICM has a clear advantage when it comes to operational functionality, keeping the level of effort to build and run the model low, and the project team experienced minimal technical challenges compared to its counterpart. However, it falls short in some critical areas. Notably, its inability to model 2D groundwater presents a limitation for climate planning.

Insights gained from this project about each model's strengths and weaknesses will guide the District's model platform selection for climate planning. Additionally, the pilot model served as a valuable learning tool for the District. It shed light on critical decision points, highlighted the importance of specific datasets that guided data collection efforts, and provided foresight to potential challenges so the District can proactively plan to mitigate difficulties during watershed-wide construction.

This report highlights significant work areas, key learnings, and scaling considerations, all drawn from technical work and documentation across six project-related memorandums or reports:

- Data Discovery Memorandum
- Automated Script Design Report
- Model Build Technical Report
- Model Calibration Report
- Scenario Modeling Report
- Evaluation Framework Memorandum

1.0 INTRODUCTION

Over the last decade, Minnehaha Creek Watershed District (MCWD or District) has experienced the wettest seven years on record, followed by periods of severe and extreme drought. These changes in precipitation intensities and swings from wet periods to drought conditions appear to be here to stay and continuing to stress our natural and built systems, highlighting the importance of integrated land-use water resource planning. To help guide the District towards climate adaptation, the District developed its Climate Action Framework (CAF). The CAF lays out a pathway for the District to identify and implement high-impact solutions in collaboration with its partners. This pathway has three key pillars:

Pillar 1: Understand and Predict - Utilize and expand technical capabilities in data collection, analysis, and tools to understand and predict the impacts of climate change at a watershed scale.

Pillar 2: Convene and Plan - Bring together local, regional, and state agencies to build consensus around the issues, align goals, form partnerships, leverage resources, and develop a coordinated strategy.

Pillar 3: Implement, Measure, and Adapt - Coordinate implementation actions with partners to make measurable progress towards goals. Implementation actions may include projects, policy changes, and operational improvements.

Pillar 1 is centered around the need to first understand and characterize how and where the watershed is being impacted today, and in the future to facilitate the evaluation and prioritization of climate adaptation strategies. A key tool identified within pillar 1 is the development of a watershed-wide 2D model, designed to provide a high-resolution understanding of how water moves through the regional hydrologic system, location and frequency of flood conditions, and the range of possible impacts in the future. Furthermore, the District looks to use this tool to begin assessing regional strategies to adapt to climate change, which could include both capital projects and adapted policies and regulation. The importance of this modeling tool, paired with the inherent technical and relational risks associated with a large-scale high-resolution model, led the District to pursue this project, the 2D Pilot Model.

1.1 Background

The District relies on multiple models, all constructed and designed to serve unique needs. The current watershed-wide Hydrology and Hydraulic model (XP-SWMM) was developed in 2003 and was considered state of the art for its time. It was designed to characterize the total volume and pollutant runoff from the landscape and understand the impact of runoff on receiving water bodies. Over the years, this model has served as the District's day-to-day operational model and has been used to estimate pollutant loading, conduct creek flood forecasting, support floodplain management, aid permitting assessments, and provide boundary conditions to District partners. These uses are still needed and continue to be met today by XPSWMM, however, it was deemed that this model is not granular enough to also understand and predict the impacts of climate change on a localized scale or to evaluate adaptation strategies. This limitation stems from the model being one-dimensional, low resolution, and implausible to keep updated, which are common limitations for a model of its time.

Over the past 20 years, major advancements in computing power along with the availability of high-resolution digitized datasets, make building and operating a large-scale 2D model possible. Taking advantage of these technological advances provides the District with the opportunity to not only quantify runoff volumes, but also represent how water moves across the landscape via runoff, storm pipes, wetlands, best management practices, and surficial groundwater. Over the last decade, the District has experienced firsthand the important role surficial groundwater can play during periods of drought and also its influence on flooding during extended wet periods.

The District was particularly interested in understanding if any of the available modeling platforms could accurately characterize surface water-groundwater interactions and groundwater movement. Understanding the relative extent of groundwater contributions during wet conditions or during periods of drought could provide valuable insight to the District related to the effectiveness of stormwater management practices such as infiltration and irrigation reuse systems and provide a greater understanding of volume management across the entire system.

Understanding the needs and use of the modeling tool is critical to selecting the right platform. With the understanding that the model would be used to holistically understand volume management across the 178 square miles, while also characterizing localized flooding issues, District staff worked with internal workgroups, consultants, and external partners to evaluate and identify modeling software that would best serve the District’s needs. Key components identified during the initial evaluation were the ability to model overland flow (2D surface features); include detailed stormwater pipe networks (integrated 1D-2D model); and integrate a realistic representation of the water table (integrated surface-water groundwater model).

District staff ultimately decided that the modeling software that met the most criteria were Infoworks ICM and ICPR4 (ICM and ICPR). The District chose to pursue this 2D Pilot Model Build study to further evaluate the two selected model platforms to address the technical challenge of incorporating numerous high-resolution datasets into a modeling tool, specifically the challenge of integrating the unique stormwater datasets from the 29 different communities within the District. The District intentionally selected the two geographically distinct pilot subwatershed areas show by the dots in Figure 1 to evaluate the models in a fully developed urban area in Edina and an undeveloped rural area in Carver County.

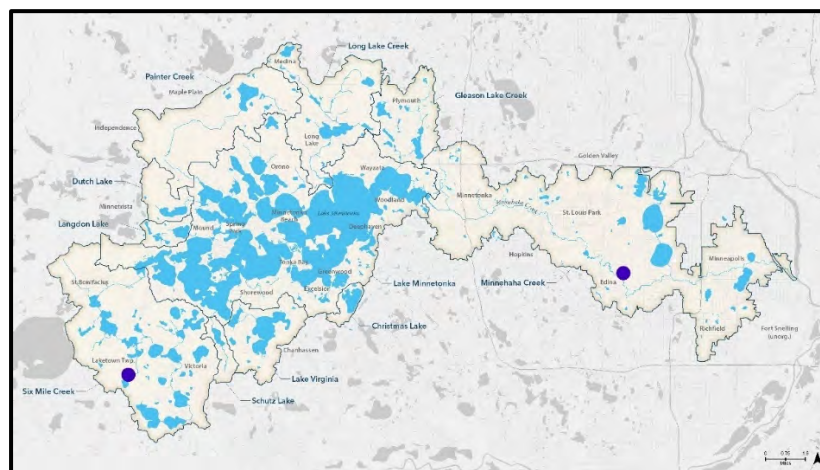


Figure 1. Pilot Subwatershed Locations in the Minnehaha Creek Watershed

1.2 Pilot Model Project Overview

The pilot model objectives were to establish an automated workflow for processing model inputs, understand the benefits and drawbacks of the two tested software suites, and inform which software is best suited for climate adaptation planning. To accomplish this, several tasks were outlined, which can be grouped into four key project phases:

1. **Data Development:** This phase of the project defines what datasets are needed, evaluates the quality of the available datasets, and establishes an automated framework with supporting scripts that create model ready datasets.
2. **Model Build and Calibration:** This phase of the project characterizes the process for constructing and calibrating each model while highlighting challenges and key differences between ICM and ICPR.
3. **Model Scenario Runs:** This phase of the project assesses the ability of each model to run a variety of scenario types and highlight functionality and operational differences.
4. **Model Evaluation and Comparison:** This closing phase of the project looks to summarize all the learnings and observations from previous task areas and describe each model's strengths and weaknesses, as they relate to the District's upcoming climate planning modeling needs.

Throughout this process, the project team identified and documented key observations related to the scalability of each software to a watershed-wide build and better understand some of the unique features of each model. These key observations are highlighted throughout this report.

Additional detail on each technical task area of the pilot model build project is provided in the following Technical Memorandums and Reports incorporated in full as Appendices A through F:

- A. Data Discovery Memorandum
- B. Automated Script Design Report
- C. Model Build Technical Report
- D. Model Calibration Report
- E. Scenario Modeling Report
- F. Evaluation Framework Memorandum

2.0 DATA DEVELOPMENT

A combined 1D-2D surface water model draws on a variety of spatial datasets, each collected and maintained from different agency/entity with its own unique schema. The ability to efficiently build and reasonably maintain a high-resolution watershed-wide model hinges on the idea that repeatable, largely automated, workflows can be developed to process and integrate the stormsewer datasets of the 29 communities within the District. A key objective of the pilot model build was to establish scalable automated workflows for processing required model data inputs, such as the stormsewer datasets.

The data development phase encompasses the work required to deliver on that project objective and included two essential steps: data discovery and script development. Refer to the Data Discovery Memorandum in Appendix A and the Automated Script Design Report in Appendix B for more information.

2.1 Data Discovery

The primary objectives of the data discovery phase of the pilot study were to collect and review the data types available through public sources and the direct project partners and to understand input needs of each model. Both aspects were intended to help identify gaps within available datasets that may impact the pilot model build process and guide scripting efforts.

District staff and project partners provided datasets in multiple formats and data types for use in the model development process. These data sets, often containing substantially more data than needed to build a working model, were reviewed and refined to the data needed for the model as described in the following section.

2.1.1 REQUIREMENTS FOR MODEL BUILD

Development of a combined 1D/2D model requires data that can be divided into two categories, model base data and hydraulic network features. For both categories, the data is needed as a direct input model parameter or is needed to generate (i.e., calculate) a required model parameter. Table 1 summarizes the major data input categories and the format that each model requires for import. Items in *italicized text* indicate that the input data needed or preferred is significantly different between the two models.

Based on a review of the available data, there was also a number of datasets that were not used in the automated pilot model build including:

- Pipe inspection and maintenance records and corresponding dates;
- Infrastructure ownership information including date constructed; and
- Detailed information for special drainage structures, including notes and descriptions of multi-stage outlet control devices.

While these data may have been noted in a municipal dataset, the process to collect as-built drawings and create the special structure manually in the pilot models was beyond the scope of the project. There may be a small number (less than 5) of the special structures that are critical hydraulic control devices throughout the watershed that should be considered further for the full watershed-wide model build

out. Critical hydraulic control devices would be those structures that could significantly change the system response for larger events.

Table 1. Shared vs. Specific Data Inputs and Sources

| Data Input | ICM | ICPR4 | Source(s) |
|----------------------------|--|--|--|
| Coordinate System | NAD 1983 State Plane Minnesota South FIPS 2203 (US Feet) | NAD 1983 State Plane Minnesota South FIPS 2203 (US Feet) | NA |
| DEM/Ground Surface | <i>Elevation Point Data</i> | <i>Raster Data</i> | MnDNR (MnTOPO, 2011) |
| Soils Data | Soil Zones | Soil Zones | NRCS (2003) |
| Land use/Land Cover | Zones | Zones | Metropolitan Council (2016, 2020) |
| Lookup Tables | CN, Impervious, Manning’s n, Inlet Head Discharge Curves | CN, Impervious, Manning’s n, Inlet Head Discharge Curves | Created from various hydrologic references |
| Rainfall | Depth and Distribution | Depth and Distribution | NOAA precipitation data server, local weather stations for 2021 and 2022 event |
| Nodes | Subsurface Junctions (manholes), 2D Interface Nodes (inlets) | Subsurface Junctions (manholes), 2D Interface Nodes (inlets) | Edina |
| Links | Pipes, <i>1D/2D Links (Open Channel Crossings)</i> | Pipes, <i>Rating Curves, Percolation</i> | Edina, MnDOT (Turbid), Carver County (Turbid) |
| 1D/2D Interface Elements | <i>Storage Area Volume Controls</i> | <i>Pond Control Volumes, Channel Control Volumes</i> | User created |
| 2D Overland Domain | Grid (Triangular) | Grid (Triangular) | Created from DEM |
| 2D Terrain Characteristics | Building footprints, Breaklines, Breakpoints | Building footprints, Breaklines, Breakpoints | DEM ground surface User created breaks |
| Groundwater | <i>Infiltration Parameters</i> | <i>2D Domain</i> | USGS Geologic Atlas, MCWD monitoring data |
| Boundary Conditions | Overland | Overland, <i>Groundwater</i> | User created from various reference sources |

2.1.2 GAP ASSESSMENT

During the data development process, a number of anticipated data gaps and errors were observed. Data gaps were categorized as minor gaps if the data could be corrected or assumed and still support a base model build or as major gaps if the data were critical to building the model or supporting a specific function of a model. Minor gaps were expected to be present and are common within pipe datasets where data entries may have entered the wrong pipe size or have no data for a given pipe segment at all. Beyond the minor and major gaps identified below and specifically related to building a working model, additional data needs were identified through the pilot study that relate more to the quality of the model and ability to achieve a desired level of calibration quality. These data needs are discussed further in the calibration section of this report.

Minor Gaps

The most common minor gaps consisted of incomplete or missing data for pipes and structures, such as pipe size or pipe invert and rim elevations, leaving the District with two possible solutions:

- Solution A: Utilize a process to calculate or assign an assumed value to the incomplete or missing data based on Link data.
- Solution B: Field survey of incomplete or missing data.

For the pilot study, Solution A was used to fill the gap for most parameters. When considering the watershed-wide build, Solution B may be a more effective approach in certain areas to ensure accuracy, especially when considering the potential differences in model response due to having an assumed pipe size that is significantly different than what is actually in place.

Major Gaps

Overall, there were no major gaps in the available data needed to successfully build a functioning model in both ICM and ICPR formats within the two pilot geographies when considering the District’s primary model use goals for evaluating a range of hydrologic and hydraulic responses in the pilot subwatersheds.

While there were no major data gaps related to building a functioning model, the pilot study identified a few critical data needs (or gaps) when considering what body of data is needed to build a more detailed model to support climate planning. One opportunity to improve model quality is to collect channel survey data to develop more accurate channel geometry data in critical stream channel areas. This survey data would allow the channel to be created in a 1D format or burned into the available lidar to at the below water surfaces and better represent the actual channel sections.

2.2 Data Processing (Script Design)

The overall objective of the data processing effort, or Automated Script Design, was to develop a set of scripts to support a more repeatable and automated data development process that would produce model-build ready datasets from the available raw datasets.

2.2.1 DATA WORKFLOW

One of the greatest technical challenges relating to developing a consistent dataset of model inputs is the range of raw stormwater datasets that exist in different schemas within the 27 cities, two townships and two counties. A set of automated scripts was envisioned from the outset that would process raw data to model-build ready data. Early in the project, the team recognized the need to rely on a standardized geodatabase as a central component to the overarching framework. The MetroGIS draft stormwater geodata transfer standard (MGIS) was selected since it has been vetted by industry experts and includes thorough documentation.

As the script design process advanced beyond the initial concepts, it became clear that there would need to be multiple scripts at particular stages of the processing pipeline. As illustrated in Figure 2, the dark blue boxes are where the scripting tools are applied.

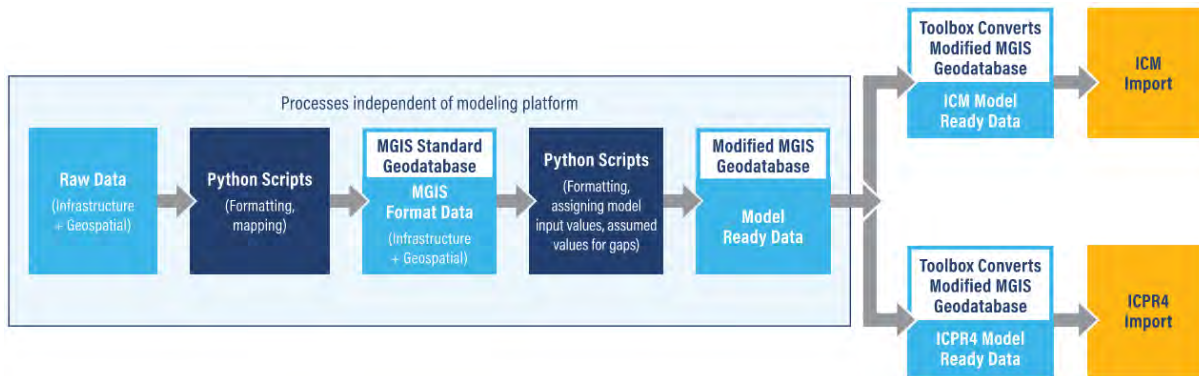


Figure 2. Datasets and Workflow Process to Produce Model Ready Data

First, the raw datasets are processed through a set of Python scripts to format the data and translate the datasets into the MGIS format. This set of scripts relies on mapping tables that are specific to each pilot stormwater dataset and note how each dataset’s fields correlate to the MGIS fields. Standardizing each dataset into an established standard schema allows for reduced user input effort and errors and overall reduced complexity throughout the remaining processes to develop the model-ready datasets. It is important to note that within the baseline MGIS database, no new or assumed data is being added.

The second step is to process the MGIS format data through another set of Python scripts to further format the data into model ready data, including (1) calculating derived model input parameters from the raw data and (2) populate values to fill gaps and/or correct abnormalities. The resulting data is referred to as the Modified MGIS Geodatabase. There are three main ways the package of scripts looks to fill gaps and/or adjust values within the stormwater infrastructure datasets:

- Reference spatial datasets to correct elevation issues
- Reference downstream/upstream pipe segments to populate gaps
- Utilize engineering best practices/standards to fill remaining gaps

This decision-tree workflow allows for multiple pathways to fill a gap, which looks to first take advantage of known values before utilizing an assumption. An important feature to this package is that all adjusted values are annotated and labeled as an adjusted value to support the modeler’s understanding of the source data. From that point, each model has its own required format for data import, with ICM the final toolbox function converts the data into a shapefile to be model import ready, while for ICPR, the final toolbox function converts the data to a GWIS Geodatabase format to be model import ready.

In total, a combined 20 scripts were developed to process the raw data to MGIS and the MGIS data to model-ready input format. An important benefit, by design, is that the bulk of the scripted processes are model agnostic. That is, the work completed during this project to establish repeatable processes for developing model-ready datasets will serve the District beyond the upcoming build and regardless of platform.

2.3 Learnings

Throughout the data development and script design process a number of important lessons were learned that will be beneficial for the District and its partners to understand when moving into the data development process for the initial watershed-wide climate model build and for consideration of maintaining a watershed-wide stormwater database long-term. The District's intent is for the data sharing process to become more efficient in the years ahead and become repeatable and reliable over the long-term. Key learnings include the following:

1. Even with automated processes designed to correct and fill data gaps, manual corrections within the software were still needed during the pilot model build. It is implausible to script for every potential data entry mistake or anomaly that may be encountered, meaning the data will always require some level of spatial and/or model-specific analysis to identify erroneous values that could impact model performance and results.
2. Data mapping tables created for the pilot stormwater datasets are unique to each municipality/agency and any changes in how a municipality/agency maintains their data will impact how the standard data mapping fields apply within the scripts and adjustments will need to be made. The overall process hinges on the consistent attribute naming of transformed data to develop model parameters. The raw attribute data mapping is an important step to successfully convert data from raw to MGIS. This step acts to ensure data types, formats, and naming conventions are documented so that the scripting process and data conversion steps are easily repeatable.

2.4 Scaling Considerations

Considerations related to scaling the data discover and processing efforts to the watershed wide model build effort include the following:

1. Individual data mapping tables will be needed for each local partner's dataset to bring into the standard. This includes obtaining a summary of basis and intent of each data field, ideally from the database owner that has the most recent knowledge of each dataset's schema. For example, the Edina dataset was provided with a summary document that defined which data fields were the most applicable where there were more than one field with what appeared to be the same data.
2. Since the scripts were created to account for issues observed in the pilot model datasets, the package of scripts will need to be refined to account for a wider range of issues and values that are expected to be encountered as the District reviews stormwater datasets watershed-wide.
3. The District and its partners will need to consider how the manual adjustments made within the modeling software (outside the automated script processing steps) will get incorporated back into the local partner's dataset. The intent would be that these modified values could and would be flagged such that the next iteration of data sharing would already have the previously manually adjusted value in the model ready dataset. By defining this process more clearly, it should decrease the level of effort needed during future model updates and could provide an added value for the corresponding city/agencies.

3.0 MODEL BUILD AND CALIBRATION

Following the data development phase of the pilot study, the data was imported into both model formats to initiate the model build process. The overall objective of this phase of the project was to build functioning existing conditions 1D-2D models in both model platforms, each in two distinct geographic areas, calibrate the models to a defined set of tolerances and document the challenges encountered during the model build process and the observed differences identified between the two platforms.

3.1 Model Build

The major elements of the build process for both models involved importing the pre-processed data, building the 2D surface mesh, executing the model under existing condition simulations, and then completing an iterative process to resolve remaining model functional errors, if any. Figure 3 illustrates the major steps in the model build process for both models, with the major difference being the addition of the groundwater mesh for ICPR. This additional step in ICPR seems to impede the user’s ability to leverage automated mesh generation tools, resulting in heightened complexity and increased effort required to achieve a finalized model. In addition, the build scenarios box in the ICPR process includes the final step in the parameterization of the 2D surfaces for use within the simulation run in ICPR. Building the 2D surface mesh and 2D groundwater mesh is required to be finalized within the scenario building process. ICM performs this task automatically during the initial simulation initialization when a simulation is run.

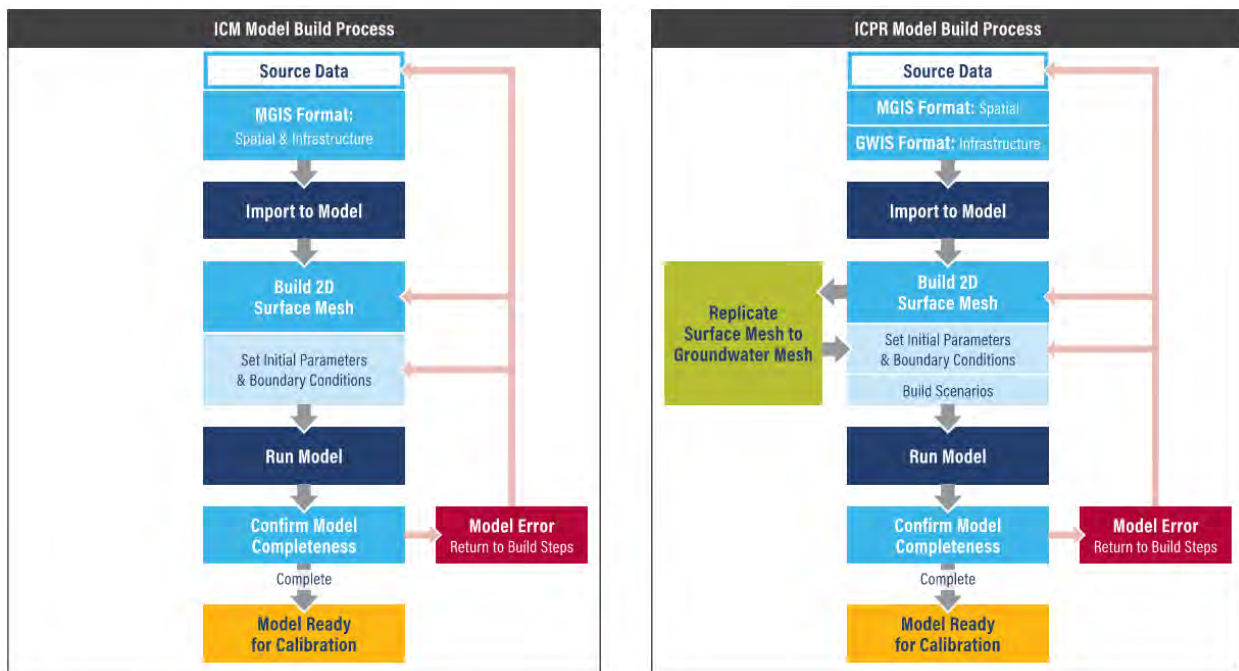


Figure 3. Comparison of ICM and ICPR Model Build Processes

Additional detail of the build process is provided in the Model Build Report. The following section summarize the model build process for ICM and ICPR, respectively.

3.1.1 ICM

Data needed to create the ICM model can largely be imported using the Open Data Import Centre (ODIC). The ODIC accepts standard data types and allows the user to overwrite, append, and merge new datasets with previously imported datasets. The new datasets (including data attributes) can be assigned flags during the import process for future reference. Data flagging is beneficial to the modeler when updates to a model occur, during the calibration process and during model review processes.

Spatial datasets such as the hydrologic parameters and 2D surfaces typically require two data types: the delineation or boundary of the data; and the parameters that describe the impact of the dataset. These parameters include roughness zone (delineation), the boundary of landuse or ground cover types and the roughness definition contains the roughness parameter for each landuse/land cover. This two-step process for spatial data allows for large-scale changes to be made to the parameters or delineations with relative ease. ICM can use terrain-sensitive meshing feature when developing the 2D mesh which allows the modeler to specify the rate at which new 2D elements are created during the mesh generation process. To perform a simulation run, a run object must be created that references the scenario, simulation time, simulation run parameters, and other objects. The other objects can include rainfall files, initial conditions, inflow/outflow conditions as well as specifying that multiple scenarios be run simultaneously.

ICM Build Challenges:

- In areas with significant elevation changes, multiple mesh elements are required to accurately simulate the change in topography. Using the terrain-sensitive mesh generation technique, allows the user to specify a larger range of acceptable element sizes. Determining the correct size of mesh will be important to developing results at a scale that is functional for the full build.
- When GIS data has duplicate pipes (i.e., pipes that start and end at the same exact points), neither pipe was imported into ICM. The missing pipe error occurred once during the Edina subwatershed model build and was found by using the GIS Layer Manager to bring in the pipe data as a background file and verifying that all pipes were imported. This is a quick process but a necessary one to verify the automation.
- The ICM model encountered some model instability issues during the initial simulations that caused the model to crash within the initial 5 percent of the run time. The instabilities were most commonly due to 1D-2D connection elevation variances at the end of pipe runs. The most common causes for this issue include:
 1. A pipe discharges below what the model sees as the water level of a creek or pond. This can be due to LiDAR data not having a surface below the normal water level. This can be addressed by including storage below the normal water level in either a 1D or 2D format for wet features.
 2. Pipes that do not have raw invert data are automatically assigned inverts with a DEM offset from the user input parameter. This new downstream invert is then set below the DEM and will need to be adjusted during the model-build and verification process.

3.1.2 ICPR

ICPR uses the GWIS import process for the creation of 1D hydraulic data within a scenario. The spatial datasets are imported through the corresponding surface and map layer manager. Multiple surfaces and map layers can be imported at once and referenced to the corresponding scenario. Similar to ICM, the delineation and parameters for spatial layers live in separate locations can be adjusted separately. The

overland flow region manager in ICPR creates and parameterizes the 2D mesh elements for surface flow. The overland flow manager allows for terrain-sensitive meshing but only when surface flow is going to be analyzed. The overland flow manager uses the specified roughness and infiltration spatial layers to parameterize the mesh.

ICPR can model two-dimensional groundwater flow using a triangular mesh like the two-dimensional overland flow mesh. The groundwater mesh and the surface water mesh can interact with each other through recharge, infiltration, seepage, and leakage. The pilot model build incorporated recharge, infiltration and seepage. The groundwater region should be setup by copying the previously created data from the overland region. Once the overland flow and groundwater regions are created and built within the respective manager tool, the scenario must be built. The scenario finalizes the various components (i.e., 1D, 2D overland, 2D groundwater) into a single file for use during the simulation run. The simulation manager can be used to specify rainfall, run times, and other simulation parameters.

Model Build Challenges:

- ICPR determines connectivity in the 1D network based on data associated with the pipe so when data entry errors are present or name data is missing, ICPR will not be able to connect pipes to nodes regardless of spatial relationship. When these errors occur, the modeler must manually define the names of upstream and downstream connections for each pipe. This occurred a couple of dozen times during the pilot build for the Edina subwatershed. It appears that the occurrences were generally located at or near where newer construction had taken place.
- When GIS data has duplicate pipes (i.e., pipes that start and end at the same exact points), both pipes are imported into ICPR. This causes a fatal error when the model tries to run. This error can be solved by deleting the duplicate pipes from ICPR.
- The ICPR model encountered instability when boundary conditions were not applied directly to the model boundary, when nodes were left in the model that did not attach to any pipes, and when inlets are placed very close to stage boundary conditions. Fixing the boundary condition to conform exactly to the model boundary and removing pipe inlets close to the downstream stage boundary condition stopped errors from crashing the model.
- The ICPR model encountered model instability issues when starting elevations within nodes were not properly assigned. The default water surface elevation is set to the rim of the structure/node at the beginning of the simulation. This produces high velocities and flow rates within the 1D pipe network. The starting water surface elevation at a node must be changed to correspond with the lowest pipe invert at the structure to eliminate this instability.
- A fully functioning overland flow model must be created prior to creation of the groundwater model. All edits to the overland flow model must be transferred into the groundwater model, including breakpoints, breaklines, and refinement areas.
- The size of the individual groundwater meshes begins to reach a practical limit around 12,000 groundwater cells based on the guidance provided by the model creator. During the pilot build, a mesh just below this practical limit was created and no issues were encountered. Multiple groundwater regions can be used within a single model but the interface line between groundwater regions must be wet (e.g., a lake, pond or creek) to allow flow across the boundary.
- The 2D flow methodology of ICPR only allows flow along the triangle faces of the overland flow region. This was found to significantly impact model run times and stability when the faces were

not aligned with the direction of flow. Aligning the triangle faces with principal flow paths is accomplished through the creation of breaklines. When considering a watershed wide build, the recommended approach is to create breaklines in GIS to allow for multiple users to create shapefiles that can be joined into a single large file for incorporation within ICPR. All breaklines created within the overland flow region should be transferred (copied) to the associated groundwater flow region for the area.

While ICPR poses some additional challenges with the model build process compared to ICM, several relate to the added complexity of the groundwater function that only ICPR offers. ICM and ICPR both need to preprocess the mesh to parameterize each 2D mesh element with infiltration and roughness values prior to use during model runs. While not a significant model build limitation, the models do require a different level of effort to preprocess the respective meshes. ICM completed the preprocessing in one to five minutes for two scenarios in the scenario evaluation process. ICPR completed the preprocessing for the same scenarios in under 30 minutes for the low-resolution scenario and between two and five hours for the high-resolution scenario.

3.2 Model Calibration

It is important for the upcoming watershed-wide climate model build to have a high level of accuracy in the model's ability to characterize the current system, so there is trust in the results projected for the future. Gaining confidence is significantly influenced by having a model that can be adjusted to match a known watershed response, or better yet, multiple known responses.

The Model Calibration Report provides the details of the calibration process for both models and how the calibration process may be improved during the watershed wide model build. Before the calibration process began, the team established primary and secondary categories of calibration metrics. Secondary metrics were intended to be more visual observation of differences in groundwater influence on results and observations of flood inundation levels compared to other reported data. The primary calibration metrics were:

- **R – Squared** represents the proportion of the variance between a modeled and measured value. For the model results in this study, R-squared is based on the model stage results with tolerance levels ranging from a poor rating (0.60 to 0.70) to very good for a result at or above 0.90.
- **Standard Deviation** relates to the differences in the stage (in feet) between the recorded monitoring data and model simulation results. Tolerance levels ranged from a poor rating from (2.0 feet to 0.5 feet) to very good for a result less than 0.1 feet.
- **Continuity Error (Volume)** is the total error that occurs within the simulation process and is a measure of the total volume of runoff retained and accounted for in the model results. Due to computational processes in a model, this error takes the form of either additional volume that is introduced to the model or a reduction of volume discarded from the model run. Tolerance levels ranged from a poor rating with greater than 5% error to very good for less than 1% error.
- **Stage Difference.** Stage corresponds to a measured water level in the pond, storage area, creek, or river. The average metric is the average difference calculated over the full model run time and indicates whether the data overall are higher (positive result) or lower (negative result) than the average stage. The goal is to have a lower average stage difference. Therefore, stage differences were reported as only numeric results and were not given a poor to very good rating.

To guide the pilot model calibration effort, a calibration process was established that included the following five steps:

1. Evaluating the Base Model Performance;
2. Adjusting Physical Components (e.g., mesh refinement, breakline adjustments);
3. Reevaluating the Physically Adjusted Model's Performance;
4. Adjusting Model Parameters (e.g., hydraulic parameters, hydrologic parameters, groundwater parameters); and
5. Documentation of processes, results and observations.

An import aspect of a successful calibration effort is that true calibration is based on the availability of known results (i.e., recorded data) for the range of model conditions to be assessed. Due to a lack of available data within the Turbid-Lundsten model area, only the Edina model was brought through the calibration process outlined above. Recorded data within the Edina geography was still limited; and calibration largely relied on monitoring data within the creek (Mill Pond outlet and W. 56th street).

3.2.1 CALIBRATION RESULTS

The results of the calibration process have demonstrated that both models can be calibrated to within generally accepted calibration tolerances for the selected parameters: Stage (peak high-water level), Standard Deviation, R-Squared and Continuity Error.

Stage

Figure 4 shows the 77-day simulation for both calibrated models. Throughout the calibration process, ICM tended to have stage results higher than the recorded data at the 56th Street gage location while ICPR showed results lower than the recorded data.

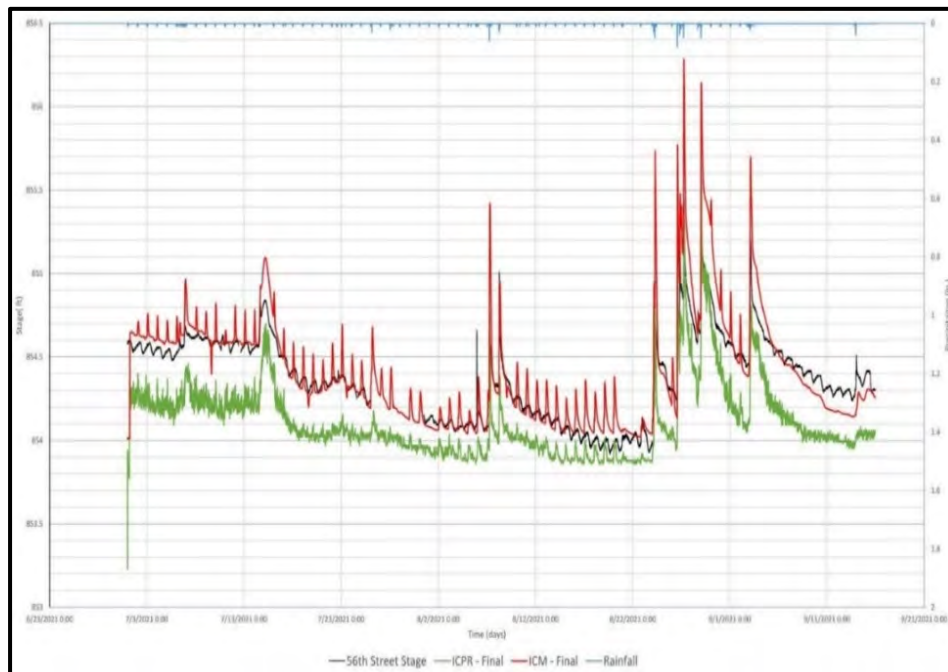


Figure 4. ICM – ICPR Long Term Stage Comparison

The overall shapes of both pilot model hydrographs follow the recorded data well. ICM tends to draw down at a slower rate after a peak stage than the recorded data and ICPR draws down much quicker than the recorded data. ICPR tends to have more “noise” in the hydrograph than ICM. The differences in the peak high-water level (stage) varies through a give storm simulation, although both models have produced peak elevation results that are within about 5 inches or less of the recorded data and average stage differences on the order of 1-4 inches for the September 2021 and July 2022 events.

R-Squared (RSQ), Standard Deviation, Continuity Error

At the outset of the calibration effort, the team established the goal of reaching a “Good” calibration rating for each of the listed calibration metrics. As illustrated in Table 2, both models were able to reach a calibration rating of Good to Very Good for the July 2022 long-duration event following the iterative calibration process.

Table 2. Model Calibration Results

| July 2022 Event | R-Squared (Stage) | | Standard Deviation (Stage) | | Continuity Error (Volume) | |
|-----------------|-------------------|-----------|----------------------------|--------|---------------------------|-----------|
| | Value | Rating | Value (ft) | Rating | Value (%) | Rating |
| ICM –Final | 0.894 | Good | 0.168 | Good | -0.01 | Very Good |
| ICPR – Final | 0.901 | Very Good | 0.106 | Good | +0.77 | Very Good |

3.3 Learnings

Throughout the pilot model calibration effort, several key learnings were identified that impact the approach to the watershed-wide build. These include:

1. Both models achieved acceptable calibration tolerances for the purposes of the pilot study. With additional effort and additional monitoring data to calibrate the models to, we believe both models would be able to converge on a very similar model result. Calibration effort will be greater for ICPR primarily due to the added complexity of the groundwater function.
2. Accuracy and Resolution of Terrain Data.

Additional surface feature data, such as channel cross-sections, are needed to supplement the baseline data provided from LiDAR. The largest constraint to developing a 1D-2D hydrologic/hydraulic model for extreme event analysis is the quality (or lack of) of terrain data in critical hydraulic control areas. Terrain files can be generated from survey data points, lidar files, contours, and a combination of all three. The original terrain surface was developed solely from LiDAR data. This data did not include information below the water line for Minnehaha Creek, for example, which did not allow for adequate drawdown of the creek to an elevation lower than the LiDAR surface. By including the channel information from available XPSWMM cross-sections and the Arden Park redevelopment topography, this challenge was effectively resolved.

The surface discrepancies may not be apparent during extreme rainfall and flow events due to the scale of water flowing in the creek but will be critical to the understanding of minor storm events and drought conditions throughout the watershed. This may require additional survey and elevation data to be obtained either through manual processes or partnership with individual agencies throughout the watershed to gather the required data. The more accurate and complete

terrain data in the geographic locations of calibration allow for more confidence in the calibration process over the range of small to large runoff event responses.

3. Vertically Varied Parameters

Within ICM, the Manning's n roughness coefficient can be varied up to three times (three spatial zones) depending on the depth within a cell. Within ICPR, the Manning's n roughness coefficient can be varied twice (shallow and deep). Both models allow for changes to the roughness values at each inundation level and changes to the inundation level breakpoints by roughness zone. The flexibility to adjust the parameter and level allows for a higher degree of calibration. As future data collection efforts proceed with survey of channel sections, it will be beneficial to have photographs that correspond to the survey areas so that modelers can have a sense of what field conditions are when assigning these varied n-values with a give reach.

4. While the calibration process allows for additional confidence in the modeled results to be gained, the process is never truly finished. The calibration process can be reevaluated at any point for a given model when additional data is obtained and incorporated into the model including new monitoring data, terrain data or 1D infrastructure data.
5. The base data available from groundwater monitoring stations provided sufficient information to build and make assumptions relating to the starting depth for the groundwater surfaces in ICPR. As learned later in the ICPR scenario runs, the starting elevation for groundwater can have a noticeable impact on model response. Obtaining additional groundwater elevation data (or assumptions) would be beneficial to allow a larger model to be created with multiple 2D groundwater zones where the starting elevations can be different and the inflow areas to surface water features can be further refined.

3.4 Scaling Considerations

Considerations related to scaling the calibration process efforts to the watershed wide model build effort include the following:

1. Resolution of monitoring stations.

Additional monitoring station data will be critical to the future calibration of the watershed-wide model build. Additional spatial distribution of the monitoring data will help to calibrate individual segments (subwatersheds) of the overall system. Adjusting parameters to meet a single comparison point may be valuable to understanding the sensitivity of the model in general, however, calibrating to a single location can result in too broad of assumptions of the runoff parameters, for example, throughout the entire watershed.

More data collection locations and a higher data recording frequency during a runoff events are desirable to improve the calibration process and the corresponding confidence in subsequent scenario simulations. Emphasis should also be placed on collecting continuous stage and discharge data within each subwatershed, outlets of major tributaries, and key areas along Minnehaha Creek will be essential for proper calibration.

2. Range of calibration events

The bulk of the available monitoring data was collected during the 2021 and 2022 open water seasons. Typically, two years of data provides a range of creek flows and responses to varying rainfall events (small, medium, large events). However, both 2021 and 2022 were drier than normal years for MCWD. For the watershed-wide build, there ideally will be access to monitoring

data that spans a wider range of water-level and flow conditions. This is clearly outside anyone's control, but longer periods of recorded data should help yield a variety of conditions to reference.

3. Data inputs for groundwater.

For ICPR, the extent of groundwater datasets provides a baseline for setting up and using the 2D groundwater surface in the model. Referencing all available hydrogeologic data will allow for improved definition of the groundwater region, initial water table settings, and inflow.

This additional groundwater data will be helpful to support "phased" groundwater regions for watershed-wide scaling. As discussed elsewhere in this report, creating multiple groundwater regions may help manage the longer run times as well as providing a more complete picture of the relative groundwater contributions to key surface water resources, especially in the areas where field data is present.

4. Improved channel cross-sections.

Additional channel survey/cross-section information at critical channel locations throughout the watershed that are spatially referenced should be collected to support the watershed-wide build. Additional detail is especially important at and near the current and planned flow and stage monitoring locations.

5. Both models encountered some model instability issues that caused the model to crash.

- For ICM, the instabilities were most commonly due to 1D-2D connection elevation variances at the end of pipe runs. One of the most common causes resulted when a pipe discharges below what the model sees as the water level of a creek or pond. This can be due to LiDAR data not having a surface below the normal water level. This can be addressed by including storage below the normal water level in either a 1D or 2D format for wet features. Another common cause resulted from pipes that do not have raw invert data being automatically assigned inverts with a DEM offset from the user input parameter. This new downstream invert is then set below the DEM and will need to be adjusted during the model-build and verification process.
- The ICPR model encountered model instability issues when starting elevations within nodes were not properly assigned. The default water surface elevation is set to the rim of the structure/node at the beginning of the simulation. This produces high velocities and flow rates within the 1D pipe network. The starting water surface elevation at a node must be changed to correspond with the lowest pipe invert at the structure to eliminate this instability.

4.0 SCENARIO ANALYSIS

The Scenario Modeling report provides an overview of the selected model runs, results, and learnings. Three categories of model runs were conducted, each aimed at learning something different about the two platforms. The objective of each category is described below:

- **Rainfall Scenarios:** These runs look to compare the results of ICM and ICPR to identify where we see differences and whether observations seen during calibration hold consistent in other areas of the watershed and under a wider range of rainfall conditions.
- **Geospatial Scenarios:** These scenarios look to reveal the differences and challenges associated with (1) incorporating adjusted spatial data and (2) model functionality and performance.
- **ICPR Groundwater Sensitivity:** These runs look to examine the level of influence ICPR's 2D groundwater component has on surface water results.
- **Run Time Scenarios:** These runs look to compare run times across varying degrees of mesh resolution.

4.1 Rainfall Scenarios

Several different rainfall scenarios were evaluated to compare results between the two models including:

- Comparing the modeled peak water surface elevations along Minnehaha Creek through the Edina subwatershed to the FEMA published Base Flood Elevation (BFE);
- Comparing the peak water surface elevations for the 10-year and 100-year events in the Edina subwatershed to the Edina localized flood maps;
- Comparing peak discharge rates leaving the Turbid-Lundsten subwatershed at Highway 5;
- Comparing peak water surface elevations and peak discharge rates along Minnehaha Creek through the Edina subwatershed for the 2014 Flood of Record rainfall event; and
- Comparing the peak water surface elevation, peak discharge rate and continuity error for the 2-year, 10-year and 100-year, Atlas-14 Design Storms in the Edina subwatershed.

In addition to the capabilities of each model related to simulating each of the event scenarios noted above, both models are fully capable of producing simple and complex graphical output results to illustrate important model results data. A sample of the more common and typical output from the ICPR Model (left) and ICM Model (right) is shown in Figure 5. The images illustrate the triangular mesh elements shown in the black outlines for ICM and the irregular mesh elements as the white outlines for ICPR. Both images also show the irregular size of the mesh elements with greater detail (smaller cells) in the areas of greater elevation change along the creek channel and larger cells in areas in the areas with smaller elevation changes.

Overland inundation depth results in both images are represented for each individual mesh element for each model. ICM displays the inundation depth as light to dark blue (shallow to deep), while ICPR displays the inundation depth as purple to green to blue to yellow to red (shallow to deep). The background colors in both images represent the elevation layer directly from the DEM.

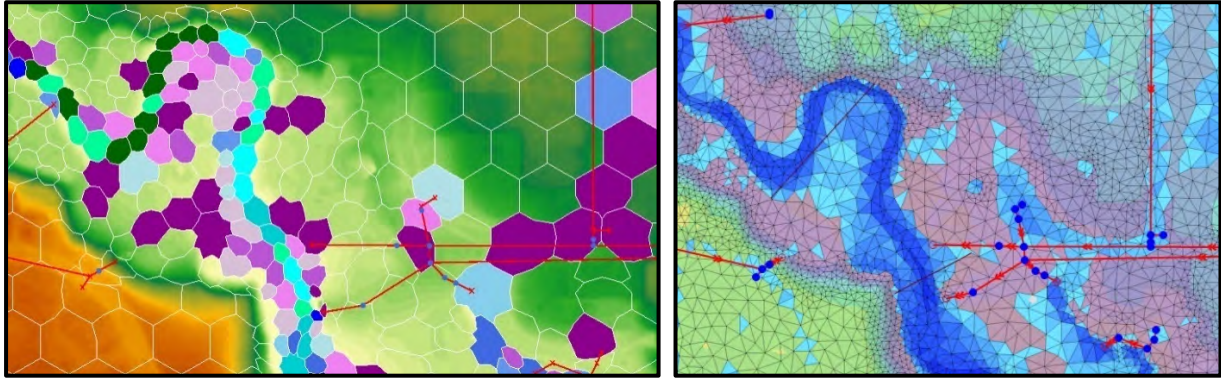


Figure 5. Sample Model Output Results for ICPR (Left) and ICM (Right)

4.2 Geospatial Scenarios

Two scenarios were evaluated to compare results between the two models related to geospatial changes including:

- Comparing peak discharge rates and volumes leaving the Turbid-Lundsten subwatershed at Highway 5 for the 2-year, 10-year, 100-year 24-hour events and the 100-year 10-day event, when changing the land use conditions from Pre-settlement to Existing to Future Development conditions; and
- Comparing the modeled peak water surface elevations to the results of the Edina Neighborhood flood reduction project for the 2-year, 10-year, and 100-year 24-hour design storm events.

ICPR and ICM were found to have a similar level of effort required to update landuse and swap out DEM files for the various land use scenario runs. ICM allows for multiple options when importing including overwrite, prompt, merge, and ignore when duplicate features are encountered during import. ICPR requires that the import dataset is clipped to only include the new/updated features. This allows for efficient updates and removal of previously created features. Figure 6 illustrates the results from both models for the discharge rates at Highway 5 leaving the Turbid-Lundsten pilot subwatershed. Both models show similar and expected trends of increased peak discharge from pre-development to existing and from existing to future conditions. The difference in results between the two models was not of concern since neither model was calibrated.

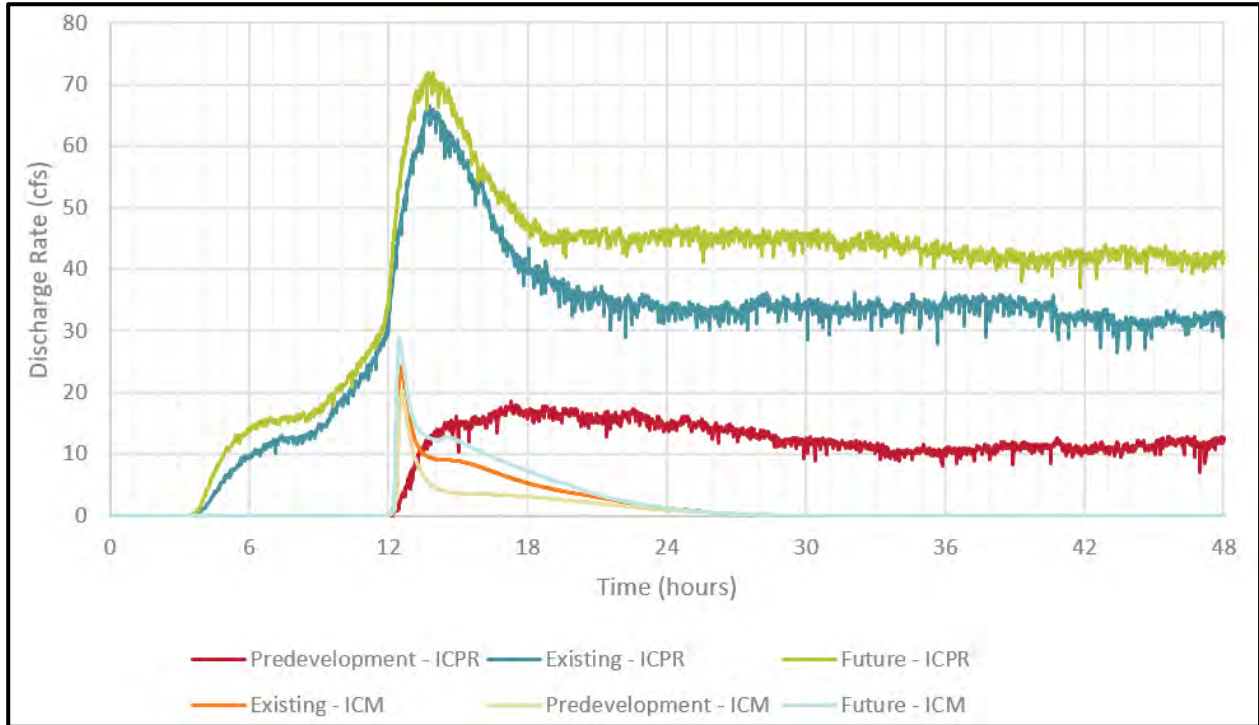


Figure 6. Discharge Rate Hydrographs – 2-year, 24-hour

4.3 ICPR Groundwater

To further understand the influence of the ICPR groundwater module and its impact on results, three model runs were completed to assess the impact of the starting groundwater level condition on the model results:

1. Low: Constant elevation of 853 feet for the entire model area;
2. High: Matching the terrain (e.g., water table is at the ground surface level); and
3. Varied: 6-feet below the terrain. (the level used for all model build, calibration, validation and scenario analyses).

Results showed that the initial groundwater elevation assumption can have a significant impact for smaller storm event results when assessing high-water level results on ponding and low areas as shown in Figure 7. Groundwater level assumptions had a smaller impact on larger events results and on creek peak flow and stage results.

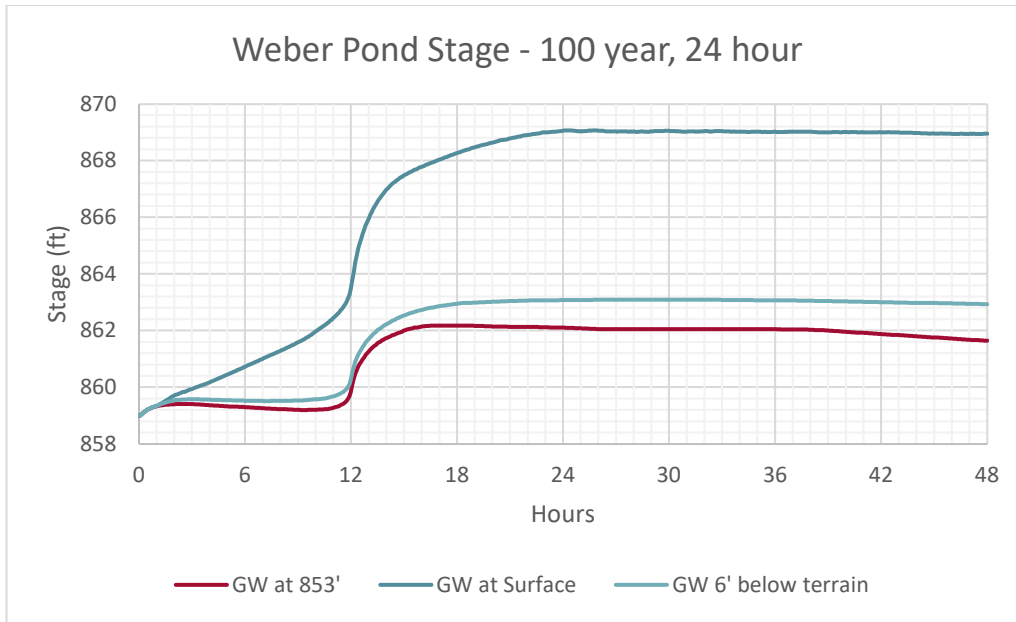


Figure 7. ICPR: Impact of Groundwater Initial Conditions on Weber Pond Stage

At a minimum, the modeled differences in high-water level for both Weber Pond and 56th Street highlight the need for greater emphasis on having confidence in the groundwater elevations throughout the watershed if ICPR is the selected model for the watershed-wide build. Over long simulation time periods, including a 77-day simulation, the influence of groundwater is allowed to equilibrate as the model “warms-up” at the beginning of the simulation time. This allows the ICPR model to simulate groundwater more accurately over longer periods such that the influence of the starting groundwater elevation becomes less significant.

4.4 Model Run Times

Model run times can be an important factor for modelers, primarily when considering the model build and calibration process. As each iteration of model adjustments are made, the model is executed to confirm initial build steps to debug the model and, once built and running, calibration adjustments are made to refine the results; longer run times mean fewer iterations can be completed for a given time period. The net result may be on the order of a few hours to several days if the model crashes towards the end of a long run or crashes during an overnight run, for example. Model run times are not as critical once the initial model is created and calibrated since there is generally not such urgency to obtaining results for a given scenario run.

The models were developed and run using various laptop setups to assess the overall usability and processing power needs and considerations. A computer with a good graphical processing unit (GPU) will be beneficial to reduce simulation run times for ICM. A computer with a fast CPU is beneficial for performing ICPR simulations and reduce overall run times.

Run times were recorded and compared for the full range of scenarios described in the previous sections. While ICM run times were generally lower for most scenarios, these results do not represent a

true apples-to-apples comparison, considering that the models built at different resolutions and included different adjustments to achieve acceptable calibration tolerances.

To evaluate run times on a more representative apples-to-apples basis, model resolution was set to be comparable between the two models. The purpose of these runs was solely to evaluate run times, and the impact the resolution change had on results was not considered. A computer with NVIDIA Quadro T2000 with Max-Q Design GPU and an Intel Core i7-10850H CPU was utilized for the comparison. Results for a 100-year, 24-hour design storm event run are presented in Table 3.

Table 3. Model Run Times for the Turbid-Lundsten Pilot Area

| Resolution | ICM | | ICPR | | |
|------------|--------------------|------------------|--------------------|------------------|----------------------|
| | Run Time (minutes) | # of 2D Elements | Run Time (minutes) | | # of 2D Elements |
| | | | Overland Only | With Groundwater | |
| Low | 20 | 12,053 | 33 | 47 | 11,900 |
| High | 42 | 92,931 | 78 | 106 | 50,842 ¹ |
| | | | 169 | N/A | 105,498 ² |

¹ ICPR high-resolution run developed from hand-delineation tools (breakpoint offset, breaklines)

² ICPR high-resolution run developed from automated build tool

The results indicate that longer run times will be experienced with ICPR, even if the groundwater portion isn't included. ICM is known for its fast run times and this advantage over ICPR stems from its ability to process on GPU's vs CPU's. These run times will increase for both models as they are scaled watershed-wide and as greater resolution is created.

4.5 Learnings

Throughout the scenario analysis processes a few important lessons were learned that will be beneficial for the District and its partners to understand when moving into the watershed-wide model build. Key learnings include the following:

1. For both models, adding sufficient detail to the mesh and manually refining mesh elements through breaklines and break points within critical hydraulic areas is critical to allow the model to move water in a more representative manner for a wider range of hydrologic conditions.
2. One interesting observation with ICPR relates to a consistently higher peak discharge rates which appears to relate to the computational processes within the pilot models. As we have discussed previously, ICPR shows much more variation in the peak discharge results with relatively high values shifting to lower values in subsequent time steps while ICM produces a much smoother hydrograph. For ICPR we recommend a standard process to use the values taken from a consistent approach (model or exhibit) based on the model users best professional judgement of viewing the hydrograph and reviewing the exported data.
3. The level of effort to swap land use files and set up new scenario runs with modified geospatial datasets required a similar level of effort for both models. No significant difference was experienced between the two models.

4.6 Scaling Considerations

Key considerations related to scaling the model build, calibration and scenario analysis process to the watershed wide model build effort include the following:

1. There are multiple decision points throughout the build process, such as mesh resolution, detail of pipe network to include, etc., and these decisions (both in number and their importance) will only increase as you scale watershed-wide. Further defining and prioritizing scenarios will help guide how those decisions should be made.
2. The inclusion of selected 1D features within the watershed-wide model build will allow for increases in efficiency and reduction in simulation run times. 1D features (ponds, lakes, channels) remove portions of the simulation area from the 2D calculation. This reduces the overall size of the model without losing accuracy within the model when the 1D features are accurately created and implemented. The 1D features should be created outside of the model as shapefiles and imported. This allows for creation of multiple model scenarios with consistency and for updates to be completed with new data as major surface water features are created within the watershed. The greatest value in creation of 1D features will be for the larger lakes and pond such as Lake Minnetonka, Lake Minnewashta, Lake Harriet and Long Lake.
3. Considering “phased” groundwater regions in ICPR may be important for watershed-wide scaling and would likely help manage the longer run times as well as providing a more complete picture of the relative groundwater contributions to key surface water resources.

5.0 MODEL EVALUATION AND COMPARISON

A critical aspect of the pilot model build project and the first task within the scope of work was to establish a clear, comprehensive evaluation framework that the District would ultimately use as a resource to inform which of the two models is best suited to meet the District’s current needs; and to understand the operational considerations and challenges of scaling the selected model watershed-wide. This section outlines the evaluation approach and framework that was developed, along with a summary of each model’s strengths and weaknesses.

5.1 Evaluation Approach

The framework was developed with two categories of evaluation factors, MCWD Model Uses and Model Operations. As shown in Figure 8, the first two sections (lines 1-8) address model uses and the remaining sections focus on model operations, function and model specific factors. The right-hand columns indicate the relative capabilities of each model for each of the evaluation factors.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|--|---------|---|---|------|
| | | | ICM | ICPR |
| MCWD Primary Model Uses | 1 | Produce channel and localized flood inundation maps | 2 | 2 |
| | 2 | Run long-term extreme wet or dry years to evaluate groundwater-surface water interactions | 0 | 2 |
| | 3 | Evaluate impacts of current and alternative regulation/policies on surface water quantity | 2 | 2 |
| | 4 | Quantify impact of regional volume management strategies on surface water quantity | 2 | 2 |
| MCWD Secondary Model Uses | 5 | Short-term channel and localized flood forecasting (consider snowmelt) | 2 | 2 |
| | 6 | Characterize water quality changes/impacts | 1 | 0 |
| | 7 | Provide boundary conditions for other models | 1 | 1 |
| | 8 | Establish updated FEMA certified flood maps | 0 | 0 |
| Data Processing | 9 | Accepted file formats of input datasets | 2 | 2 |
| | 10 | Repeatability of data process to model build ready data | 1 | 1 |
| | 11 | Manual processing effort to get model input data ready for model import. | 1 | 1 |
| | 12 | Manual data processing feedback loops. Ability to export manually adjusted data. | 2 | 2 |
| Model Build Processes (Including Calibration and Validation) | 13 | Model node limitations (scale capabilities) | 2 | 2 |
| | 14 | Default hydrology method and processing | 1 | 1 |
| | 15 | Watershed-wide construction considerations. | 2 | 1 |
| | 16 | Ability to carve out smaller sections of the model. | 2 | 1 |
| | 17 | Model resolution required to support primary uses | 1 | 1 |
| | 18 | 2D overland mesh methodology | 2 | 1 |
| | 19 | 1D-2D Connection Points | 1 | 1 |
| Model Function and Results (Scenario Analyses) | 20 | Pump system functions/capabilities | 1 | 1 |
| | 21 | Method/approach to calibration | 1 | 1 |
| | 22 | Ease and options for BMP evaluation | 1 | 1 |
| | 23 | Ease of land-use change scenarios | 1 | 1 |
| Software Specifics | 24 | Model runtime (common processing system) | 2 | 1 |
| | 25 | Results quality and output format | 2 | 1 |
| | 26 | Export process and format | 2 | 2 |
| | 27 | Sharing model versions | 2 | 1 |
| Software Specifics | 28 | Local versus network - processing ease | 1 | 1 |
| | 29 | License type and cost | 1 | 2 |
| | 30 | Model maintenance (version management, security, technical support) | 1 | 1 |
| | 31 | User Community | 2 | 1 |

Figure 8. Evaluation Matrix: Categories, Factors and Ratings

Within the model uses category, the framework distinguishes between primary and secondary uses. The District recognizes that while this upcoming model build is intended to first and foremost serve climate planning needs, it's understood that a model at the watershed scale could serve as a replacement to some or all of the functionality that XPSWMM currently provides to the District and its partners. It was important that these two categories of uses were distinguished during the evaluation process.

Primary Uses: The Primary uses section lists modeling capabilities that were deemed essential to the District's ability to support climate adaptation planning. Emphasis will be placed on this category during model selection. The primary uses and how each factor supports the District's primary needs are provided in Table 4.

Table 4. Primary Use Factors to Support District Needs

| Evaluation Factor | District's Need |
|--|---|
| Produce channel and localized flood inundation maps | It is critical that the District can characterize the areas, frequency, and magnitude of flooding issues under current and future climate. |
| Run long-term extreme wet or dry years to evaluate groundwater-surface water interactions | It is important for the District to characterize how groundwater will respond to predicted rainfall patterns and how those responses impact flood risk. |
| Evaluate impacts of current and alternative regulation/policies on surface water quantity | The District needs to be able to quantify the impact varying policies/regulation will have on flooding and volume management at a systems scale. |
| Quantify impact of regional volume management strategies on surface water quantity (projects/BMPS) | The District needs to evaluate and quantify the impact of varying project strategies to prioritize actions within each subwatershed. |

Secondary Uses: The secondary model uses section includes capabilities from which the District and its partners would benefit, although these metrics will not drive model selection. Many of these secondary uses have historically been or can be obtained from other modeling tools. It's important to characterize how ICM and ICPR could serve these needs so the District can understand if any of the day to day operational needs and uses of XPSWMM could be replaced.

Operational Factors: The remainder of the matrix includes Model Operations factors that address each model's ability to efficiently be built, run, and for data to be exported to a usable format, as well as factors addressing how each model may be scaled and maintained considering a watershed-wide application.

Additional detail on the list of secondary uses and operations factors is presented in the Evaluation Framework Memorandum.

Observations in the matrix were first populated by the data development and modeling team members, then supplemented by the model software developers and finally were refined into a single set of observations and ratings.

The following sections summarize the key findings presented in the matrix for the District's defined primary and secondary uses and for the overall model operations. These sections are intended to highlight any observed strengths and limitations of each model.

5.2 Model Use Comparison

5.2.1 PRIMARY USES

Both models are able to characterize and quantify flood risk across the watershed and both are capable of evaluating how changes in policy and/or projects may impact the surface water runoff contributions to water bodies and the creek. This is due to their matched ability to include detailed 1D pipe networks and track 2D overland flow. However, by far the most distinguishing difference between the two platforms comes down to how they represent groundwater. ICPR is unique and one of few models that has a true integrated surface-water groundwater component. This means that ICPR positions the District to understand how surficial groundwater is responding to forces on the surface, such as land-use change or increased precipitation, and characterize surficial groundwater flow. ICM models groundwater in a much more simplistic way and the user is not able to characterize how the water table itself is responding to surface adjustments.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|----------------------------------|---------|---|---|------|
| | | | ICM | ICPR |
| MCWD Primary Model Uses | 1 | Produce channel and localized flood inundation maps | 2 | 2 |
| | 2 | Run long-term extreme wet or dry years to evaluate groundwater-surface water interactions | 0 | 2 |
| | 3 | Evaluate impacts of current and alternative regulation/policies on surface water quantity | 2 | 2 |
| | 4 | Quantify impact of regional volume management strategies on surface water quantity | 2 | 2 |

Differences: The primary difference in the two models is that ICPR is able to track and simulate horizontal groundwater movement and ICM is not.

5.2.2 SECONDARY USES

Of the four secondary uses, both models are fully capable to complete short-term channel and localized flood forecasting and capable to provide boundary condition inputs to other models. Neither model is currently nationally accepted by FEMA as a model format for producing official flood mapping. Both models have one or more local or regional examples of being accepted. ICM is currently in the process of seeking approval.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|------------------------------------|---------|--|---|------|
| | | | ICM | ICPR |
| MCWD Secondary Model Uses | 5 | Short-term channel and localized flood forecasting (consider snowmelt) | 2 | 2 |
| | 6 | Characterize water quality changes/impacts | 1 | 0 |
| | 7 | Provide boundary conditions for other models | 1 | 1 |
| | 8 | Establish updated FEMA certified flood maps | 0 | 0 |

Differences: The primary difference in the two models is in the capabilities to characterize water quality changes and impacts. It is important to note that this capability was not directly evaluated during the pilot study and the capability assessment is based on review of the materials available from the model

creators. ICM is capable of modeling total phosphorus (TP) and other parameters of interest to the District. ICPR does not currently have capabilities to track nutrients or other urban runoff pollutants.

5.3 Model Operations Comparison

5.3.1 DATA PROCESSING

Both models draw on the same underlying datasets to construct the model. Furthermore, the automated processing framework established during the pilot project is primarily model agnostic, leaving only the final step dependent on platform, which processes datasets into each model’s required input format. No significant differences were identified between the two models when considering the pre-processing work.

Manual adjustments will be required within ICM and ICPR to make additional corrections to the underlying datasets. Both models flag these changes and would support the development of a feedback loop process to prevent repetitive changes always being made to 1D infrastructure elements.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|---------------------|---------|--|---|------|
| | | | ICM | ICPR |
| Data Processing | 9 | Accepted file formats of input datasets | 2 | 2 |
| | 10 | Repeatability of data process to model build ready data | 1 | 1 |
| | 11 | Manual processing effort to get model input data ready for model import. | 1 | 1 |
| | 12 | Manual data processing feedback loops. Ability to export manually adjusted data. | 2 | 2 |

Differences: There were no significant differences identified between the models related to data processing. ICPR requires one additional step to get the infrastructure data in the GWIS format for model import, but that is accomplished easily through the automated scripting tools.

5.3.2 MODEL BUILD PROCESSES

For six of the nine factors in this evaluation category, both models are equally capable with only minor differences in the approach each model takes for a given factor and the associated level of effort. The primary difference in the effort needed to build the models relates to the added groundwater function in the ICPR model. That is, this additional step to add a second 2D surface requires the modeler to import and create the second 2D surface for groundwater including replicating the surface 2D grid and creation of new breakpoints for the groundwater mesh. When considering the build process and calibration, this effort could be twice the effort needed to build and calibrate a similar resolution ICM (surface only) model.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|--|---------|---|---|------|
| | | | ICM | ICPR |
| Model Build Processes (Including Calibration and Validation) | 13 | Model node limitations (scale capabilities) | 2 | 2 |
| | 14 | Default hydrology method and processing | 1 | 1 |
| | 15 | Watershed-wide construction considerations. | 2 | 1 |
| | 16 | Ability to carve out smaller sections of the model. | 2 | 1 |
| | 17 | Model resolution required to support primary uses | 1 | 1 |
| | 18 | 2D overland mesh methodology | 2 | 1 |
| | 19 | 1D-2D Connection Points | 1 | 1 |
| | 20 | Pump system functions/capabilities | 1 | 1 |
| | 21 | Method/approach to calibration | 1 | 1 |

Differences: The factors where ICM has an advantage related to:

- Watershed wide construction considerations. The groundwater surface adds a level of complexity to building the ICPR model and poses questions on creating separate groundwater regions within a single model (which was not tested during the pilot build). Both models have the terrain sensitive meshing tool, although creating a model with groundwater eliminates the ability to use the automated function in ICPR.
- Ability to carve out smaller sections of the model. Both models have a scenario manager function/option that allows for separate/discrete model areas to be created and saved, which will be beneficial as the District evaluates numerous future climate scenarios. However, ICM retains a direct connection of sub-models to base model whereas ICPR scenarios are not linked to the base model after creation. This difference gives a slight edge to ICM in terms of building separate smaller sections of the model in specific areas of interest. In addition, providing BC to other models is easier in ICM through the use of 2D results objects. Which are essentially point, lines, and objects that allow you to extract more complete information form the model.
- 2D overland mesh methodology. The more complex mesh element build process in ICPR requires the user to expend more time to build and refine the model (mesh) to produce a working and calibrated model. Without the use of automated tools when using ICPR with groundwater, the modeler must manually place additional breaklines and breakpoints to define flow directions and critical hydrologic/hydraulic features.

5.3.3 MODEL FUNCTION AND RESULTS (SCENARIO ANALYSES)

For three of the five factors in this evaluation category, both models are equally capable with only minor differences in the approach each model takes for a given factor. For model run times in general for a common processor and comparable resolution, ICM is generally faster even when comparing only 2D surface runs without an ICPR groundwater layer. Based on the model runs from the automated 2D builds under this pilot study, ICM was observed to have better visual results (less noise) and more stable hydrographs. ICPR stability may improve by going to 1D channels and ponds as was mentioned previously in this report.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|--|---------|--|---|------|
| | | | ICM | ICPR |
| Model Function and Results (Scenario Analyses) | 22 | Ease and options for BMP evaluation | 1 | 1 |
| | 23 | Ease of land-use change scenarios | 1 | 1 |
| | 24 | Model runtime (common processing system) | 2 | 1 |
| | 25 | Results quality and output format | 2 | 1 |
| | 26 | Export process and format | 2 | 2 |

Differences: ICM was observed to have an edge in processing speed when comparing only the 2D surface runs and more consistent output results format (i.e., the same exported result as what was shown in the model compared to ICPR where differences were observed between the internal model results and exported result for peak flows. ICM runs on a GPU and has the ability to run true parallel processes.

5.3.4 SOFTWARE SPECIFICS

For two of the five factors in this evaluation category, both models are comparable with significant differences in the specific software related factor. For ease of sharing model versions, ICM has transportable database which is more portable from a file size transfer standpoint compared to copying a full folder for ICPR.

For the user community factor, Innovyze reports over 450 consultant and communities using ICM in the US and the users in Minnesota are starting to see ICM be used in some areas. ICPR is widely used in Florida and the model developer is currently working with more than a dozen universities for research and teaching. We are not aware of any communities in Minnesota utilizing ICPR at this time.

| Evaluation Category | Line ID | Evaluation Factor / Description | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|---------------------|---------|---|---|------|
| | | | ICM | ICPR |
| Software Specifics | 27 | Sharing model versions | 2 | 1 |
| | 28 | Local versus network - processing ease | 1 | 1 |
| | 29 | License type and cost | 1 | 2 |
| | 30 | Model maintenance (version management, security, technical support) | 1 | 1 |
| | 31 | User Community | 2 | 1 |

Differences: ICM's transferrable database is a nice feature that allows sharing with much smaller file sizes than ICPR. Related to licensing and costs, ICPR has a substantially lower cost with an annual subscription cost of \$2,400 per simultaneous user. ICM has a significantly higher annual subscription cost of \$18,000 for the high-end version.

5.4 Conclusion

From the outset of this pilot study the District's goal was to evaluate how ICM and ICPR could be built with the assistance of automated tools to a watershed-wide scale and to assess how each model would allow the District to meet their climate planning goals. Both models can meet the District's primary uses

for characterizing flood risk and evaluating impacts of regulation and policies on runoff quantity.. However, only ICPR has the ability to meet the District’s primary use goal to better understand and quantify groundwater-surface water interactions within the watershed.

While ICPR’s capabilities are well aligned with the District’s needs for climate planning, it does score lower across many operational categories; this indicates that ICPR has less refined operational features than ICM and/or is more challenging to build and operate than ICM.

Climate Planning with ICPR

ICPR will allow the District to more holistically understand watershed volume management through the inclusion of groundwater, while still serving the other primary uses relating to assessing flood risk and evaluating adaptation strategies. However, this added benefit comes with increased technical challenges and level of effort to construct and operate the model. To build ICPR at a watershed-wide scale, while still maintaining a high-resolution surface, close attention will be needed when constructing the groundwater region(s). Difficulties were experienced during the pilot model build that led to surface resolution being sacrificed in order to manage the need to effectively mirror the groundwater and surface water meshes. There are alternative techniques to construct the groundwater region that may help alleviate those issues.

While ICPR will be strong in serving the District’s primary uses, it does pose more relational challenges. ICPR is not well accepted within this region, meaning consultants and partners are largely unfamiliar with this software and it will be more difficult to interplay with partner models. Because of this, ICPR will likely not serve the District as well as other models beyond immediate climate planning needs.

Climate Planning with ICM

ICM is limited in its ability to grow the District’s understanding of surface water-groundwater interactions, however it can serve the other primary needs for climate planning very well. ICM is known for its fast run-times and as the District looks to evaluate a wide range of future climate challenges and related policy changes to adapt, ICM has a greater ability to create, track changes and manage multiple model scenarios.

ICM would also allow the District to better coordinate and share model information with partners due to the broader base of model users and the more simplified processes to carve out sections of the model for use in areas of focus. ICM’s strengths make the model better suited to serve District and partner modeling needs beyond immediate climate planning.

APPENDIX A – DATA DISCOVERY MEMORANDUM



MEMORANDUM

To: Kailey Cermak, Project Manager | Minnehaha Creek Watershed District

From: Ron Leaf, Project Manager | Kimley-Horn

Date: June 1, 2022

Subject: 2D Pilot Model Build – Data Discovery Memorandum

BACKGROUND AND PURPOSE

The Minnehaha Creek Watershed District's (MCWD) current modeling tools do not provide the required granularity and features to answer pressing climate change questions and evaluate adaptation strategies. The District identified the need to build a new tool that not only quantifies volume but represents how water moves through the watershed via runoff, storm pipes, wetlands, best management practices and surficial groundwater. However, maintaining such a detailed, large-scale model hinges on the premise that repeatable automated workflows can be established to process and integrate the storm sewer datasets of the 29 municipalities within the District. The District pursued a pilot model build, in part, to help mitigate for and better understand this technical risk.

A key objective of the pilot model build is to establish scalable automated workflows for processing model inputs. An essential step in the development of the automated processes is to understand the base data available for building the models and to define the extent of data gaps that may impact the automated model build process for both models.

This data technical memorandum documents existing datasets supplied by the District, City, and other sources; provides a summary of the data input needs for the ICM and ICPR4 models; and identifies the gaps in data that will require further review and action to produce a model-build ready dataset. While this memorandum focusses on data for the pilot model build areas in Edina and the Turbid-Lundsten subwatersheds, Kimley-Horn has also completed a screening level review of four datasets from other municipalities within the larger MCWD jurisdictional boundaries. This screening level review was intended to provide a general awareness of data formats and potential issues that may arise beyond the scope of this pilot study.

Review and evaluation of available datasets is expected to be an iterative process that will conclude with the identification of gaps between the data import requirements for each model and the actual data available.

DATA SETS PROVIDED

Several datasets were provided in multiple formats and data types for model development. Table 1 summarizes the base files publicly available or provided by the District and its partners. Data types

include shapefiles, geodatabases, and LIDAR (.laz) files. Shapefiles are used to store spatial data in the form of points, lines, or polygons. The shapefile includes an attribute table which lists data for each shape included in the shapefile. The attribute table can hold data in the following data types: integer, float, double, text, and date. Geodatabases are typically used to provide file and folder management of large spatial datasets. Geodatabases can also be easily zipped and transferred to another folder location. Geodatabases can hold multiple shapefiles of the same or different types of spatial data. The geodatabase holds the shapefiles as a feature class. Feature classes can be grouped under a Feature Dataset within the geodatabase. This folder structure allows for increased file management ability.

Table 1. Summary of Datasets Provided for Pilot Areas

| Item | Dataset Description | Data Type (Subtype) | Provider / Source (Source year) | Spatial Reference |
|------|---|-------------------------------------|--|---|
| A | Watershed Boundary – PilotAreas.gdb | Shapefile (polygon) | Edina – Subwatershed Dataset | NAD 1983 HARN Adj MN Hennepin (US Feet) |
| | | Shapefile (polygon) | Minnehaha Creek Watershed District – HHPLS | NAD 1983 UTM Zone 15N |
| B | Landuse/Land Cover – plan_generl_Induse2020.gdb | Geodatabase (polygon feature class) | Metropolitan Council (2016, 2020) | NAD 1983 UTM Zone 15N |
| C | Soils Data | Shapefile (polygon) | NRCS (2003) | WGS 1984 |
| D | Geologic Atlas | Geodatabase (polygon feature class) | Carver County University of Minnesota (2009) | NAD 1983 UTM Zone 15N |
| | | | Hennepin County University of Minnesota (2018) | NAD 1983 UTM Zone 15N |
| E | LiDAR | LAZ | MnDNR (MnTOPO, 2011) | NAD 1983 UTM Zone 15N |
| F | Pipes – DGravityMain | Geodatabase (line feature class) | Edina | NAD 1983 HARN Adj MN Hennepin (US Feet) |
| | Pipes - pipes.shp | Shapefile (line) | MnDOT (Turbid Corridor) | WGS 1984 |
| | Pipes – CG_StormCulverts | Geodatabase (line feature class) | Carver County (Turbid Corridor) | NAD 1983 HARN Adj MN Carver (US Feet) |
| G | Manhole - DManhole | Geodatabase (point feature class) | Edina | NAD 1983 HARN Adj MN Hennepin (US Feet) |
| H | Flared End Section - End_Sections.shp | Shapefile (point) | MnDOT (Turbid Corridor) | WGS 1984 |
| | Flared End Section – CG_StormOutlets | Geodatabase (point) | Carver County (Turbid Corridor) | NAD 1983 HARN Adj MN Carver (US Feet) |
| I | PW_Storm_Features | Geodatabase (feature dataset) | Edina | NAD 1983 HARN Adj MN Hennepin (US Feet) |

Additional detail on specific dataset parameters for the municipal infrastructure (i.e., pipes, culverts, catch basins and manholes) is provided in Table 2A within Attachment B to this memorandum. Table 2A

identifies the minimum preferred data needed to build a functional model and which parameters are more suitable for using assumed or assigned values where project are-specific data is not available.

LAZ files are used to store point elevation (LIDAR) data in a compressed format. To access and use the LIDAR data, the file must be decompressed and transferred into the LAS format. The LAS file can then be used to create a digital elevation model (DEM). The DEM is included directly in the modeling software to create the 2D surface. The LAS point files can also be used directly within the modeling software to develop the 2D surface. The final column in the table lays out the spatial reference that each dataset uses. The spatial reference for each will need to be transformed to Universal Transverse Mercator (UTM) Zone 15 North for use in both of the modeling software packages. This spatial projection was chosen due to the size of the future watershed-wide model.

Within several of the base datasets, additional detail is available for specific features. For example, within Edina's *PW_Storm_Features* file, there are more than a dozen subcategories of storm sewer features including *BMP*, *DManhole* and *DOutlet*, for example. The City's database also includes a number of files that are not needed for the model build processes including those listed from *StormGravityMain_Jetting* to *StormMS4_OutletInspectionHasOutlet*. A screen clip of the detail for this portion of the City's database file is provided in Attachment A. Attachment A also includes a screen clip of the detail for the Turbid pilot subwatershed.

MODEL INPUT NEEDS

This evaluation of available data is driven by the District's goal to develop automated processes that will process existing infrastructure and geospatial data into a standardized central geodatabase and then processed into hydrologic and hydraulic model input data with both one-dimensional (1D) and two-dimensional (2D) elements. Both models require very similar data to support building a functioning model. The following sections describe the data that is generally required to build each model to a level that would yield meaningful model results in support of District goals. As the scripting process workflow is developed, these datasets will be mapped from raw input files to model build ready formats. Table 2 defines the shared and specific data inputs that each model requires.

Coordinate Systems

All input spatial datasets need to be in matching coordinate systems. The overall watershed spans two counties and is approximately 26 miles long. The standard coordinate system to be used for the spatial datasets and model build will be Universal Transverse Mercator Zone 15 North (UTM 15N). Coordinate system transformation will need to be performed on any dataset that does not match the standard.

ICM vs. ICPR4 Model

Development of a combined 1D/2D ICM model or ICPR4 model requires data that be divided into two categories, **model base data** and **hydraulic network features**. In both categories, the data is needed as a direct input model parameter or is needed to generate (calculate) a required model parameter. These model input categories can be characterized and further subdivided as summarized in Table 2. Items in *italicized text* indicate that the input data needed or preferred is significantly different between the two models.

Table 2. Shared vs. Specific Data Inputs

| Item | Data Input | ICM | ICPR4 |
|------|----------------------------|--|--|
| A | Coordinate System | NAD 1983 State Plane Minnesota South FIPS 2203 (US Feet) | NAD 1983 State Plane Minnesota South FIPS 2203 (US Feet) |
| B | DEM/Ground Surface | <i>Elevation Point Data</i> | <i>Raster Data</i> |
| C | Soils Data | Soil Zones | Soil Zones |
| D | Land use/Land Cover | Zones | Zones |
| E | Lookup Tables | CN, Impervious, Manning's n, Inlet Head Discharge Curves | CN, Impervious, Manning's n, Inlet Head Discharge Curves |
| F | Rainfall | Depth and Distribution | Depth and Distribution |
| G | Nodes | Subsurface Junctions (manholes), 2D Interface Nodes (inlets) | Subsurface Junctions (manholes), 2D Interface Nodes (inlets) |
| H | Links | Pipes, <i>1D/2D Links (Open Channel Crossings)</i> | Pipes, <i>Rating Curves, Percolation</i> |
| I | 1D/2D Interface Elements | <i>Storage Area Volume Controls</i> | <i>Pond Control Volumes, Channel Control Volumes</i> |
| J | 2D Overland Domain | Grid (Triangular) | Grid (Triangular) |
| K | 2D Terrain Characteristics | Building footprints, Breaklines, Breakpoints | Building footprints, Breaklines, Breakpoints |
| L | Groundwater | <i>Infiltration Parameters</i> | <i>2D Domain</i> |
| M | Boundary Conditions | Overland, <i>Groundwater</i> | Overland, <i>Groundwater</i> |

DATA GAPS

During this initial data review process, we have categorized data into one of three groups based on how well the current raw dataset is suited to automated model build processes. Categories range from model-build ready to major gaps, where the dataset does not have required data to build a working model.

- Model-build ready. These data sets are complete and can be processed for model import without having to supplement data using engineering assumptions.
- Minor gaps. These data gaps can generally be addressed through the automated scripting process. An example of a minor gap is a pipe segment that is missing the pipe size that is located between two adjacent pipes with known pipe sizes. This is considered minor as the automated scripting process can resolve the gap by assigning the missing pipe size as the size of the downstream adjacent pipe (or any assigned rule) and flagging it in the database as an assumed data value. Another example is development of a runoff curve number for a drainage area. While the base data do not directly contain curve numbers, the data can be processed/calculated based on the land use and soils data, for example.
- Major gaps. These data gaps consist of missing data or parameters that cannot be assumed unless additional data is provided and data that may require additional field work or data collection efforts by the District or City of Edina.

Based on our initial review of the data, we have identified the following minor and major gaps. In general, these gaps apply to both models, although where specific to one model, that model is identified directly.

Minor Gaps

- Node Data:
 - Invert Elevations
 - Issue: Incomplete or Missing Data.
 - Solution A: Develop process to calculate or assign an assumed value to the incomplete or missing data based on Link data.
 - Solution B: Field survey of incomplete or missing data.
 - Rim Elevations
 - Issue: Incomplete or Missing Data.
 - Solution A: Develop process to calculate incomplete or missing data
 - Solution B: Acquire new lidar data for areas of incomplete or missing data using the process developed in Solution A to assign elevations.
 - Solution C: Field survey of incomplete or missing data.
 - Inlet restrictions
 - Issue: Request for varying levels of inlet restriction to simulate in-place conditions
 - Solution A: Adjust orifice size to simulate inlet restriction
 - Solution B: Vary the head-discharge curve to simulate inlet restriction
- Link Data:
 - Invert Elevations
 - Issue: Incomplete or Missing Data
 - Solution A: Develop process to calculate or assign an assumed value to the incomplete or missing data based on Node data.
 - Solution B: Field survey of incomplete or missing data.
 - Pipe Roughness
 - Issue: Parameter is not within data sets.
 - Solution: Parameter calculated based off pipe material.
 - Pipe Size
 - Issue: Incomplete or Missing Data.
 - Solution A: Develop process to calculate or assign an assumed value to the incomplete or missing data.
 - Solution B: Field survey of incomplete or missing data.
- Standard Hydrologic Parameters
 - Standard values for curve numbers, impervious percentages, and Manning's roughness parameters will be needed to generate the hydrologic parameters. One example of an area of further discussion is to define the assigned value of impervious cover for each land use category.
- Terrain
 - Extrusions/Blockages
 - Issue: Raw LAS data missing building data
 - Solution: Utilize Building Footprints shapefile to simulate building locations.

- Standard Coordinate System:
 - Files in Table 1 including Items C and F (Turbid) will need to be converted to NAD
 - 1983 State Plane Minnesota South FIPS 2203 (US Feet)

Note/Discuss: For models with 2D overland flow, the 1D/2D connection points may run better in the model if every node rim elevation is set based on the terrain data. This data field may be critical to effective modeling of the 1D/2D interface and further review of the variations in the data are needed to develop options for defining the model build input parameters. For example, with the surface being from 2011, it may not be advisable to use the surface elevation instead of rim elevations as some could be significantly off. Maybe not in the two pilot subwatersheds, but other areas where significant development may have occurred since 2011. We anticipate some type of screening process to evaluate how close the numbers are, and if close, then use the surface elevation so it matches the 2D surface. Example of one area where more discussion is needed.

Major Gaps

- Water Quality Data
 - ICM Model specific: The water quality modeling tools within the ICM simulation are set up to run point source pollutants that are either expressed in terms of concentration, for dissolved pollutants, or potency factor, for attached pollutants. If there is a desire to model specific pollutants for water quality, a pollutograph with time-varying water quality determinants would be needed to input into the system.
- Groundwater Modeling Data
 - ICM Model specific: ICM models groundwater using a soil storage reservoir and a groundwater storage reservoir. There is no interaction between ground water levels in adjacent subcatchments. To run a groundwater infiltration module event, initial soil saturation and initial groundwater levels are required in addition to soil parameters and baseflow threshold levels.

SUMMARY

For a successful automated build of the models in each software, at a minimum the data in rows A-H from Table 1 is required. Supplemental data included within the PW_Storm_Features feature database include BMP outlines, pump and forcemain locations, outlet control structures, sensors, flow control features, and unknown structures. These data types can be used for additional detail built into a model, although may not be suitable for direct automated model build processes due to their highly variable attribute data.

Overall, there are no major gaps in the available data needed to successfully build a functioning model in both ICM and ICPR formats. The primary differentiators between the two models in terms of data sources and data needs will be understood more fully as the scripting workflow and actual script writing and testing processes advance.








ATTACHMENTS

ATTACHMENT A – DATABASE DETAIL



















Edina Database

- Edina
 - 2022_0203
 - EDINA_STORM_FEATURES_20220124
 - v10
 - gis_sde.gdb
 - PW_Storm_Features
 - BMP
 - DCatchBasin
 - DConnectPts
 - DDischargePoint
 - DFitting
 - DFlood
 - DFlow_Control_Area
 - DFlow_Control_Feature
 - DFlow_Control_Structure
 - DGravityMain
 - DInletPoint
 - DLiftStation
 - DManhole
 - DOutlet
 - DPressurizedMain
 - DScadaSensor
 - DSumpClean
 - DSumpdrainTile
 - DUNKStructure
 - Storm_GeometricNetwork
 - Storm_GeometricNetwork_Junctions
 - StormGravityMain_Jetting
 - StormGravityMain_JettingHasDGravityMain
 - StormManhole_Jetting
 - StormManhole_JettingHasDManhole
 - StormMS4_InletInspection
 - StormMS4_InletInspectionHasDInletPoint
 - StormMS4_ManholeInspection
 - StormMS4_ManholeInspectionHasDManhole
 - StormMS4_OutletInspection
 - StormMS4_OutletInspectionHasDOutlet
 - 0000STORM FEATURES.lyr
 - v104

MnDOT – Carver County Data

- [-]  TurbidCorridor
 - [-]  TurbidCorridor
 - [-]  CarverCounty
 -  LaketownTownshipStormSewer.mpk
 - [-]  MnDOT
 -  End_Sections.shp
 -  pipes.shp
-

LaketownTownshipStormSewer.mpk

-  **gis_cartegraph.gdb**
 -  CG_Bridges
 -  CG_Culverts
 -  CG_StormBasins
 -  CG_StormCasings
 -  CG_StormCleanOut
 -  CG_StormControlValve
 -  CG_StormFitting
 -  CG_StormGravityMain
 -  CG_StormInlets
 -  CG_StormManhole
 -  CG_StormNetworkStructure
 -  CG_StormOpenDrain
 -  CG_StormOutlets
 -  CG_StormPressurePipe
 -  CG_StormSystemValve
 -  CG_StormVirtualDrainline
 -  CG_StormWeirStructure

ATTACHMENT B – TABLE 2A ADDITIONAL DETAIL

Municipal Infrastructure Dataset Review Table

| Dataset/Parameter | Note | Yes/No | Notes/Assumptions |
|----------------------------------|------|--------|---|
| Pipe | | | Overall Storm Sewer File (polyline shapefile or layer file) |
| Pipe Global ID | | | If no, can be assigned |
| Length | 1 | | If no, can be spatially calculated |
| Shape | | | Can be assumed as circular. |
| Diameter (Size) | 1 | | Can be assigned. |
| Width | | | Only used for non-circular pipes |
| Material | | | Parameter used to derive pipe roughness |
| Upstream Invert | 2 | | If no, can be derived from connected structure invert elevation |
| Downstream Invert | 2 | | If no, can be derived from connected structure invert elevation |
| Slope | | | If no, can be derived from US Inv., DS Inv., Length |
| Upstream Structure | | | If no, can be derived from Manhole/Catch Basin files |
| Downstream Structure | | | If no, can be derived from Manhole/Catch Basin files |
| Culvert | | | Culvert File (polyline shapefile or layer file) |
| Included in Overall Pipe File | | | Are culverts included with overall storm sewer file? |
| Pipe Global ID | | | If no, can be assigned |
| Length | 1 | | If no, can be spatially calculated |
| Shape | | | Can be assumed as circular. |
| Diameter | 1 | | Can be assigned. |
| Width | | | Only used for non-circular culverts |
| Material | | | Parameter used to derive culvert roughness |
| Upstream Invert | | | If no, can be derived from DEM surface |
| Downstream Invert | | | If no, can be derived from DEM surface |
| Slope | | | If no, can be derived from US Inv., DS Inv., Length |
| Manhole | 1 | | Overall Structure File (point shapefile or layer file) |
| Structure Global ID | | | If no, can be assigned |
| Rim Elevation | | | If no, can be derived from DEM surface |
| Invert Elevation | 2 | | If no, can be derived from connected pipe invert elevation |
| Catch Basin | 1 | | Overall Catch Basin File (point shapefile or layer file) |
| Structure Global ID | | | If no, can be assigned |
| Included in Overall Manhole File | | | Are catch basins included in overall manhole file? |
| Rim Elevation | | | If no, can be derived from DEM surface |
| Invert Elevation | 2 | | If no, can be derived from connected pipe invert elevation |
| Grate Length | | | Can be assumed. |
| Grate Width | | | Can be assumed. |
| Combination Style | | | Can be assigned. |

1. Items listed with 1 in the note column are the minimal basic data preferred to build a functional model. While a model can be built without these data by making assumptions, the reliability of the model may be significantly reduced. Manhole-catch basin refers to the structure being an inlet or not.
2. Items with a 2 in the note column are also beneficial for basic model build, although are more easily assumed and still resulting in a fairly reliable 2D model.

2D Pilot Model Build Automated Script Design Report

Prepared by:
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Prepared for:



MINNEHAHA CREEK
WATERSHED DISTRICT
QUALITY OF WATER, QUALITY OF LIFE

Date:
January 2023

Kimley»»Horn

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Document History

| Date | Version | Description of Change |
|----------------|---------|---|
| April 2022 | 1.0 | Initial version started to kick off design. |
| September 2022 | 2.0 | Additional revisions following comments and script development. |
| November 2022 | 2.1 | Updated figure formats and addition of new Figure 2 (Data Workflow) |
| January 2023 | 3.0 | Addition of Raw to MGIS automated field mapping script |

Definitions

- API – Application Programming Interface is a way to interface from one system to another
- ArcGIS – a desktop and cloud solution provided by Esri for GIS analysis, storage, data management, and data processing
- Automation – any process that is modified through scripting
- File Storage – cloud or physical file storage location utilized as a central repository for raw, input, and output datasets
- Geodatabase (GDB) – GIS file and data format that allows for standardization and template creation
- GIS – Geographic Information System
- GIS Process – queries and tools built into the Esri ArcGIS environment used to convert and transpose data. These scripts are executed directly within the Esri ArcGIS environment
- Graphical User Interface (GUI) – a system (or single set) of interactive visual windows that allow the user to input data, read output messages, and perform the functions of a tool
- ICM – InfoWorks ICM software package developed by Innowyze to perform 1D and 2D hydrologic/hydraulic simulation modeling
- ICPR – Interconnected Channel and Pond Routing (ICPR) software package version 4 developed by Streamline Technologies to perform 1D and 2D hydrologic/hydraulic simulation modeling
- IDLE – Integrated Development Environment
- MetroGIS (MGIS) – a GIS format designed for use by Twin Cities Metropolitan-area municipalities for the standardization of infrastructure data
- Microsoft SQL Server – relational database system that supports data processing, data management, and data analytics
- Python Script – software programs written in the Python programming language
- Toolbox – includes a single or multiple individual tools, scripts, or manual processing steps to be run in conjunction to create and format data for use in the MGIS database and model creation

1 INTRODUCTION

Major technological advancements have taken place since the District last built its watershed-wide model in the early 2000's. These advancements in both computing power and availability of high-resolution spatial datasets now make it possible to build and operate a high-resolution, large-scale model. However, maintaining such a detailed, large-scale model hinges on the premise that repeatable automated workflows can be established to process existing infrastructure and geospatial raw datasets into a model-ready dataset. The District pursued the pilot model build, in part, to help mitigate for and better understand this technical risk.

This memorandum provides an overview of the automated system that was developed for the 2D Pilot Model Build. This overview will include a summary of the overarching IT elements, a high-level design of the three key workflows, and a detailed breakdown of the individual scripts created to support the project workflow.

2 SYSTEM AND SOFTWARE DESIGN

The first step of developing a useable and maintainable information technology (IT) process is developing a system infrastructure plan and data processing strategy to ensure that the system can be properly maintained after it has been built. The two components of this planning process include:

- The information technology (IT) ecosystem that will house the data transformation process.
- The generalized workflow for data processing, model development, and model storage.

The section includes:

- A description of the software and hardware components needed to deliver the solution.
- Details of the data flows from third-party systems or hardware, and software components.

2.1 IT System Overview

The system overview provides descriptions of the IT ecosystem (i.e., software and hardware components) used in the implementation of the modeling and scripting process. This description includes definitions and detailed breakdown of each user, software components, and data flow within MCWD’s data processing and modeling system. Figure 1 provides a context diagram of the system that illustrates the overarching location, development process, stakeholders, data locations, and flow of modeling from raw data to model development, to scenario analysis, to external projects and analysis.

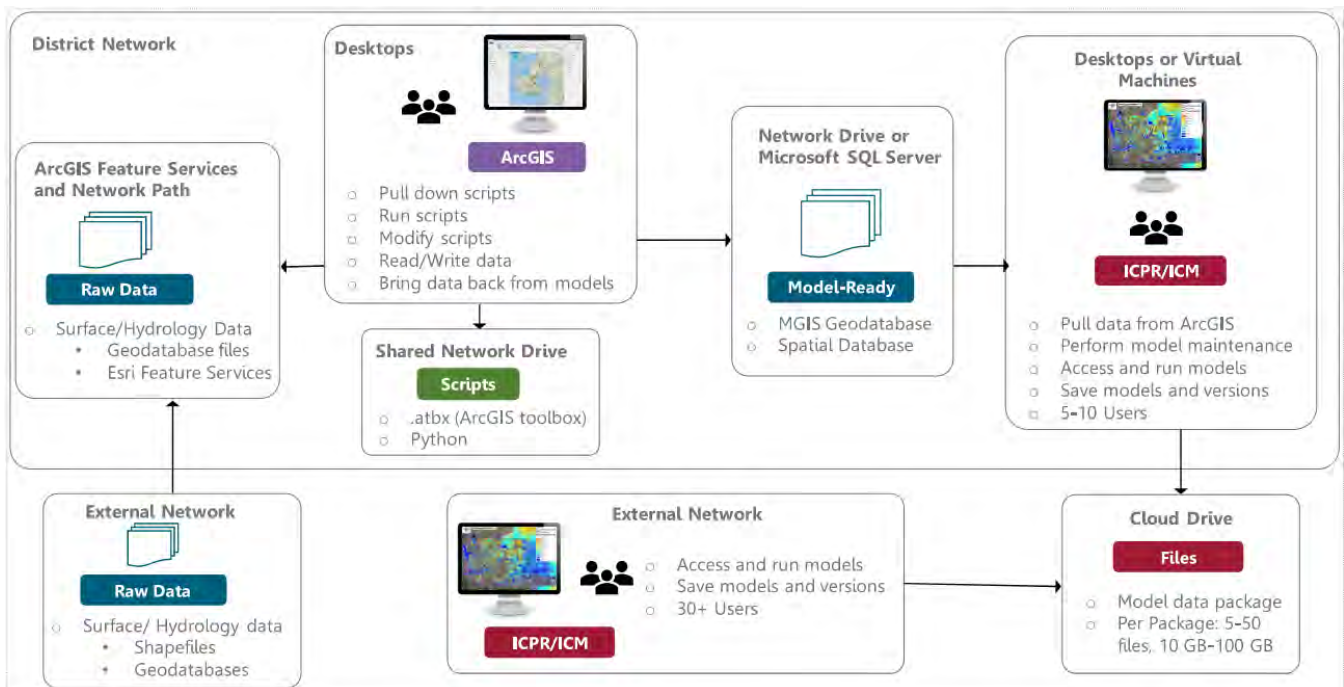


Figure 1. 2D Pilot Model Build System Context

- **Users**

- **MCWD/Consultant on behalf of MCWD**

- ArcGIS Users
 - Run scripts to move from pre-processed data into model-ready files
 - Modify scripts to meet new needs, or make other improvements
 - Bring data back from the modelers to keep GIS data up to date
 - Place raw surface and hydrology data from stakeholders in the District Esri instance. This may be used for scripting in the 2D modeling effort where access will be available through the Esri feature services.
- ICPR/ICM Modelers
 - Pull post-processed data from ArcGIS for use in the models
 - Perform model maintenance
 - Access and run models to perform scenario analysis
 - Save models and scenarios into different versions
 - Push model updates to a cloud accessible drive for access by external stakeholders

- **External Stakeholders**

- Local Geospatial Data Authority
 - Provide non-publicly available GIS files for MCWD GIS staff. These will be provided in Geodatabase format primarily, though it may also include access to Esri feature services and raw shapefiles.
 - *In the future, the District would benefit from a regional GIS dataset in Esri Online or agency provided cloud Esri instance.*
- ICPR/ICM Modelers
 - Access models in a cloud drive and download to perform scenario analysis.
 - Push updates to the cloud drive following scenario analysis.

- **Data Formats**

Figure 2 the general workflow of data from the raw available data to data that is ready for import into each modeling platform. Datasets for each step in the process are defined below.

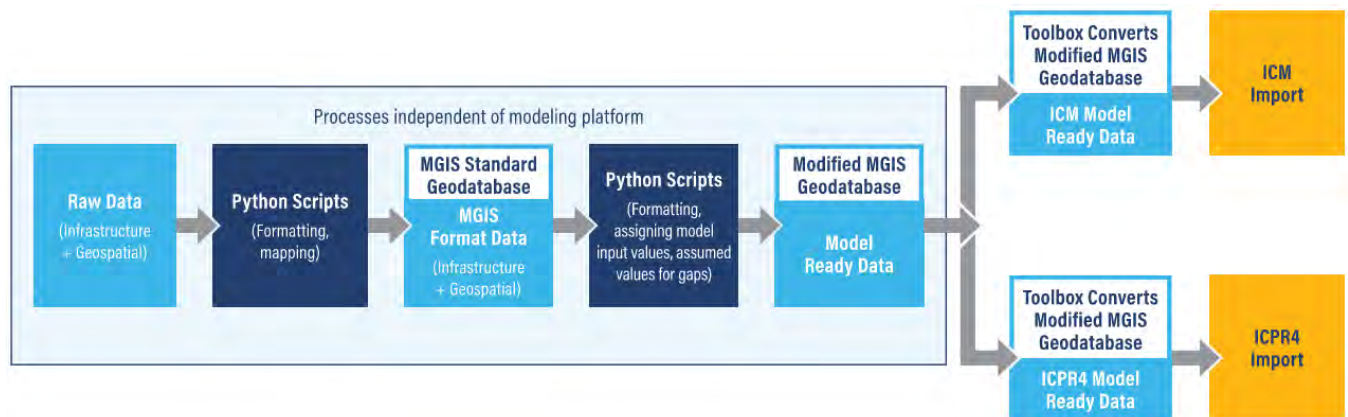


Figure 2. Datasets and Workflow Process to Produce Model Ready Data

- **Raw Input Files** – the agency provided raw data will need to be converted and processed into the MGIS standard geodatabase. These raw datasets are typically provided in Geodatabase format, although may be provided in a shapefile or with access through an Esri feature service for download to the District’s network.
- **MGIS Standard Geodatabase** – Populated with raw data files that have been transformed to standard attribute fields. Consolidation of multiple input fields to a single field occur during the transformation. A MGIS database will be created for each agency. Multiple versions of the MGIS database can also be created. A watershed-wide database could be developed along with multiple smaller subwatershed (or municipal) databases.
- **Modified MGIS Geodatabase (GWIS/ICM)** – Datasets for model creation. These datasets include interpolated, assumed, and calculated data for missing data and model parameter input. Model parameter inputs include loss coefficients, roughness coefficients, inlet capacities, etc.
- **Output Files** – Result files incorporating data such as inundation depths and locations, overland flow velocities and directions, pipe flow conditions, and groundwater saturation levels. Other datasets may be extracted from the modeling software for additional comparison of results. These output files will range in size depending on level-of-detail in the underlying model, simulation length and complexity, and output scale. The output packages may range in size between a single gigabyte (GB) to over 100 GB per scenario. These files are contained in a single file structure which may be neatly pushed to the Cloud storage for archive and access from external stakeholders.

3 HIGH LEVEL DESIGN

This section provides a high-level overview of the process that was designed to convert raw datasets into cleaned and processed model-ready data. 2D models reference three primary spatial data types, that include:

- Surface Data: Landuse and LiDAR (DEM)
- Sub-Surface Data: Soils
- Infrastructure: Stormwater Pipes, Inlets, and Manholes

These datasets are referenced in two ways to create model input parameters. Parameters are either directly sourced from a dataset/field (ex: Surface DEM) while others are a derived parameter that references two or more datasets/fields (ex: Green-Ampt Parameters). Figure 3 shows how each dataset supports the required model input parameters. This high-level overview walks through the key workflow processes and shows how each dataset supports the overall model build requirements.

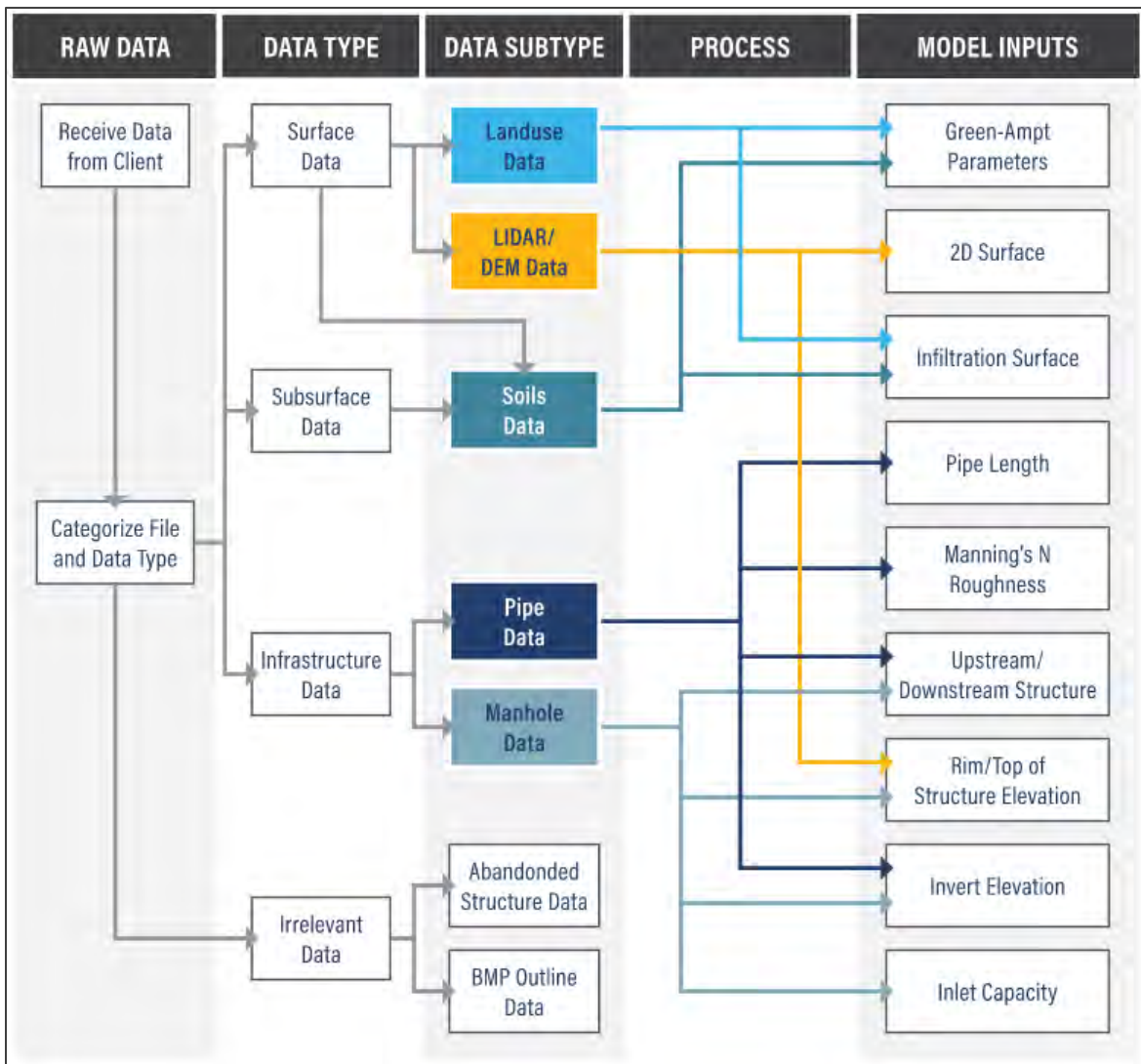


Figure 3. Overall Data Conversion Process Flow Diagram

To generate the model input parameters, each dataset needs to be processed into a standard format and cleaned to address formatting and data gap issues. While each dataset requires varying degrees of formatting, they generally flow through a similar pathway. Figure 3 shows at the highest level how datasets flow through the automated processing steps, become model-ready to be directly imported into each modeling software.

- Raw to MGIS: Infrastructure data is received from a source (municipality) and is transferred to the MGIS standard GDB;
- Spatial Data: Spatial data is received or developed and is utilized to create spatial model input layers;
- MGIS to Model Input Dataset: MGIS infrastructure dataset utilized to calculate model input parameters, spatial model input layers utilized to fill infrastructure data gaps.

3.1 Raw to MGIS Standard Flow

Raw stormwater infrastructure data from several sources (individual project, local, regional, and state municipalities) are needed to develop a comprehensive model-ready infrastructure dataset. It is understood that, currently, each raw dataset may be stored in different data formats and sub-formats for individual attribute fields, as well as varying degrees of accuracy and completeness. A key step in the overall process is standardizing each dataset into an established standardized schema. The standardization allows for reduced user input and complexity throughout the remaining inputs and processes to model-ready dataset.

This step in the process does not generate any new data, simply mapping data that exists into the standard. Each municipal dataset will need to be reviewed to correctly map the raw input fields to the corresponding MGIS standard attribute field. The full data mapping tables are shown in Appendix A of this memo. Multiple raw data fields may be used for a single MGIS field and some raw data fields may not be utilized in the MGIS standard. The most common fields for raw inputs are invert elevation, rim elevation, pipe diameter, and notes/comments. The Python script was modified to allow for the user to specify a pre-made csv file that contains the raw data field names and the corresponding MGIS output field. Mapping of multiple raw fields allows for partial datasets to be utilized and mapped into the MGIS field. Partial datasets may be contained within the raw dataset due to multiple surveys, joined datasets, user inputs, or spatial tools. In addition to mapping, mild formatting may need to take place to standardize how a field's data is populated. For example, the same pipe material might be written in three different ways or date fields may be empty and require a null data value at a minimum. It is important to standardize the attribute data so it can be referenced properly in subsequent automated steps.

An important aspect to this workflow is that the raw data field mapping will need to be performed for each individual infrastructure dataset. This is due to the varying attribute naming conventions, number of multiple raw fields, or separated input datasets. Meaning, as the District scales watershed-wide, a thorough understanding of each source's data will be required to correctly map and format into the MGIS Standard. Reviewing and understanding the Draft Stormwater Geodata Transfer Standard document and required inputs and outputs is key to successful long-term implementation of the scripts and processes developed in this memo. The Draft Stormwater Geodata Transfer Standard is included in Appendix B of this memo.

3.2 Spatial Data Flow

The raw spatial data is supplied through publicly available sources, municipal dataset, or project-specific creation. Elevation data is available through the MnTOPO download portal or through the USGS download portal. The elevation data may be located within a compressed format for ease of data transfer.

Multiple sources of current landuse/land cover (LULC) datasets at varying levels of detail are publicly available. These sources include the National Land Cover Dataset, the Twin Cities Metro Area (TCMA) landcover dataset. Landuse/land cover datasets are commonly developed by municipalities for current and future planning efforts. Landuse/land cover datasets can also be created by a user to reflect a future condition to be used in scenario analysis. The TCMA dataset was used as a basis for the LULC automated process development. User-created LULC dataset that reflect future scenarios can be utilized but are required to follow the standard SCS LULC codes (e.g., 100 = Agriculture, 151 = Industrial, etc.) to be utilized with the automated process.

The soil datasets (Hennepin and Carver counties) were downloaded from the Natural Resources Conservation Service (NRCS) Web Soil Survey (WSS). The county-wide WSS datasets were downloaded to encompass the entirety of the District. Multiple soil data tables must be extracted from the soil database for each county to calculate the representative values for the Green-Ampt Parameters. Additionally, the NRCS WSS dataset can be supplemented through site-specific soil borings, previous soil investigations, and historical data to reflect in-place soil conditions. Separate GIS processes for Hennepin and Carver counties were developed to reduce the amount of required user inputs and increase efficiency.

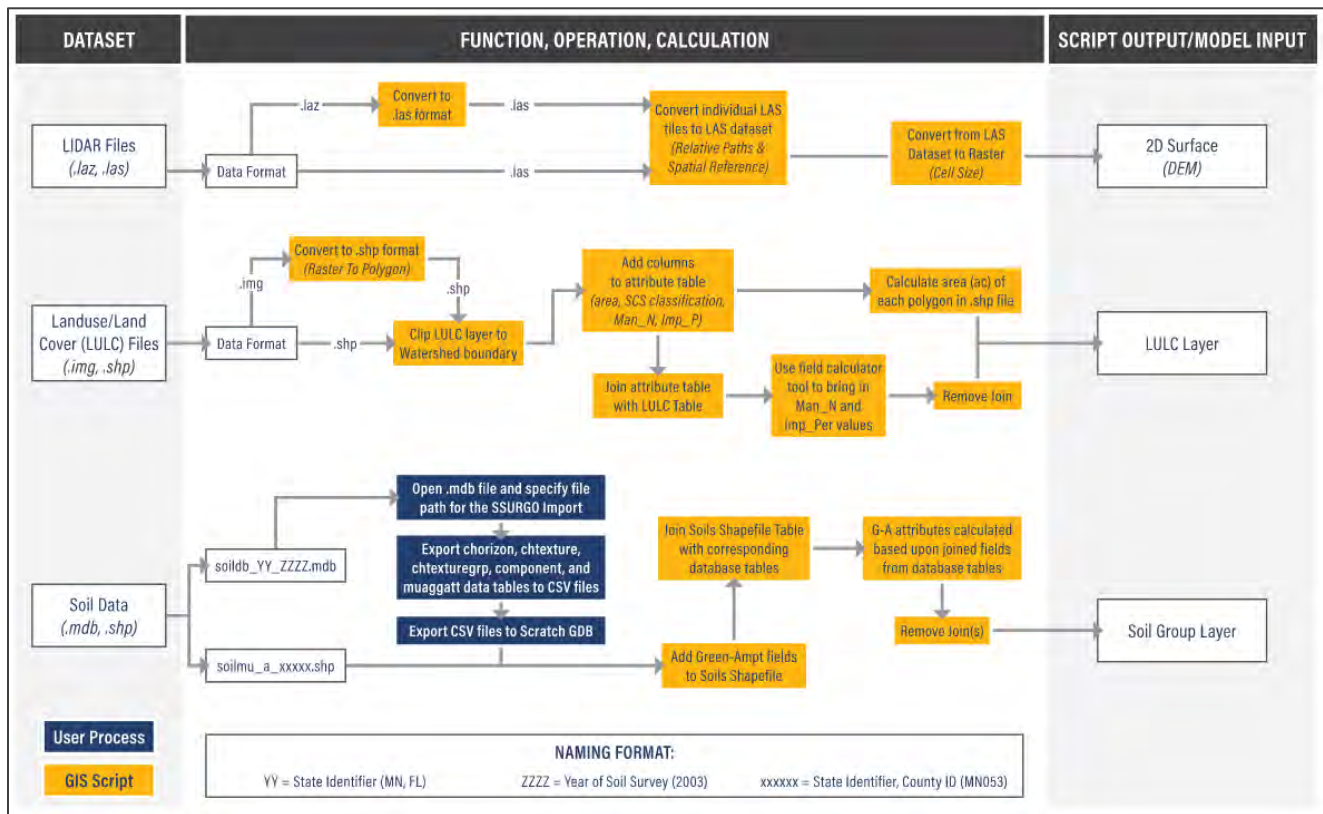


Figure 4. Spatial Data Conversion Process Diagram

Figure 4 lays out the general workflow and required steps to process the spatial data (elevation, landuse, and soil) from raw files to the model-ready datasets. These steps will be the same between the ICPR and ICM model development pathways.

The processes to produce the model-ready datasets for the Soil and LULC data layers can be completed once for the entire watershed then clipped down to the required model area. The processes can also be rerun as new data is collected or becomes available. The same technique can be completed for the surface (DEM) layer and clipped down to the model area. This may produce a very large raster file depending on the user-defined cell spacing. It is recommended to produce a new DEM for each model area or group of model areas to reduce the size of storage required.

3.3 MGIS to Model Input Dataset Flow

Once the raw infrastructure has been standardized and surface data has been processed, data gaps can be addressed, and model parameter generation can occur. This workflow includes multiple scripts/toolboxes and is centered around creating a clean, complete infrastructure dataset and generating required model input parameters.

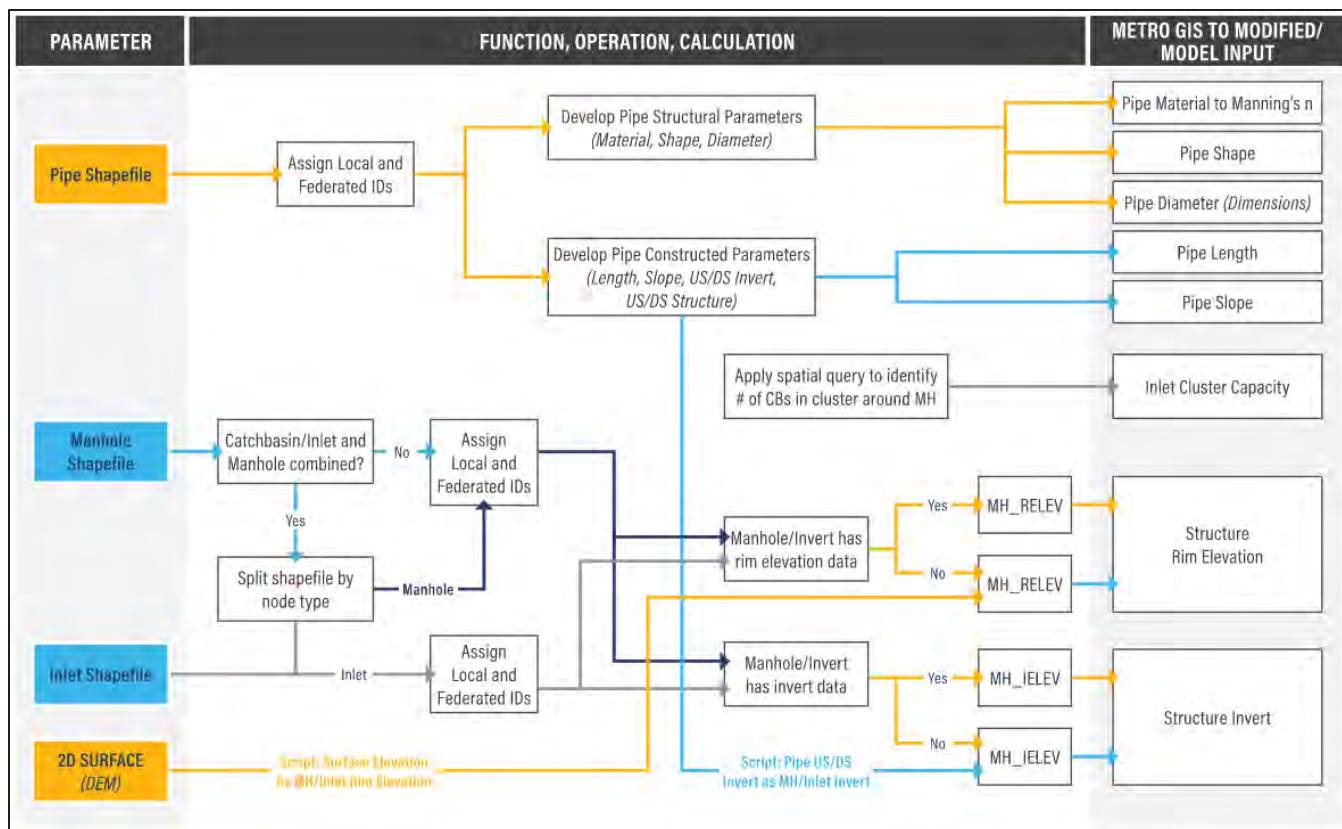


Figure 5. Infrastructure Data Gap Filling and Model Parameter Process Diagram

The first step is to evaluate the completeness of the infrastructure data. Upon review of the raw infrastructure datasets within the pilot geographies, missing data was identified such as manhole invert data, roughness type data, and others. To fill infrastructure data gaps, three methods were utilized:

- Reference spatial datasets to correct elevation issues
- Reference downstream/upstream pipe segments to populate pipe data
- Utilize engineering best practices/standards to fill remaining gaps

Additional user review and updates to assumed standard values may be required during the model build process to accurately represent in-place conditions. User inputs may be necessary to specify the desired model parameter value.

Figure 5 outlines the processes needed for the development and the overall flow of individual data subtypes to model inputs. Figure 5 is further detailed in Section 3.4 for additional MGIS data attributes that are not shown in Figure 5. The infrastructure data transformation and data filling steps will be the same between the ICPR and ICM model development pathways.

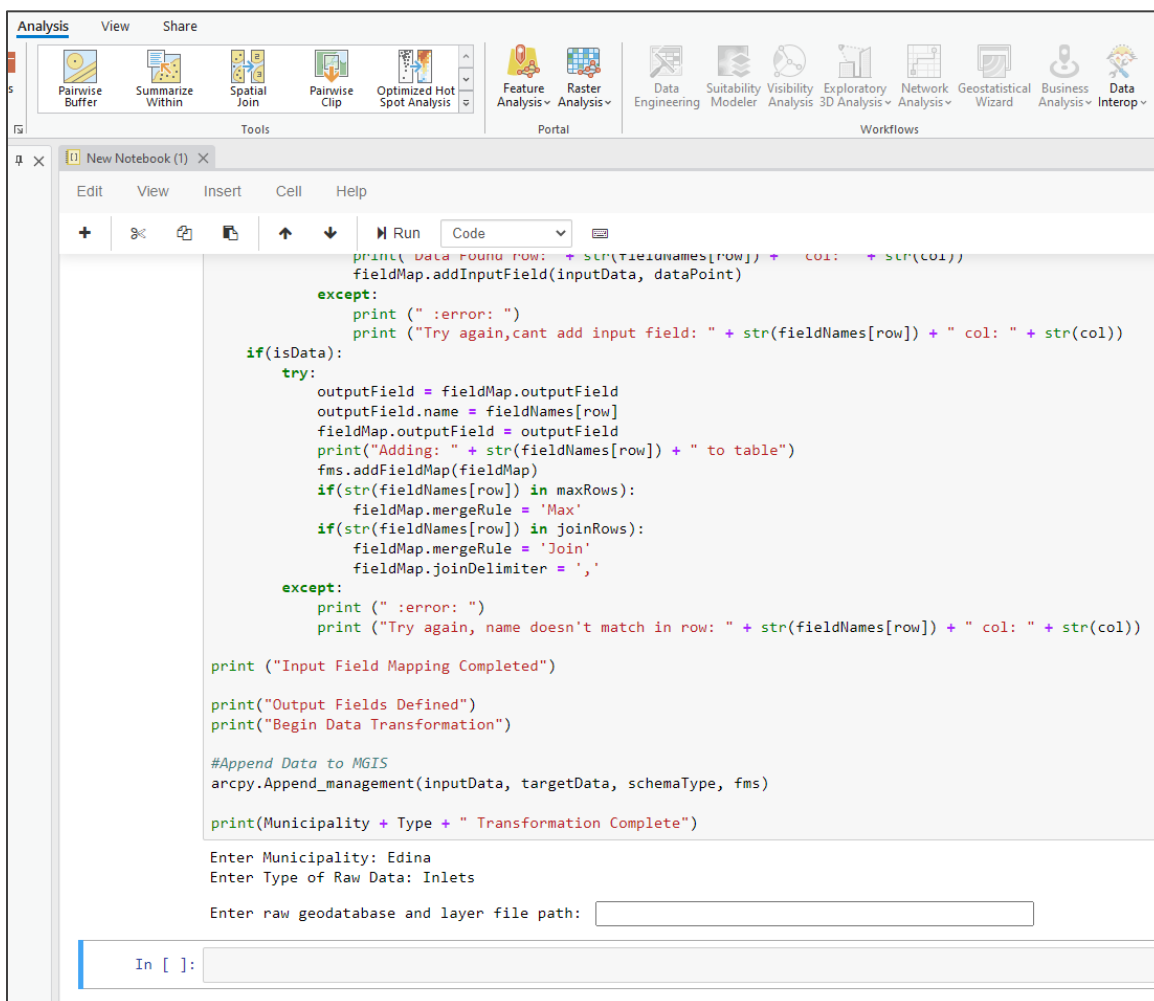
4 DETAILED DESIGN

The detailed design section includes user interface snapshots, manual processes, and pseudocode for automated processes. The overall process hinges on the consistent attribute naming of transformed data to develop model parameters. The raw attribute data mapping is an important step to successfully convert data from raw to MGIS. This step acts to ensure data types, formats, and naming conventions are documented so that the scripting process and data conversion steps are easily repeatable. The Raw Data Mapping tables for the Edina and Turbid-Lundsten Subwatersheds are shown in **Appendix A**.

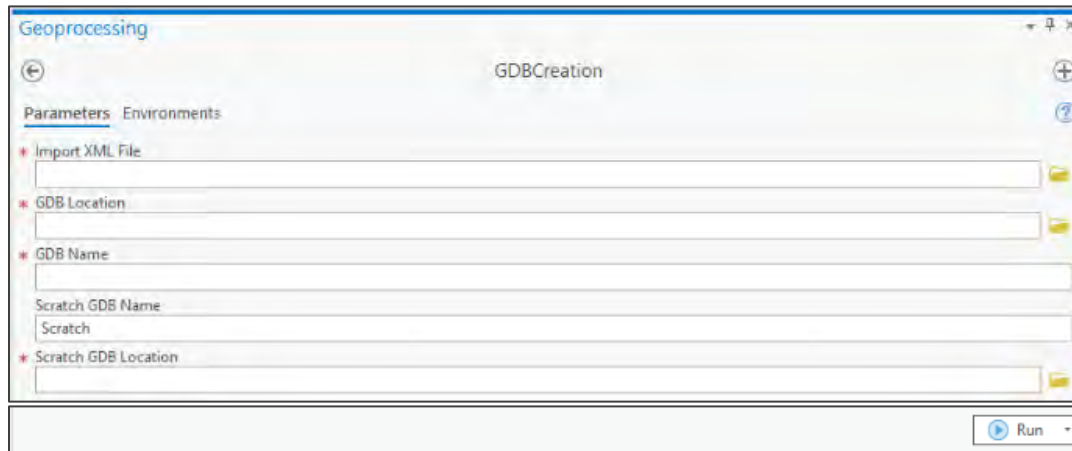
4.1 Graphical User Interface

There are two main GUIs that are utilized through the automated process are the Python IDLE window and the ArcPro Geoprocessing window.

- Python ArcPro GUI – this is the Python interface that comes standard with ESRI ArcGIS. This can be used to execute the Python scripts to transform the data from raw attributes to the MGIS standard directly within ArcPro. Alternative IDLE packages may be utilized to perform the Python scripts outside of ArcPro as the GIS users processing skills and abilities allow. A new Python window is opened within an ArcPro project, python code inserted, and the tool run. User inputs are requested along the bottom of the notebook with additional runtime messages that populate as well to log the progress of the tool.



- **ArcPro Geoprocessing** – This GUI is located within ArcPro and is accessed as any standard GIS tool would be accessed. The interface allows for user input in the form of folder and file specification. Some tools allow for additional user inputs such as cell spacing, search radius, and others. The GIS Processes may also be viewed through Model Builder within ArcPro depending on the GIS user processing skills and abilities. Model Builder allows the user to specify the inputs for each process independently and change additional parameters that are not shown in the Geoprocessing window.



4.2 Manual Process

The Web Soil Survey data for determining the Green-Ampt parameters must be manually extracted from the Microsoft Access Database file for each county. This process must be completed prior to executing process 4d. The steps to extract the files are listed below.

1. Open Microsoft Access Database file
2. Select the chtexture table and right-click
 - a. Select the Export, Excel option
 - b. Browse to desired output location
 - c. Check “Export data with formatting and layout” box
 - d. Check “Open the destination file after the export operation is complete” box
3. Once exported excel file opens, perform Save As to XLS file type
4. Repeat steps 2-3 for chtexturegrp, component, muaggatt, and chorizon database files

4.3 Pseudocode (Python Script and GIS Process)

1. **Project Setup (GIS Process)**

a. **Folder Creation**

This tool creates a standardized folder structure for a new project. The user specifies the desired location of the project folder and the project folder name. The folder structure includes folders for documents (DOCS), calculations (ENG), GIS files (GIS), ICM and ICPR model files (Models), and reference data (xIN).

i. Pseudocode

1. Create overall project folder [\[Create Folder\]](#)
 - a. User Specified Location
2. Create main subfolders [\[Create Folder\]](#)

| Folder | Subfolder | Sub-Subfolder |
|----------------------------------|---------------------|---------------|
| DOCS (Documentation and Reports) | Working | |
| | Final Report | |
| ENG (Engineering Calculations) | Hydrologic Folder | |
| | Results Comparison | |
| GIS (Files, Raw & MGIS) | MGIS | |
| | Model Inputs | Raster |
| | | Shapefiles |
| Models | ICPR | |
| | ICM | |
| xIN | Infrastructure Data | |
| | Spatial Data | LULC |
| | | DEM |
| | | Soils |
| | | Groundwater |
| | | Boundary |

b. GDB Creation – MGIS and Scratch

This tool creates an empty standard MGIS geodatabase (GDB) and a scratch GDB. The scratch geodatabase is utilized to perform manipulations and calculations of the raw data. Some GIS tools work best when files are located within a geodatabase, instead of a shapefile format. The user specifies the import XML file (MGIS), output MGIS GDB location and name, and the scratch GDB location and name. The XML file holds the MGIS format to reproduce the standard MGIS GDB.

i. Pseudocode

1. Specify MGIS import XML file [Import XML Workspace Document]
2. Specify MGIS GDB location
3. Specify name of MGIS GDB [Create File Geodatabase]
4. Specify scratch GDB location
5. Specify name of scratch GDB [Create File Geodatabase]

- a. Recommendation: Same name as MGIS GDB with _Scratch at end

c. GDB Creation – MGIS

This tool performs the same function as the previous tool but only creates the MGIS GDB.

i. Pseudocode

1. Specify MGIS XML file [Import XML Workspace Document]
2. Specify MGIS GDB location
3. Specify name of MGIS GDB [Create File Geodatabase]

d. Data Transfer

This tool transfers the data from the raw format to the scratch GDB. The user specifies the input file(s) and the scratch GDB location and name. There are no data transformations or calculations that take place with this tool. Perform this tool on all raw shapefile data, not necessary for lidar datasets.

- i. Pseudocode
 1. Specify Input Feature(s)
 2. Specify target Scratch GDB [Feature Class to Geodatabase]
 3. Files transferred from xIN folder to scratch GDB for future processing

2. Raw to MGIS (Python Scripts)

a. Inlets / Manholes / Pipes

This tool takes the user-specified scratch GDB inlet (or manhole or pipes) layer and performs data mapping via user-created csv file with raw attribute field names to create the MGIS inlet (or manhole or pipes) layer.

- i. Pseudocode
 1. Specify location (Municipality) of raw input data
 2. Specify folder location of scratch GDB and input file (layer) name
 3. Specify output MGIS GDB location and corresponding layer (Inlets, Manhole, Pipes)
 4. Specify the corresponding field mapping csv file
 - a. The csv file should follow this format for the top row:
 - i. MGIS Field Name, Raw Data Field, Raw Data Field2, Raw Data Field3, Raw Data Field4
 - b. Fields with a singular raw field input should be placed in the Raw Data Field column. Additional raw field inputs should be placed across the row. A maximum of 4 raw field inputs can be specified for any MGIS output field.

b. Scratch Conversion

This tool takes raw dataset and transfers them into the scratch GDB. The tool also takes user input to specify where data is located to calculate municipal, county, and state codes and standard names. This data is used to develop the federated ID field within the MGIS GDB. This tool should be used with all infrastructure data.

- i. Pseudocode
 1. Specify folder of raw data to be transferred
 2. Specify scratch geodatabase
 3. Specify location (i.e., City) of origin of data
 4. Script creates CTU Code and ID, County Name and ID, State Code data fields [Add Field]
 5. Data fields filled with appropriate codes and IDs from user specification [Calculate Field]
 6. Data is transferred from raw format to layer format within geodatabase

c. Value Data Null Fill

This tool takes a user input file and field to fill with a value or null. This tool is utilized to fill columns in the raw datasets that need a null value for tools to run. Typically, at least null values are required for date fields to be processed into the MGIS GDB. Other processes may require or be requested by the user to perform the same type of field fill process.

3. Surface Generation (GIS Process)

a. *LAZ to LAS*

This tool iterates through a user specified folder, searches for any files in the folder that end with .laz and decompresses the files to the .las format for processing by the following tools. The user specifies the input folder, wildcard (.laz), output format (.las), and the output folder name for the final files, and the output LAS file(s) name. The output LAS file name contains the wildcard %Name%. This wildcard is used to specify the original file name will be utilized in creation of the new file name. Without the wildcard, the name of each file would need to be specified individually.

i. Pseudocode

1. Specify folder on of all LAZ files to be uncompressed to LAS format [Iterate Files]
2. Specify input wildcard (file extension)
 - a. .LAZ is the standard
3. Specify output format
 - a. .LAS is the standard
4. Specify output folder location
5. Specify output name for uncompressed (.LAS) files [laszip]
 - a. %Name%.las will utilize the original name of each file and append .las to the end. Keep this format unless change is required.

b. *Create LAS Dataset*

This tool creates a LAS dataset (LASD) collection within the GIS environment. The LASD allows for batch processing and manipulation of a group of LAS tiles. LAS tiles are generally limited to 250 MB per tile. The user specifies the input LAS files, the output spatial coordinate system, the initial LASD file location and name, the projected LASD file location and name. The tool runs through the processes twice, thus the “Create PRJ for LAS Files” input twice in the dialog box. The first instance should be set to All LAS Files and the second instance set to No LAS Files. This was done to correct a spatial projection error in the underlying MnTOPO data.

i. Pseudocode

1. Specify folder location where .las files are saved
2. Specify coordinate system
 - a. UTM Zone 15N is standard for region
 - b. Watershed spans multiple counties
3. Specify where initial LAS dataset file will be saved [Create LAS Dataset]
 - a. Typically, saved in same folder as .las files
4. Specify final LAS dataset file will be saved [Create LAS Dataset]
 - a. Same location as 3.a.
5. Final option must remain “No LAS Files” for Create PRJ for LAS files

c. *LASD to TIF*

This tool creates the geotiff (DEM) raster file from the LASD that was created previously. The user specifies the input LASD name and location, cell spacing (meters), class code (2), intermediate location, final raster file location, and the output spatial coordinate system. The class code of 2 filters the LASD to only use the

ground elevation points. The cell spacing is specified in meters due to the MnTOPO data setup. 1 foot = 0.3048 meter for conversion purposes.

i. Pseudocode

1. Specify the previously created final LAS dataset file to be used in DEM creation
2. Set cell spacing (in meters)
 - a. Smaller cell spacing = increased detail in DEM but larger output file
 - b. Default set to 2m, can be reduced or increased as needed
 - c. Z factor (height) of 3.28084 is applied automatically to transform elevation from meters to feet.
3. Class Code = 2 for only ground elevation points [[Make LAS Dataset Layer](#)]
4. Intermediate DEM Location [[LAS Dataset to Raster](#)]
 - a. Can be deleted after processing [[Delete Raster \(after completion of tool\)](#)]
5. Output DEM (raster) location
6. Output Coordinate System [[Project Raster](#)]
 - a. Can be projected to a different coordinate system

d. **ClipDEM**

This tool clips the created DEM to the input file limits. The user specifies the input raster (DEM), the output raster (final DEM) name, the output extent (clipping feature). The “Use Input Features for Clipping Geometry” check box should be checked. If the box is unchecked, then the input raster will be clipped to the minimum bounding rectangle surrounding the output extent.

i. Pseudocode

1. Specify Input Raster to be clipped
2. Specify clipping boundary shapefile
 - a. Must be in same spatial projection as input raster (UTM Zone 15N)
3. Input Distance for Buffer [[Buffer](#)]
 - a. Applies a buffer to the clipping boundary
4. Check Use Input Feature for Clipping Geometry, if user wants DEM boundary to follow input file exactly [[Clip Raster](#)]
 - a. Unchecked uses a minimum bounding rectangle of the input file
5. Specify output raster name and location

4. **Spatial Tools (GIS Process)**

a. **File Reproject**

This tool reprojects an input dataset from one spatial projection to the standard spatial projection for the project. The tool also checks the input dataset for the current spatial projection, if the spatial projection matches the standard spatial projection, then the tool does nothing.

i. Pseudocode

1. Specify input shapefile for spatial transformation
2. Specify output file folder location

3. User specifies output file name [Project]
 - a. Recommended to include new spatial projection (UTM) in file name
4. Raw shapefile current spatial projection displayed on screen for reference

b. Input File Clip Model

This tool takes an input dataset and clipping feature to produce a clipped version of the input dataset. This tool also buffers the clip by 500 feet. The buffer reduces the need to rerun tools for model build, in the case that a model area boundary is adjusted slightly, or input data stretches across the desired model boundary.

- i. Pseudocode
 1. Specify input file or dataset
 2. Specify clipping boundary [Clip Features]
 3. Specify distance for buffer [Buffer]
 - a. Default = 500 feet
 4. Specify output file path and name

c. WSS Data – Add Fields

This tool adds the required Green-Ampt parameter fields to the specified table. The G-A parameter inputs are not consistent between modeling software packages. ICPR takes the underlying soil parameters and generates runoff values internally, ICM takes preprocessed soil parameters. The underlying soil parameters are found in the web soil survey (WSS) database and the preprocessed soil parameters are based on a look-up table specified by the user.

- i. Pseudocode
 1. Specify raw Soils shapefile name
 2. All Green-Ampt (G-A) parameter fields added to attribute table [Add Field]
 - a. Fields that are added include:

| Field Alias | Field Name | Type |
|----------------------------------|------------|--------|
| Component Key | CompKey | Text |
| Percent Clay – Representative | pClay | Double |
| Percent Sand – Representative | pSand | Double |
| Percent Organic – Representative | pOrganic | Double |
| CHorizon Key | ChorzKey | Text |
| Bulk Density 1/3 Bar | BD_13bar | Double |
| K saturated | Ksat | Double |
| Moisture Content 1/3 Bar | MC_13bar | Double |
| Moisture Content 15 Bar | MC_15bar | Double |
| Initial Water Table | WT_Int | Double |
| Soil Type | Soil_Type | Text |
| Suction | Suction | Double |
| Hydraulic Conductivity | Hyd_Cond | Double |
| Porosity | Porosity | Double |

d. WSS Data – Carver County & Hennepin County

i. Pseudocode

1. Load database tables into ArcPro for faster processing
2. Specify input shapefile from previous step
3. Specify exported WSS tables
 - a. Component Table
 - b. Chorizon Table
 - c. Muaggatt Table
 - d. Chorizon Texture Table
4. Join raw soils shapefile to WSS lookup tables [[Add Join](#)]
5. Verify individual field names (Names must follow !FieldName! format or syntax error will occur (*field names specified in background*))

| | |
|---------------------------------------|--------------------------|
| Total Clay – Representative | 1/3 bar Bulk Density |
| Total Sand – Representative | 1/3 bar Moisture Content |
| Total Organic Matter – Representative | 15 bar Moisture Content |
| Ksat | Soil Type |
| Water Table - Initial | |

6. Join G-A parameter lookup table [[Add Join](#)]
7. Calculate G-A parameter fields from lookup table [[Calculate Field](#)]
 - a. Suction
 - b. Hydraulic Conductivity
 - c. Porosity
8. Remove table joins [[Remove Join](#)]

e. WSS Data Fill

This tool fills the empty soil parameters with a standard clay value. The soil type field is set as aClay for assumed clay soil type. All soil parameters match standard clay values. Urban soils are not given soil parameters within the WSS system. These soils are assumed to be compacted due to grading and development activities and act as clay soils in relation to hydrologic factors.

i. Pseudocode

1. Specify soil shapefile
2. Shapes that include Soil Types equal to Zero or Null are selected [[Select Layer by Attribute](#)]
3. Assume that areas with no Soil Type are Clay
 - a. Soil Type name computed to aClay [[Field Calculator](#)]
 - b. Compute remaining Green-Ampt parameters in accordance with standard Clay values [[Field Calculator](#)]
4. Selection of Null shapes removed [[Remove Selection](#)]

f. LULC Imp Mann

This tool uses a lookup table and landuse layer to specify impervious and roughness coefficients based upon land cover. The user specifies the land use layer, lookup table, lookup table field, corresponding shapefile field, and the output folder. The lookup table and landuse layer must have a column that match to transfer the data from one to the other.

i. Pseudocode

1. Specify landuse shapefile
2. Impervious attribute field created [[Add Field](#)]
3. Manning's n attribute field created [[Add Field](#)]
4. Specify Manning's n/Impervious lookup table file path
5. Specify Landuse ID attribute shapefile for use with lookup table
6. Specify corresponding Landuse ID lookup table attribute field [[Join Table](#)]
7. Tool calculates the related Manning's n and Impervious values based on the input landuse shapefile landcover attribute field [[Field Calculator](#)]
8. Specify output file location [[Feature Class to Shapefile](#)]

5. Model Parameters (GIS Process)

a. Drop Null Segments

This tool uses the MGIS pipe layer to remove null (zero length) pipe segments from the input pipe file. Zero length shapes can occur during digitization of the raw dataset and may cause issues during model creation and simulation analysis. The tool also calculates the length of all pipes and checks against a user specified field. All lengths with a variance greater than 20 feet are flagged for review by the user.

i. Pseudocode

1. Calculate all Link lengths [[Calculate Geometry](#)]
2. Select Links with null length and assign them the calculated length [[Select by Attribute and Calculate Field](#)]
3. Compare Calculated link length vs recorded link length [[Calculate Field](#)]
4. Tag with a note if greater than 20% off [[Select by Attribute & Calculate Field](#)]
5. For flagged pipes, fill in length to match measured length [[Calculate Field](#)]
6. Select and delete pipes less than 0.1' [[Select by Attribute & Delete Features](#)]
7. Add a name field based on the FacilityID field [[Calculate Field](#)]
8. End with 'Length' field, 'Name' field, and 'Comment' field calling out 20% variance

b. Node Data Merge

This tool merges the inlets and manhole layers to create a single node layer for model creation. The layer maintains all of the individual data for each layer type.

i. Pseudocode

1. Compile all nodes into a single shapefile [[Merge](#)]
2. End with a shapefile containing all node features

c. Assign Nodes

This tool utilizes spatial selection to name upstream and downstream nodes for pipes based upon a search radius specified by the user. The tool utilizes the previously merged manhole and inlet layer file to perform the analysis.

i. Pseudocode

1. Pull first (last for downstream nodes) vertex of pipe [[Feature Vertices to Points](#)]
2. Spatial join first vertex (last for downstream nodes) to nearest node respectively [[Spatial Join](#)]
3. Calculate name of the nearest node into the first (last for downstream nodes) vertex of pipe table [[Calculate Field](#)]
4. Join first (last for downstream nodes) vertex node table back to the working link table and assign the calculated name to a temporary US name [[Join](#)]
5. Repeat process for DS node (see above)
6. Tag with a note if the US or DS nodes are missing [[Select by Attribute](#) and [Calculate Field](#)]
7. End with 'Node_From' and 'Node_To' with missing nodes flagged

d. Assign Roughness

This tool calculates pipe roughness coefficients based upon the pipe material field in the MGIS pipe layer. The tool uses a pipe lookup table to reference the corresponding roughness values to each material.

i. Pseudocode

1. Recalculate all blank and null material types with 'Unknown' [[Select by Attribute](#) and [Calculate Field](#)]
2. Join known Manning's "n" table to pipe material field [[Join](#)]
3. End with 'UsManningsN' and 'DsManningsN'

e. Clean Depth

This tool takes a user input to insert assumed pipe sizes for pipes that have null diameters. The default assumed pipe size is set to 2 feet (24 inches).

i. Pseudocode

1. Create field 'UsMaxDepth' and set it equal to the PIPESIZE_I field converted to feet [[Calculate Field](#)]
2. Select all 0 depth pipes and assign them a user input assumed value [[Calculate Field](#)]
3. Comment on all assumed depths [[Calculate Field](#)]
4. Assign 'DsMaxDepth' field the same values as the 'UsMaxDepth' [[Calculate Field](#)]
5. End with 'UsMaxDepth' and 'DsMaxDepth' fields, comment on assumed depths

f. Fill in Null Invert with DEM Offset

This tool updates pipes with missing invert values based upon a user specified pipe cover value. The default pipe cover value is 1 foot. The diameter of the pipe is added to the specified cover value to calculate the assumed pipe inverts.

i. Pseudocode

1. Pull first (last for downstream invert) vertex of pipe [[Feature Vertices to Points](#)]

2. Pull elevation data from desired surface [[Add Surface Information](#)]
3. Subtract a desired cover and the existing pipe depth from the elevation [[Calculate Field](#)]
4. Join new elevations to the pipe shapefile [[Join Field](#)]
5. Tag with a note if inverts are missing [[Select by Attribute](#) and [Delete Features](#)]
6. Replace missing elevations with the calculated DEM depth [[Calculate Field](#)]
7. Repeat process for downstream inverts (*see above*)
8. End with 'UsInvert' and 'DsInvert' field

g. Find Bends within Pipes

This tool performs spatial analysis of the pipes layer to determine the presence of bends/blind junctions within individual pipe segments. Any segments with bends are specified to have an internal loss coefficient of 0.5. The bend location is assumed to be in the middle of the pipe and input into the attribute table for the pipe layer.

i. Pseudocode

1. Calculate the central point and centroid x- and y-coordinates for all pipes [[Calculate Geometry Attributes](#)]
2. Select all pipes where central point and centroid do not match for either the x- or y-coordinates [[Select by Attribute](#)]
3. Add the field 'BendLossCoef' and assign a value of 0.5 [[Calculate Field](#)]
4. Add the field 'BendLocation' and assign a value of 0.5 [[Calculate Field](#)]

APPENDIX A DETAILED INPUT DATA MAPPING STANDARDS

EDINA

Manhole Dataset: DManhole – 9,070 shapes

| Input Raw Attribute Name | Description | Data Type | Output MGIS Standard Attribute Name |
|--------------------------|--|-----------|-------------------------------------|
| FacilityID | Edina Manhole ID Name-Number | Text | MH_ORID |
| AncillaryRole | | Short | <i>not utilized</i> |
| Enabled | | Short | <i>not utilized</i> |
| SubType | Structure Subtype Number ID | | <i>not utilized</i> |
| AccessDiameter | Manhole Access Diameter | | <i>not utilized</i> |
| AccessType | Access Type and Material | Text | <i>not utilized</i> |
| Depth | Structure Height | Double | MH_HT |
| InteriorDrop | Inlet to Outlet Pipe Vertical Drop, <i>No Data</i> | | <i>not utilized</i> |
| BarrelMaterial | Structure Barrel Material #5 of 5 | Text | MH_CMNT2 |
| StepMaterial | Structure Step Material | Text | <i>not utilized</i> |
| BarrelDiameter | Structure Diameter | Double | MH_WID, MH_LNG |
| BenchMaterial | Structure Bench Material | Text | <i>not utilized</i> |
| ChannelMaterial | Structure Channel Material | Text | <i>not utilized</i> |
| RingMaterial | Structure Ring Material, #3 of 5 | Text | MH_CMNT2 |
| AccessMaterial | Structure Access Material, #2 of 5 | Text | MH_CMNT2 |
| FrameMaterial | Structure Frame Material, #1 of 5 | Text | MH_CMNT2 |
| ConeMaterial | Structure Cone Material, #4 of 5 | Text | MH_CMNT2 |
| BaseMaterial | Structure Base Material | Text | <i>not utilized</i> |
| MH_ID | Old Manhole ID | Long | <i>not utilized</i> |
| MH_B_OTTOM | Invert Elevation, #1 of 2, Main Dataset, 2,132 entries | Double | MH_IELEV |
| GROUND_EL | Rim Elevation, #1 of 2, Main Dataset, 2,024 entries | Double | MH_RELEV |
| MH_BOTTOM | Invert Elevation #2 of 2, Partial Dataset, 32 entries | Double | MH_IELEV |
| GROUN_EL_D | Rim Elevation #2 of 2, Partial Dataset, 397 entries | Double | MH_RELEV |
| MH_TXT | Previous Manhole ID, #1 of 2 | Text | MH_CMNT |
| SOURCETHM | As-Built Information Source | Text | MH_ABDQC |
| SUMP | Presence of a Sump in Structure | Text | <i>not utilized</i> |
| SUMP_INV | Sump Elevation | Double | MH_SUMP |
| YEAR_INST | Year Installed | Text | <i>not utilized</i> |
| RECON_YR | Year Reconstructed | Text | <i>not utilized</i> |
| ASB_NUM | As-Built Number | Text | <i>not utilized</i> |
| Condition | Condition Rating | Text | <i>not utilized</i> |
| ConditionDate | Condition Date | Date | MH_CDATE |
| SubTypeMH | Manhole Type | Text | MH_CMNT |
| AccessLength | Manhole Access Opening Length | Double | <i>not utilized</i> |
| AccessWidth | Manhole Access Opening Width | Double | <i>not utilized</i> |
| SPCD | SPCD | Text | <i>not utilized</i> |
| Verified | Structure Data Quality Level | Text | <i>not utilized</i> |
| created_user | Manhole Data Created By | Text | MH_DASRC |
| created_date | Manhole Data Created Date | Date | <i>not utilized</i> |
| last_edited_user | Last Edited By | Text | MH_DATAN |
| last_edited_date | Last Edited Date | Date | MH_DAMOD |
| UnitCost | Unit Cost | Text | <i>not utilized</i> |
| ReplacementValue | Cost to replace structure | Double | <i>not utilized</i> |
| Owner | Owner of structure | Text | MH_OWNT, MH_MAINT |
| Notes | General notes on structure, #2 of 2 | Text | MH_CMNT |

Inlet Dataset: DCatchBasin – 3,588 shapes

| Input Raw Attribute Name | Description | Data Type | Output MGIS Standard Attribute Name |
|--------------------------|--|-----------|-------------------------------------|
| FacilityID | Edina Inlet ID Name-Number | Text | IN_ORID |
| AncillaryRole | <i>No Data</i> | Text | <i>not utilized</i> |
| AdministrativeArea | <i>No Data</i> | Text | <i>not utilized</i> |
| LegacyID | <i>No Data</i> | Text | <i>not utilized</i> |
| Location | <i>No Data</i> | Text | <i>not utilized</i> |
| OperationalArea | <i>No Data</i> | Text | <i>not utilized</i> |
| SubBasin | <i>No Data</i> | Text | <i>not utilized</i> |
| Rotation | <i>No Data</i> | Double | <i>not utilized</i> |
| LifeCycleStatus | Inlet Structure Status | Text | IN_STAT |
| SubType | <i>No Data</i> | Text | <i>not utilized</i> |
| WarrantyDate | <i>No Data</i> | Date | <i>not utilized</i> |
| InstallContractor | <i>No Data</i> | Text | <i>not utilized</i> |
| WaterType | <i>No Data</i> | Text | <i>not utilized</i> |
| Elevation | <i>No Data</i> | Double | <i>not utilized</i> |
| BelowGrade | Depth below surface, <i>No Data</i> | Double | <i>not utilized</i> |
| Manufacturer | Inlet Structure Manufacturer, <i>No Data</i> | Text | <i>not utilized</i> |
| Measurement1 | <i>No Data</i> | Double | <i>not utilized</i> |
| Measurement2 | <i>No Data</i> | Double | <i>not utilized</i> |
| Depth | <i>No Data</i> | Double | <i>not utilized</i> |
| ID | ID Number, <i>No Data</i> | Long | <i>not utilized</i> |
| MH_ID | Old Manhole ID numbers | Long | <i>not utilized</i> |
| NODE_TYPE | Inlet Type (CB, drain) | Text | IN_CMNT |
| PREFIX | Previous Compiled Node Type and MH ID | Text | <i>not utilized</i> |
| MH_B_OTTOM | Invert Elevation (#1 of 10) | Double | IN_IELEV |
| GROUND_EL | Rim Elevation (#1 of 2) | Double | IN_RELEV |
| MH_BOTTOM | Invert Elevation (#2 of 10) | Double | IN_IELEV |
| GROUN_EL_D | Rim Elevation (#2 of 2) | Double | IN_RELEV |
| MH_TXT | Old Manhole Structure Name | Text | IN_CMNT2 |
| DISTANCE2 | Height of Structure, Partial Dataset | Double | <i>not utilized</i> |
| COMMENTS2 | Connecting Pipe Notes | Text | IN_CMNT2 |
| ZOOY_SURCH | Node Surcharges in 100Y event | Double | <i>not utilized</i> |
| ZOY_SURCHA | Node Surcharges in 10Y event | Double | <i>not utilized</i> |
| ZY_SURCHAR | Node Surcharges in 1Y event | Double | <i>not utilized</i> |
| SOURCETHM | As-Built File name | Text | IN_ABDON |
| CREATED_BY | GIS feature created by | Text | IN_DATAT |
| RIM_VER | Rim Elevation Verified | Text | <i>not utilized</i> |
| TYPE_VER | Inlet Type Verified | Text | <i>not utilized</i> |
| TR | Top of Rim Elevation | Text | IN_CMNT |
| SUMP_INV | Sump Elevation | Double | IN_SUMP |
| YEAR_INST | Year Installed, <i>No Data</i> | Text | <i>not utilized</i> |
| RECON_YR | Reconstruction Year | Text | <i>not utilized</i> |
| ASB_NUM | As-Built Number, <i>No Data</i> | Text | <i>not utilized</i> |
| Condition | Inlet Condition, <i>No Data</i> | Text | <i>not utilized</i> |
| ConditionDate | Inlet Condition Date, <i>No Data</i> | Date | <i>not utilized</i> |
| INV_S | South Pipe Invert Elevation (#3 of 10) | Text | IN_IELEV |
| INV_N | North Pipe Invert Elevation (#4 of 10) | Text | IN_IELEV |
| INV_E | East Pipe Invert Elevation (#5 of 10) | Text | IN_IELEV |
| INV_W | West Pipe Invert Elevation (#6 of 10) | Text | IN_IELEV |
| INV_NW | North-West Pipe Invert Elevation (#7 of 10) | Text | IN_IELEV |
| INV_NE | North-East Pipe Invert Elevation (#8 of 10) | Text | IN_IELEV |
| INV_SE | South-East Pipe Invert Elevation (#9 of 10) | Text | IN_IELEV |
| INV_SW | South-West Pipe Invert Elevation (#10 of 10) | Text | IN_IELEV |
| created_user | Inlet Data Created By | Text | IN_DASRC |
| created_date | Inlet Data Created Date | Date | <i>not utilized</i> |
| last_edited_user | Last Edited By | Text | IN_DATAN |
| last_edited_date | Last Edited Date | Date | IN_DAMOD |
| InstallDate | Installation Date, <i>No Data</i> | Date | <i>not utilized</i> |

Pipe Dataset: DGravityMain – 9,831 shapes

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|--|-----------|------------------------------|
| FacilityID | Edina Pipe ID Name-Number | Text | PIPE_ORID |
| SubType | Pipe Type (DCollector) | Text | <i>not utilized</i> |
| RecordedLength | Pipe Length, Partial Dataset | Double | <i>not utilized</i> |
| Material | Pipe Material | Text | PIPE_MAT |
| UpstreamInvert | Pipe Upstream Invert | Double | PIPE_IELVU |
| DownstreamInvert | Pipe Downstream Invert | Double | PIPE_IELVD |
| Slope | Pipe Slope (%) | Double | PIPE_SLOPE |
| DepthUpstream | Depth to Pipe at Upstream End | Double | PIPE_DEP |
| DepthDownstream | Depth to Pipe at Downstream End | Double | <i>not utilized</i> |
| PIPE_ID | Old Pipe ID System | Long | <i>not utilized</i> |
| PIPE_LEN | Pipe Length | Double | PIPE_LNG |
| US_MH_ID | Upstream Manhole ID | Long | PIPE_FROM |
| DS_MH_ID | Downstream Manhole ID | Long | PIPE_TO |
| PIPETYPE | Pipe Material and Shape | Text | PIPE_CMNT |
| PIPESIZE | Pipe Diameter | Double | PIPE_DIA |
| DS_MH_TXT | Downstream Manhole ID – Old | Text | <i>not utilized</i> |
| US_MH_TXT | Upstream Manhole ID – Old | Text | <i>not utilized</i> |
| SOURCETHM | As-Built Document/File | Text | PIPEABDOC |
| LENGTH_FEE | Fee Length | Double | <i>not utilized</i> |
| CREATED_BY | Object Created By | Text | PIPE_DATAT |
| PIPESIZE_I | Pipe Diameter – Inches | Text | PIPE_CMNT |
| UPSTR_VER | Upstream Verified | Text | <i>not utilized</i> |
| DSTR_VER | Downstream Verified | Text | <i>not utilized</i> |
| PIPE_VER | Pipe Verified | Text | <i>not utilized</i> |
| RECON_YR | Reconstructed Year | Text | <i>not utilized</i> |
| Condition | <i>No Data</i> | Text | <i>not utilized</i> |
| ConditionDate | <i>No Data</i> | Date | <i>not utilized</i> |
| ASB_Path | As-Built Path | Text | <i>not utilized</i> |
| ASB_Folder | As-Built Folder | Text | <i>not utilized</i> |
| ASB_Num | As-Built File Number | Text | <i>not utilized</i> |
| Asbuilt | Compiled As-Built Path, Folder, File | Text | PIPEABLINK |
| created_user | GIS created by | Text | PIPE_DASRC |
| created_date | GIS created date | Date | <i>not utilized</i> |
| last_edited_user | Last Edited By | Text | PIPE_DATAN |
| last_edited_date | Last Edited Date | Date | PIPE_DAMOD |
| UnitCost | Unit Cost, <i>No Data</i> | Double | <i>not utilized</i> |
| ReplacementValue | Replacement Value, <i>No Data</i> | Double | <i>not utilized</i> |
| Owner | Owner of Pipe | Text | PIPE_OWNN |
| Notes | General Notes on Pipe Information | Text | PIPE_CMNT2 |
| LifeCycleStatus | Status of Pipe | Text | PIPE_STAT |
| Plansheet | <i>No Data</i> | Text | <i>not utilized</i> |
| OldASBNumber | Old As-Built Number and Data | Text | <i>not utilized</i> |
| LiningType | Lining Type | Text | PIPE_CMNT2 |
| InstallDate | Install Year | Text | <i>not utilized</i> |
| Old_ASB_Folder | Old As-Built Folder | Text | <i>not utilized</i> |
| Old_Asbuilts | Old Compiled As-Builts Folder, File Path | Text | <i>not utilized</i> |

TURBID

Manhole Dataset: StormManhole – 29 shapes

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|--|-----------|------------------------------|
| FACILITY ID | Carver County Manhole ID , Partial Dataset | Text | <i>not utilized</i> |
| INSTALLDAT | Installation Date | Date | MH_IDATE |
| HIGHELEV | Highest Elevation | Double | <i>not utilized</i> |
| INVERTELEV | Structure Invert Elevation | Double | MH_IELEV |
| INVERT | Invert Data, <i>No Data</i> | Double | <i>not utilized</i> |
| RIMELEV | Structure Rim Elevation, (#1 of 2) | Double | MH_RELEV |
| CVTYPE | Control Valve Type, <i>No Data</i> | Text | <i>not utilized</i> |
| WALLMAT | Structure Wall Material, <i>No Data</i> | Text | <i>not utilized</i> |
| MHTYPE | Manhole Type, Partial Dataset | Text | <i>not utilized</i> |
| CONDITION | Structure Condition, <i>No Data</i> | Text | <i>not utilized</i> |
| LOCDESC | General Location, <i>No Data</i> | Text | <i>not utilized</i> |
| CUTDEPTH | <i>No Data</i> | Double | <i>not utilized</i> |
| FLOWDIR | <i>No Data</i> | Text | <i>not utilized</i> |
| LINED | <i>No Data</i> | Text | <i>not utilized</i> |
| GPSDATE | <i>No Data</i> | Date | <i>not utilized</i> |
| ENABLED | Enabled (from model software) | Long | <i>not utilized</i> |
| ACTIVEFLAG | Active Flag (from model software) | Long | <i>not utilized</i> |
| OWNEDBY | Structure Owned By | Long | <i>not utilized</i> |
| MAINTBY | Structure Maintained By | Long | <i>not utilized</i> |
| SUMFLOW | <i>No Data</i> | Double | <i>not utilized</i> |
| LASTUPDATE | Data Last Updated Date | Date | MH_DAMOD |
| LASTEDITOR | Data Last Updated By | Text | MH_DATAN |
| PROJECT_NU | Project Number, <i>No Data</i> | Text | <i>not utilized</i> |
| PROJECT_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| PROJECT_2 | <i>No Data</i> | Text | <i>not utilized</i> |
| OWNERSHIP | <i>No Data</i> | Text | <i>not utilized</i> |
| CAST_TYPE | Casting Type | Text | <i>not utilized</i> |
| SUMP | <i>No Data</i> | Text | <i>not utilized</i> |
| SUMP_DEPTH | <i>No Data</i> | Double | <i>not utilized</i> |
| OVERFLOW | <i>No Data</i> | Text | <i>not utilized</i> |
| WATERSHED | <i>No Data</i> | Text | <i>not utilized</i> |
| NOTES | <i>No Data</i> | Text | <i>not utilized</i> |
| Receiving_ | <i>No Data</i> | Text | <i>not utilized</i> |
| created_us | Created By User | Text | <i>not utilized</i> |
| created_ds | Created Date | Date | <i>not utilized</i> |
| last_edite | Last Edited By | Text | <i>not utilized</i> |
| last_edi_1 | Last Edited Date | Date | <i>not utilized</i> |
| ID_prefix | Structure ID Prefix | Text | <i>not utilized</i> |
| MAPKEY | Map Key ID | Text | <i>not utilized</i> |
| STORM_STRU | Storm Structure ID | Text | <i>not utilized</i> |
| MS4_ID | <i>No Data</i> | Text | <i>not utilized</i> |
| TYPE | Structure Type | Text | <i>not utilized</i> |
| SIZE | Structure Diameter | Text | <i>not utilized</i> |
| RIM_ELEV | Structure Rim Elevation (#2 of 2) | Text | MH_RELEV |
| INVERT_ELE | Structure Invert Elevation (#2 of 2) | Text | MH_IELEV |
| PROJ_NUM | Project Number, Partial Dataset | Text | <i>not utilized</i> |
| SHEET | Plansheet Sheet Number | Text | <i>not utilized</i> |
| BLOCK | CAD Block Type | Text | <i>not utilized</i> |
| LAYER | CAD Layer | Text | <i>not utilized</i> |
| ANGLE | Rotation Angle | Double | <i>not utilized</i> |
| X_COORD | X Coordinate | Double | <i>not utilized</i> |
| Y_COORD | Y Coordinate | Double | <i>not utilized</i> |
| MATERIAL | Structure Material | Text | <i>not utilized</i> |
| LOCATION | General Location, Partial Dataset | Text | MH_LOC |
| INSTALL_DA | Installation Date, Partial Dataset | Text | <i>not utilized</i> |
| MS4 | <i>No Data</i> | Text | <i>not utilized</i> |
| MS4_SPCD_T | <i>No Data</i> | Text | <i>not utilized</i> |

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|---|-----------|------------------------------|
| MS4_STRUC | No Data | Text | not utilized |
| STRUCTURAL | No Data | Text | not utilized |
| MS4_OUTFAL | No Data | Text | not utilized |
| LEVEL_OF_A | Level of Accuracy | Text | not utilized |
| OWNER | Structure Owner Type | Text | MH_OWNT |
| STATUS | Structure Status | Text | not utilized |
| COMMENTS | No Data | Text | not utilized |
| MH_NUM | No Data | Text | not utilized |
| ORIG_FID | No Data | Long | not utilized |
| INSTALL__1 | Install Date | Date | not utilized |
| City_ID | No Data | Text | not utilized |
| Plan_ID | No Data | Text | not utilized |
| BMP_Type | No Data | Text | not utilized |
| Diameter | No Data | Text | not utilized |
| Casting_Si | No Data | Text | not utilized |
| Casting_Ty | Casting Type (Number) | Text | not utilized |
| Cone_Top_S | No Data | Text | not utilized |
| Project | No Data | Double | not utilized |
| Rim_Elevat | No Data | Double | not utilized |
| Rim_Adjust | No Data | Text | not utilized |
| Consultant | No Data | Text | not utilized |
| Inlet_Outl | No Data | Text | not utilized |
| Pond_Basin | No Data | Text | not utilized |
| Original_C | No Data | Text | not utilized |
| House_Numb | No Data | Text | not utilized |
| Street_Nam | No Data | Text | not utilized |
| AVGACCURAC | No Data | Double | not utilized |
| WORSTACCUR | No Data | Double | not utilized |
| PCDEVICE | No Data | Text | not utilized |
| PCDID | No Data | Long | not utilized |
| ESRIGNSS_R | No Data | Text | not utilized |
| ESRIGNSS_H | No Data | Double | not utilized |
| ESRIGNSS_V | No Data | Double | not utilized |
| ESRIGNSS_L | No Data | Double | not utilized |
| ESRIGNSS_1 | No Data | Double | not utilized |
| ESRIGNSS_A | No Data | Double | not utilized |
| ESRIGNSS_P | No Data | Double | not utilized |
| ESRIGNSS_2 | No Data | Double | not utilized |
| ESRIGNSS_3 | No Data | Double | not utilized |
| ESRIGNSS_F | No Data | Long | not utilized |
| ESRIGNSS_C | No Data | Double | not utilized |
| ESRIGNSS_S | No Data | Long | not utilized |
| ESRIGNSS_N | No Data | Long | not utilized |
| ESRIGNSS_4 | No Data | Date | not utilized |
| ESRIGNSS_5 | No Data | Double | not utilized |
| ESRIGNSS_6 | No Data | Double | not utilized |
| ESRIGNSS_7 | No Data | Long | not utilized |
| ESRIGNSS_8 | No Data | Double | not utilized |
| CARTEID | Old Structure ID | Text | not utilized |
| TEMPID | Temporary Structure ID | Long | not utilized |
| X | Structure X Coordinate | Long | not utilized |
| Y | Structure Y Coordinate | Long | not utilized |
| CarverCo_I | Carver County Manhole ID, Partial Dataset | Text | not utilized |
| POINT_X | Structure X Coordinate | Double | not utilized |
| POINT_Y | Structure Y Coordinate | Double | not utilized |
| CountyMain | Does County Maintain? | Text | not utilized |
| ID | Carver County Manhole ID, Full Dataset | Text | MH_ORID |
| Installed | Installation Date | Date | not utilized |
| Replaced | Replace/Modification Date | Date | not utilized |
| Retired | Retired/Removal Date, No Data | Date | not utilized |
| RouteID | No Data | Text | not utilized |

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|-------------|-----------|------------------------------|
| Measure | No Data | Double | not utilized |
| RoadID | No Data | Text | not utilized |
| Estimated_ | No Data | Double | not utilized |
| VerticalDa | No Data | Text | not utilized |
| BMP | No Data | Text | not utilized |

Inlet Dataset – Turbid

StormInlets – 287 shapes

This dataset includes the inlets from the Laketown Township Storm Sewer map package. No inlets are located within the Turbid subwatershed area.

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|---|-----------|------------------------------|
| FACILITYID | Carver County Inlet ID, Partial Dataset | Text | <i>not utilized</i> |
| INSTALLDAT | Installation Date, <i>No Data</i> | Date | <i>not utilized</i> |
| INLETTYE | Inlet Type | Text | <i>not utilized</i> |
| ACCESSDIAM | Access Diameter, <i>No Data</i> | Double | <i>not utilized</i> |
| INVERTELEV | Invert Elevation, Partial Dataset | Double | IN_IJEV |
| ACCESSMAT | Access Material, <i>No Data</i> | Text | <i>not utilized</i> |
| ACCESSTYPE | Access Type, <i>No Data</i> | Text | <i>not utilized</i> |
| ENABLED | Enabled (from model software) | Long | <i>not utilized</i> |
| ACTIVEFLAG | Active Flag (from model software) | Long | <i>not utilized</i> |
| OWNEDBY | Structure Owned By | Long | IN_OWNN |
| MAINTBY | Structure Maintained By | Long | IN_MAINN |
| LASTUPDATE | Data Last Updated Date | Date | IN_DAMOD |
| LASTEDITOR | Data Last Updated By | Text | IN_DATAN |
| AncillaryR | <i>No Data</i> | Long | <i>not utilized</i> |
| CASTTYPE | Structure Casting Type, <i>No Data</i> | Text | <i>not utilized</i> |
| TOPCAST | Structure Rim Elevation, Partial Dataset, (#1 of 2) | Double | IN_RELEV |
| PROJECT_NU | Project Number, <i>No Data</i> | Text | <i>not utilized</i> |
| PROJECT_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| PROJECT_2 | <i>No Data</i> | Text | <i>not utilized</i> |
| OVERFLOW | <i>No Data</i> | Text | <i>not utilized</i> |
| WATERSHED | <i>No Data</i> | Text | <i>not utilized</i> |
| SUMP | <i>No Data</i> | Text | <i>not utilized</i> |
| OWNERSHIP | <i>No Data</i> | Text | <i>not utilized</i> |
| Receiving_ | <i>No Data</i> | Text | <i>not utilized</i> |
| created_us | Created By User | Text | <i>not utilized</i> |
| created_da | Created Date | Date | <i>not utilized</i> |
| last_edite | Last Edited By | Text | <i>not utilized</i> |
| last_edi_1 | Last Edited Date | Date | <i>not utilized</i> |
| GPSDATE | <i>No Data</i> | Date | <i>not utilized</i> |
| AVGACCURAC | <i>No Data</i> | Double | <i>not utilized</i> |
| WORSTACCUR | <i>No Data</i> | Double | <i>not utilized</i> |
| LOCDESC | <i>No Data</i> | Text | <i>not utilized</i> |
| CULV_NUM | <i>No Data</i> | Text | <i>not utilized</i> |
| ORIG_FID | <i>No Data</i> | Long | <i>not utilized</i> |
| CARTEID | Carver County Inlet ID, Partial Dataset | Text | <i>not utilized</i> |
| TEMPID | Carver County Inlet ID, Partial Dataset | Long | <i>not utilized</i> |
| QUADRANT | Location Quadrant, Partial Dataset | Text | <i>not utilized</i> |
| PCDDVICE | <i>No Data</i> | Text | <i>not utilized</i> |
| PCDID | <i>No Data</i> | Long | <i>not utilized</i> |
| RIMELEV | <i>No Data</i> | Double | <i>not utilized</i> |
| MAPKEY | Map Key ID | Text | <i>not utilized</i> |
| City_ID | City Structure ID, Partial Dataset | Text | IN_CMNT |
| Plan_ID | <i>No Data</i> | Text | <i>not utilized</i> |
| MS4 | <i>No Data</i> | Text | <i>not utilized</i> |
| BMP_Type | <i>No Data</i> | Text | <i>not utilized</i> |
| MS4_Creati | <i>No Data</i> | Text | <i>not utilized</i> |
| Type_ | <i>No Data</i> | Text | <i>not utilized</i> |
| Material | <i>No Data</i> | Text | <i>not utilized</i> |
| Diameter | <i>No Data</i> | Text | <i>not utilized</i> |
| Casting_Si | <i>No Data</i> | Text | <i>not utilized</i> |

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|---|-----------|------------------------------|
| Casting_Ty | No Data | Text | not utilized |
| Cone_Top_S | No Data | Text | not utilized |
| Project | No Data | Double | not utilized |
| Layer | No Data | Text | not utilized |
| Rim_Elevat | Inlet Rim Elevation, Partial Dataset, (#2 of 2) | Double | IN_RELEV |
| Rim_Adjust | No Data | Text | not utilized |
| Consultant | No Data | Text | not utilized |
| Status | No Data | Text | not utilized |
| Owner | Inlet Owner | Text | IN_OWNN |
| House_Numb | No Data | Text | not utilized |
| Street_Nam | No Data | Text | not utilized |
| STORM_STRU | No Data | Text | not utilized |
| MS4_ID | No Data | Text | not utilized |
| RIM_ELEV | No Data | Text | not utilized |
| PROJ_NUM | No Data | Text | not utilized |
| X_COORD | No Data | Double | not utilized |
| Y_COORD | No Data | Double | not utilized |
| LOCATION | No Data | Text | not utilized |
| LEVEL_OF_A | No Data | Text | not utilized |
| COMMENTS | No Data | Text | not utilized |
| INSTALL_DA | No Data | Date | not utilized |
| X | X Coordinate of Point | Long | not utilized |
| Y | Y Coordinate of Point | Long | not utilized |
| CarverCo_I | Inlet Structure ID, full dataset | Text | IN_ORID |
| POINT_X | X Coordinate of Point | Double | not utilized |
| POINT_Y | Y Coordinate of Point | Double | not utilized |
| CountyMain | Does County Maintain? | Text | not utilized |
| NeedsInspe | Inspection Required? | Text | not utilized |
| InspCommen | Inspection Comment, No Data | Text | not utilized |
| InsideMS4A | Is inlet located in MS4 area?, No data | Text | not utilized |
| Inspection | Inspection performed?, No Data | Text | not utilized |
| ID | Inlet Structure ID, full dataset | Text | not utilized |
| Installed | Date of Installation | Date | IN_IDATE |
| Replaced | Date of Replacement/Modification | Date | IN_MDATE |
| Retired | Date of Removal, No Data | Date | not utilized |
| RouteID | Route ID, No Data | Text | not utilized |
| Measure | No Data | Double | not utilized |
| RoadID | County Road ID | Text | IN_LOC |
| InletShape | No Data | Text | not utilized |
| GrateType | No Data | Text | not utilized |
| Manufactur | No Data | Text | not utilized |
| Length | Inlet Length, No Data | Double | not utilized |
| Width | Inlet Width, No Data | Double | not utilized |
| VerticalDa | Vertical Datum, No Data | Text | not utilized |
| Notes | No Data | Text | not utilized |
| HighPriori | No Data | Text | not utilized |
| Source | No Data | Text | not utilized |
| LI_Date | Inspection Date | Date | not utilized |
| LI_Weather | Inspection Weather | Text | not utilized |
| LI_DaysRai | Inspection Previous Days of Rain | Text | not utilized |
| LI_RainAmo | Inspection Previous Days Rain Depth | Text | not utilized |
| LI_Materia | Inlet Material | Text | IN_MAT |
| LI_Conditi | Inspection Inlet Condition Rating | Text | not utilized |
| LI_Sedimen | Inspection Sediment Found Rating | Text | not utilized |
| LI_Scour | Inspection Scour Found Rating | Text | not utilized |
| LI_Erosion | Inspection Erosion Found Rating | Text | not utilized |

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|--|-----------|------------------------------|
| LI_Sedim_1 | Inspection Sediment #1 | Text | <i>not utilized</i> |
| LI_Sedim_2 | Inspection Sediment Depth | Text | <i>not utilized</i> |
| LI_Dischar | Inspection Discharge Condition | Text | <i>not utilized</i> |
| LI_Standin | Inspection Standing Water Observed | Text | IN_HOLDS |
| LI_Clogged | Inspection Inlet Clogged | Text | <i>not utilized</i> |
| LI_Overloa | Inspection Inlet Overflowing | Text | <i>not utilized</i> |
| LI_Immedia | Inspection Immediate Maintenance | Text | <i>not utilized</i> |
| LI_Working | Inspection Inlet Functioning | Text | <i>not utilized</i> |
| LI_Illicit | Inspection Illicit Discharge Occurring | Text | <i>not utilized</i> |
| LI_ConcDet | Inspection Concrete Replacement | Text | <i>not utilized</i> |
| LI_Grout | Inspection Grout Replacement | Text | <i>not utilized</i> |
| LI_OdorSew | Inspection Odor Sewage | Text | <i>not utilized</i> |
| LI_OdorRan | Inspection Odor | Text | <i>not utilized</i> |
| LI_OdorPet | Inspection Odor Pet | Text | <i>not utilized</i> |
| LI_OdorSul | Inspection Odor Sulfur | Text | <i>not utilized</i> |
| LI_OdorOth | Inspection Odor Other | Text | <i>not utilized</i> |
| LI_AppearO | Inspection Appearance #1 | Text | <i>not utilized</i> |
| LI_AppearC | Inspection Appearance #2 | Text | <i>not utilized</i> |
| LI_AppearS | Inspection Appearance #3 | Text | <i>not utilized</i> |
| LI_Appea_1 | Inspection Appearance #4 | Text | <i>not utilized</i> |
| LI_Appea_2 | Inspection Appearance #5 | Text | <i>not utilized</i> |
| Estimated_ | <i>No Data</i> | Double | <i>not utilized</i> |
| SymbolRota | <i>No Data</i> | Long | <i>not utilized</i> |
| BMP | <i>No Data</i> | Text | <i>not utilized</i> |

Pipe Dataset #1 – Turbid

StormCulverts – 335 shapes

This dataset includes the culverts from the Laketown Township Storm Sewer map package.

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|--|-----------|------------------------------|
| Source | Data Source (GPS, Unknown) | Text | <i>not utilized</i> |
| Type | Pipe Shape | Text | PIPE_SHP |
| DiameterSi | Pipe Diameter (inches) | Text | PIPE_CMNT |
| Material | Pipe Material | Text | PIPE_MAT |
| Notes | Location Notes | Text | PIPE_CMNT |
| FlowDirect | Pipe flowing direction | Text | PIPE_CMNT |
| FacilityTy | Type of Crossing | Text | PIPE_CMNT |
| End1Direct | Pipe End #1 Direction | Text | PIPE_CMNT2 |
| End1Type | Pipe End #1 Type | Text | PIPE_CMNT2 |
| End2Direct | Pipe End #2 Direction | Text | PIPE_CMNT2 |
| End2Type | Pipe End #2 Type | Text | PIPE_CMNT2 |
| created_us | Created by user | Text | <i>not utilized</i> |
| created_da | Created date | Date | <i>not utilized</i> |
| last_edite | Shape last edited by | Text | PIPE_STAT |
| last_edi_1 | Date of Last Edit | Date | PIPE_SDATE |
| Installed | Installed Date | Date | PIPE_IDATE |
| Replaced | Replacement/Modification Date | Date | PIPE_MDATE |
| Retired | Date of Retirement/Removal, <i>No Data</i> | Date | <i>not utilized</i> |
| RoadID | County Road ID | Text | PIPE_RDID |
| RouteID | County Route ID, <i>No Data</i> | Text | <i>not utilized</i> |
| FromMeasur | <i>No Data</i> | Double | <i>not utilized</i> |
| ToMeasure | <i>No Data</i> | Double | <i>not utilized</i> |
| ID | Pipe ID | Text | PIPE_ORID |
| Diameter | Pipe Diameter, general | Long | PIPE_DIA |
| Pavement | Type of Pavement over Pipe | Text | PIPE_CVG |
| Inspection | <i>No Data</i> | Long | <i>not utilized</i> |
| Inspecti_1 | <i>No Data</i> | Date | <i>not utilized</i> |
| Street | County Road Name | Text | PIPE_LOC |
| Length | Pipe Length | Double | PIPE_LNG |
| LengthAccu | Accuracy of Pipe Length measurement | Text | <i>not utilized</i> |
| Slope | Pipe Slope | Double | PIPE_SLOPE |
| Manufactur | <i>No Data</i> | Text | <i>not utilized</i> |
| LiningMeth | Method of Lining Pipe | Text | <i>not utilized</i> |
| UpstreamIn | Upstream Invert Elevation | Double | PIPE_IELVU |
| Downstream | Downstream Invert Elevation | Double | PIPE_IELVD |
| VerticalDa | Vertical Datum, <i>No Data</i> | Text | PIPE_VDAT |
| LI_Date | <i>Unknown</i> | Date | <i>not utilized</i> |
| LI_Utility | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelC | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelA | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelE | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelS | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelM | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Barrel_1 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_BarrelR | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Barrel_2 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Dischar | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Percent | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_RateInv | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_RatePro | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_RateEmb | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Disch_1 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Perce_1 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_Ratel_1 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_RateP_1 | <i>Unknown</i> | Text | <i>not utilized</i> |
| LI_RateE_1 | <i>Unknown</i> | Text | <i>not utilized</i> |

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|-------------|-----------|------------------------------|
| Estimated_ | No Data | Double | not utilized |

Pipe Dataset #2– Turbid

pipes – 1 shape

This dataset is from the MnDOT database and includes a single feature.

| Raw Attribute Name | Description | Data Type | MGIS Standard Attribute Name |
|--------------------|---|-----------|------------------------------|
| HYD_PIPE_I | MnDOT Pipe ID Name-Number | Long | ORID |
| HYD_PIPE_N | Pipe ID | Text | <i>not utilized</i> |
| HYD_PIPE_S | Pipe Status | Text | PIPE_STAT |
| HYD_PIPE_C | Pipe Type | Text | <i>not utilized</i> |
| OWNER_NAME | Pipe Owner Name | Text | PIPE_OWNN |
| ROUTE_NAME | Road ID | Text | <i>not utilized</i> |
| PERPEN_OFF | Perpendicular Offset | Double | <i>not utilized</i> |
| OFFSET_FRO | Offset Distance | Double | <i>not utilized</i> |
| MMS_STATIO | <i>No Data</i> | Text | <i>not utilized</i> |
| LOCAL_NAME | <i>No Data</i> | Text | <i>not utilized</i> |
| MMS_ROADWA | Crossing Type (centerline) | Text | <i>not utilized</i> |
| HYD_PIPE_SH | Pipe Shape | Text | PIPE_SHP |
| HYD_MATERI | Pipe Material | Text | PIPE_MAT |
| HYD_CURR_W | Pipe Width | Double | PIPE_CMNT |
| HYD_CURR_H | Pipe Height | Double | <i>not utilized</i> |
| HYD_PIPE_1 | Pipe Shape | Text | <i>not utilized</i> |
| HYD_MATE_1 | Pipe Material | Text | <i>not utilized</i> |
| HYD_PIPE_W | Pipe Diameter | Double | PIPE_DIA |
| HYD_PIPE_H | Pipe Height | Double | PIPE_HT |
| HYD_PIPE_L | Pipe Length | Double | PIPE_LNG |
| HYD_UPSTRE | Upstream Size | Long | <i>not utilized</i> |
| HYD_PIPE_T | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_PIPE_2 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_PIPE_3 | Pipe Outfall Direction | Text | <i>not utilized</i> |
| HYD_PIPE_4 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_LINER_ | Pipe Liner | Long | <i>not utilized</i> |
| COMMENT_ST | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INV_IN | <i>No Data</i> | Text | <i>not utilized</i> |
| MMS_YEAR_T | <i>No Data</i> | Double | <i>not utilized</i> |
| MMS_YEAR_1 | <i>No Data</i> | Long | <i>not utilized</i> |
| HYD_REP_PR | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_REP_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_REP_NO | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_REG_NO | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_MS4_AR | Is the pipe located within an MS4 area? | Text | <i>not utilized</i> |
| HYD_OUTFAL | Is the pipe an outfall? | Text | <i>not utilized</i> |
| MMS_SP_NUM | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_YEAR_B | <i>No Data</i> | Long | <i>not utilized</i> |
| DATE_ACTIV | Date of Installation | Date | PIPE_IDATE |
| DATE_RETIR | <i>No Data</i> | Date | <i>not utilized</i> |
| MMS_JUR_OW | <i>No Data</i> | Text | <i>not utilized</i> |
| MMS_MAINT_ | Pipe Maintenance Name | Text | PIPE_MAINN |
| MMS_CONST_ | Pipe Construction Notes | Text | PIPE_CMNT2 |
| COUNTY_NAM | General Location | Text | PIPE_LOC |
| MMS_STATE_ | Ownership Name - Type | Text | PIPE_OWNT |
| MMS_JUR_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| MMS_AGREEM | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_UP_ELE | Upstream Pipe Invert Elevation | Double | PIPE_IELVU |
| HYD_DN_ELE | Downstream Pipe Invert Elevation | Double | PIPE_IELVD |
| HYD_LONGIT | Longitude Coordinate | Double | <i>not utilized</i> |
| HYD_LATITU | Latitude Coordinate | Double | <i>not utilized</i> |
| HYD_LONG_1 | Longitude Coordinate | Double | <i>not utilized</i> |
| HYD_LATI_1 | Latitude Coordinate | Double | <i>not utilized</i> |
| HYD_GEOM_L | Geometric Length | Double | <i>not utilized</i> |
| HYD_UP_V_A | <i>No Data</i> | Double | <i>not utilized</i> |
| HYD_UP_H_A | <i>No Data</i> | Double | <i>not utilized</i> |

| | | | |
|------------|----------------------------|--------|---------------------|
| HYD_UP_XY_ | Horizontal Accuracy | Text | <i>not utilized</i> |
| HYD_UP_E_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_DN_V_A | <i>No Data</i> | Double | <i>not utilized</i> |
| HYD_DN_H_A | <i>No Data</i> | Double | <i>not utilized</i> |
| HYD_DN_XY_ | Horizontal Accuracy | Text | <i>not utilized</i> |
| HYD_DN_E_1 | <i>No Data</i> | Text | <i>not utilized</i> |
| EXT_ASSET_ | Asset ID | Text | <i>not utilized</i> |
| MMS_OFFSET | <i>No Data</i> | Text | <i>not utilized</i> |
| MMS_TRAFFI | <i>No Data</i> | Text | <i>not utilized</i> |
| USER_UPDAT | Updated in GIS by | Text | <i>not utilized</i> |
| DATE_UPDAT | Date Updated in GIS | Date | <i>not utilized</i> |
| HYD_INSPEC | Inspection Number | Long | <i>not utilized</i> |
| HYD_INSP_T | Pipe Type | Text | <i>not utilized</i> |
| INSP_STATU | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_S | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_D | Pipe Inspection Date | Date | PIPE_SDATE |
| HYD_INSP_N | Inspector Name | Text | <i>not utilized</i> |
| HYD_INSP_C | Pipe Inspection Condition | Text | PIPE_COND |
| HYD_INSP_M | Inspection Method (Visual) | Text | <i>not utilized</i> |
| HYD_INSP_1 | Inspection Type | Text | <i>not utilized</i> |
| HYD_INSP_2 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_3 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_R | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_4 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_I | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_A | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_E | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_W | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_P | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_5 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_6 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_7 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_8 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_H | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_9 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_10 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_11 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INSP_J | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_12 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_13 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_14 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_15 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_16 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_17 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_18 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_19 | <i>No Data</i> | Text | <i>not utilized</i> |
| HYD_INS_20 | <i>No Data</i> | Text | <i>not utilized</i> |
| INSP_COMME | <i>No Data</i> | Text | <i>not utilized</i> |
| PERIODIC_M | <i>No Data</i> | Long | <i>not utilized</i> |
| INSP_USER_ | Inspector Name | Text | <i>not utilized</i> |
| INSP_DATE_ | Inspection/Condition Date | Date | PIPE_CDATE |
| CC_ROUTE_N | County/City Route Name | Text | PIPE_RDID |
| FROM_RP_OF | Horizontal Offset | Text | <i>not utilized</i> |
| HYD_INS_21 | <i>No Data</i> | Text | <i>not utilized</i> |
| CC_SPATI_1 | <i>No Data</i> | Double | <i>not utilized</i> |

**APPENDIX B. METROGIS STORMWATER GEODATA TRANSFER
STANDARD**



Public Comments Received: April 2020 – September 2021

Metro Stormwater Geodata Project

Draft Stormwater Geodata Transfer Standard

Introduction and context. On Friday, April 17, 2020, the first release of the Metro Stormwater Geodata Project (MSWGP) draft Stormwater Geodata Transfer Standard and accompanying materials was published out to the statewide stakeholder community. The publication of the first draft of the standard represented the result of two years of consistent, focus, creativity, attention to detail of the MSWGP Steering Team members.

This material was released to the public with the specific purpose of enabling stakeholders to review the material, assess its relationship and fitness for their stormwater GIS data needs, to test and review a set of sample data and to provide feedback, suggestions, revisions and improvements to the draft data standard for its on-going improvement.

The draft release included the following materials:

- *The draft Stormwater Geodata Transfer Standard (v. 0.5) in both Word and Excel Spreadsheet format;*
- *The draft Inlet, Outlet and Pond Inspection Schema (v. 0.2) in both Word and Excel Spreadsheet format;*
- *A spreadsheet listing the way the draft standard aligned with known asset management needs;*
- *A sample dataset of stormwater system data in the v. 0.5 format for reviewers to download and test;*

These materials were published from the MSWGP's page on the MetroGIS web site which is hosted and maintained by the Metropolitan Council, available here: <https://metrogis.org/projects/stormsewers.aspx>

Public Review Period 1 (April 2020 – December 2020). Once the initial draft standard was developed and available, it was published for a round of public/stakeholder review. The first review period was intended to be a fixed 90-day period (from April 17, 2020 through mid-July of 2020), however with the impact and changing priorities brought on by the COVID-19 outbreak, the formal public release period was extended out through December 31, 2020.

Public Review Period 2 (July 2021 – September 2021). Once comments were collected from the first public review period, the MSWGP Steering Team convened for an on-line meeting during March 2021 to review these and modify the standard to reflect them. This modified version of the standard was again published for a sixty (60) day review period from July through September 2021.

On-going public input. The MSWGP Steering Team will welcome and continue to accept, review, and document recommendations, input, suggestions, and improvements from the stakeholder community as the standard continues to evolve as it is hopefully utilized by the professional community.

Purpose of this document. This document is an organized collection of the comments received by the stakeholder community during the two public review periods on the draft stormwater standard material. The MSWGP Steering Team has used the comments received input to shape and improve the content and form of the draft standard. The Steering Team membership is grateful to the members of the professional community who took time to download and review the materials and to provide their comments, suggestions, feedback, insights and input. The next version of the standard will be better for their contributions.

Summary of the themes and concepts from the stakeholder input:

Recurring themes and concepts which emerged from the comments received include the following:

- *Addition of a glossary for clearer definitions of stormwater terminology;*
- *Additional of examples of fixtures and features to explain them to GIS professionals who are generally not stormwater experts;*

- *Addition of terms and expansion of values in the domains provided in the draft standard*
- *Future inclusion and integration and inclusion of agricultural drainage systems and data;*
- *Consideration of the ability to accommodate non-structural stormwater elements*
- *Strengthen the ability to accommodate asset management activity with GIS data;*
- *Concern for the costs of data development or transition to using a standard of this type;*

The following pages contain the comments received during the two public stakeholder review periods as conducted by the MSWGP Steering Team. These comments have already been incorporated into the current draft (v. 0.6) of the proposed standard.

Molly Churchich
Ramsey County Public Works

In the Draft Inspections Schemas v. 0.2 document on page 16:

You could ***better define outlet versus outfall.*** County outfalls are non-traditionally defined because outlet could leave a system but technically be defined as an outfall due to agreement ownership. Generally, Ramsey County owns the catch basin and leads of the storm sewer while the cities and township own the storm mains and manholes. Outfalls outside county right-of-way are the responsibility of the city and outfalls inside the county right-of-way are the responsibility of the county, unless explicitly stated in the agreement. Depending on the project, ponds and associated elements are assigned to different parties.

In the Draft Inspections Schemas v. 0.2 document on page 40:

Pond inspection does not have fields for ***capacity gauging and sediment sampling results.***

In the Draft Stormwater Geodata Standard, v.0.5 document on page 17:

Could there be ***multiple fields for pipe maintenance agreements?***

We often have multiple agreements for multi-partner projects.

In the Draft Stormwater Geodata Standard, v.0.5 document on page 25

Do people use the CTU ID TXT field? We've always identified the County Road Number associated with the road.

In the Draft Stormwater Geodata Standard, v.0.5 document on page 99

We won't use outlet tide chambers; this does not apply to our infrastructure.

In the Stormwater Geodata Standard v. 0.5 Domains:

Pipe diameters: are the units of ***pipe diameter in inches or feet?***

Currently, Ramsey County has the following storm sewer infrastructure inventoried:

INFILTRATION BASINS

| | | |
|--------|--|--------------------|
| Types: | Biofiltration basin | Filtration basin |
| | Filtration trench | Infiltration basin |
| | Infiltration trench | Other |
| | Permeable pavement | Stormwater reuse |
| | Tree trench (Subtypes: CCLRT Type 1; CCLRT Type 2; None) | |

TREE TRENCH

Types: (types and subtypes are linked in **INFILTRATION BASINS**)

OUTFALLS

| | | |
|--------|---------|---------|
| Types: | Pipe | Ditch |
| | Lake | Pond |
| | Wetland | Channel |
| | Curbcut | Culvert |
| | Other | |

STORM INLETS

| | | |
|--------|---------------------|---------|
| Types: | Catch basin | Manhole |
| | Catch basin manhole | |

SPECIAL STRUCTURES

| | | |
|--------|-----------------------|---|
| Types: | Access manhole | Berm |
| | Berm weir | Bit_channel |
| | Box culvert | Channel |
| | Control manhole | Dam |
| | Deep manhole | Diversion box |
| | Diversion manhole | Diversion MH (<i>duplicate of diversion manhole?</i>) |
| | Diversion weir | Drop inlet |
| | Drop structure | Energy dissipater |
| | First flush diversion | Flapgate |
| | Floatable skimmer | Flume |
| | Gabions | Headwall |
| | Inlet manhole | Junction manhole |
| | Keepfill line | Land bridge |
| | Lined channel | Lock_dam |
| | Manhole | Multi-outfall MH |
| | Ob_well | Other |
| | Outfall baffle | Outlet control |
| | Pump | Riprap still basin |
| | Riprap channel | Sediment sump |
| | Siphon | Splitter manhole |
| | Stabil_mat | Stilling well |
| | Sump | Timber weir |
| | Trash weir | Triangular weir |
| | Turtle barrier | Ultra urban |
| | Valve vault | Weir |

OUTLETS

| | | |
|--------|------------------|--------------------|
| Types: | Assess | Emergency overflow |
| | Primary | Secondary |
| | Compound | Concrete pipe |
| | Culvert | Horiz. Pipe |
| | Horizontal pipe | Lift station |
| | Pipe | Riprap berm |
| | Submerged outlet | Submerged pipe |
| | Trash rack | Vert pipe |
| | Vertical pipe | Weir |
| | Weir orifice | Weir_channel |

| | |
|-----------|-------------|
| Subtypes: | Berm |
| | Berm riprap |
| | Channel |
| | ? |

AERATORS LIFTSTATIONS PUMPS

| | | |
|--------|---------------|---------------|
| Types: | Aerator | Compressor |
| | Control panel | Keepfill pipe |
| | Lift station | Obs well |
| | Pump | Well |

We have some cleaning up to do of the locally stored data, but the intention is to get it all migrated to the network and available to others. The main constraint preventing this is time- it's incredibly time consuming to go through each of these features.

Where would Tree Trenches fall in the standard? We currently are symbolizing them with both a point and line feature. If they are incorporated in the **line feature of Pipes** in the standard, their subtype would be slotted, as this best describes their composition. But as Mike Goodnature pointed out, **they are technically a BMP**, so perhaps would be suited for the Best Management Practice category.

The problem with identifying them as a point feature, is that placing the point midline of the feature is deceiving. Some of these trenches exceed 500 feet and I want to make sure inspectors inspect the entire facility. I'm concerned about placing the Tree Trench inventory into a database, such as pipes, because it would get lost in the inspection schedule.

Pipes are not mandated to be inspected on any regularity but are required to be mapped. Tree trenches are required to be inspected annually per the MS4 permit guidelines.

Ramsey County are in the process of consolidating condition ratings for all of its stormwater assets. Previously, we had used both text and numeric ratings for stormwater outfalls and inlets. Our new proposed rating scale is numeric 5-1 and U for unknown.

- 5** **New**
- 4** **Good**
- 3** **Fair**
- 2** **Poor**
- 1** **Extremely Poor/Replace**
- U** **Unknown**

The County's system is opposite of how the MSWGP rates conditions, as theirs generally follows MnDOT's scale. I didn't see any other comments of condition rating on the public comment period results. I just wanted to mention it if others use a different condition scale.

Lanya Ross
Metropolitan Council - Water Supply Planning

Overall, I was reviewing the document to see how the resulting data could be useful to help in groundwater modeling or other analyses of infiltration/recharge. I saw what I needed; whether this can be implemented remains to be seen, but I appreciate the goal of attempting this.

In the Draft Stormwater Geodata Standard, v.0.5 document on page 27:

Why is there no elevation data for the channels?

In the Draft Stormwater Geodata Standard, v.0.5 document on page 69:

How is the example provided in the Pollution Control Structure Type different than that provided in the Hydraulic Control Structure description?

In the Draft Stormwater Geodata Standard, v.0.5 document on page 100

How would you describe the Outlet Type for an underground structure, as an example?

In the Draft Stormwater Geodata Standard, v.0.5 document on page 101

In the Outlet Height or Mean Depth, does height refer to elevation or length?

In the Draft Stormwater Geodata Standard, v.0.5 document on page 138

Does the definition of structure include landscaped areas (For example: land graded in a way to capture water, even if no physical, constructed structure is present)? I assume so, but not entirely clear.

In the Draft Stormwater Geodata Standard, v.0.5 document on page 139

Some elevation data for BMPs could be useful to support modeling (defining head and flow)

Devon Savage
Swift County

We perform GIS work for Swift County in west central Minnesota, giving a more rural perspective to this project. Being a large farming community, our area has many drainage ditch systems that include open ditches and tile lines to move water. **Is it the intent of the standard to place ditches and tile lines into the “channels” and “pipes” layers?**

Would private ditches, tile, and lift station information be beneficial to collect? We only have the systems maintained by the county and the open ditches and tile lines are together in one layer. I think this project will be helpful for rural areas by **gaining access to the culvert and drainage data that the DNR/BWSR possess** since we have some systems that are on or near protected land. Having the ability to access a vast amount of drainage information in one location would be valuable when working with those entities on projects as well.

Duane Anderson
City of Woodbury

Like many in this business, we think it’s a good idea to **document date/time-oriented information on our assets** whether they’re related to Stormwater, Sanitary Sewer, or Water Main.

Unfortunately, that sets one up to either continually add date/time fields to accommodate the latest event, or one accepts that the only date/time information available is the last event. When the City of Woodbury opted to go with Beehive as its asset management package, we ran headlong into this concept and have since “come to Jesus” on **the more flexible concept of ‘top level events,’ i.e. a related table to accommodate events.**

Ben West
City of Inver Grove Heights

Over the last three years the City of Inver Grove Heights GIS Team has conducted a comprehensive database restructure – with a focus on key City infrastructure (Water, Storm, Sanitary). This was done with key contributions from our Engineering department, Public Works, and the help of an outside consultant (Bolton and Menk). This restructure focused on what our City staff view as key components to the different City assets while also **trying to improve: structure, logical groupings of assets, and overall completeness of data stored** (both adding fields and removing vestigial fields).

The type of guidance from a document such as the Stormwater Data Standard would have been an invaluable tool to use in that process and would have saved the City significant time (and money) in the reorganization of our GIS infrastructure. If nothing else, it would have served to provide helpful way posts to help guide internal discussions on the topic.

In part because we have so recently undergone our own data reorganization, in addition to providing feedback to MetroGIS, we as a City wanted to compare our data choices to the proposed recommendations found within the Stormwater Data Standard – and provide comments where possible. This process was done with our GIS staff and a Senior Engineering Technician – all who were the primary participants in the City’s data reorganization.

P BASN.6 – Basin Name

Have this **differentiated between dry or wet** depending the majority seasonal type of wet most of the time or dry most of the time. It might make sense to not include culvert here or rename it as something else;

P BASN.10 – Basin Design Volume

Does this encapsulate the live or the dead volume? Our engineers have defined this as an important differentiation and asked that both be included in our information.

P BASN.12 – Basin Design Flood Stage Elevation

Is this the critical water level? You already have the overflow elevation defined, so this is something different? There are almost too many different terms being used in storm water for the same thing. It would be helpful to have this defined with qualifiers, i.e. Elevation resulting from a 100 Year Storm or elevation resulting from back to back 100-year storms.

P BASN.29 – Basin Maintenance Agreement Number

Type of maintenance agreement is more important to us than the actual maintenance agreement number.

Note: *Discuss adding a field for Basin Maintenance Agreement Type and establishing a set of domains for agreement types;*

P BASN.40 – Basin Date Data Modified

When we've redone the schema for features, we have found it's easier to keep the standard ESRI naming conventions rather than creating a new one. However, I realize not every participating entity is using ESRI.

Additional values/attributes to consider adding or making use of:

- Dry/Wet Pond
- Low Floor Elevation
- Natural Overflow Elevation
- Drain Tile Present (Y/N)
- Landlocked basin (Y/N)
- DNR Pond (Y/N)

L PIPE.12 – Pipe Depth

Where on the pipe are you going to measure this?

If the pipe is 15 feet below surface on one end and 6 feet below surface on the other which value is entered?

***Note:** Discuss renaming as 'average depth of pipe' or establishing depth at the beginning/end of pipe. (More specifics are needed)*

L PIPE.12 – Pipe Depth

This will have to be field determined and would not be useful for maintenance at the city level.

L PIPE.22 – Pipe General Location

Too difficult to enter in Lat and Long for a line to make that useful. Too much inconsistency with what address would be used across length of pipe.

L PIPE.30 – Pipe Condition

*Mislabeled, should be **L_PIPE.29***

Additional pipe attributes to consider:

- Seepage collar (Y/N)
- Restrained (Y/N)

Channels:

Our channel/overland flow feature class was not part of our major redesign of features (this is a comparatively minor component of our storm water system). However, it does need to be revised and we will be leaning heavily on the MetroGIS final standard to rebuild the schema for this feature class.

Artificial Path:

We do not currently have Artificial Paths, but this is something we are highly interested in as a City and will be leaning heavily on the MetroGIS final standard to build the schema for this feature class.

Artificial Point:

We do not currently have Artificial Points, but this is something we are highly interested in as a City and will be leaning heavily on the MetroGIS final standard to build the schema for this feature class.

Additional BMP, Hydraulic Control and Pollution Control attributes to consider

- High water elevation – High water elevation the structure controls to
- Normal water elevation – Normal water elevation the structure controls to
- Sump (Y/N) – Sump present in the structure (very valuable to know this!)
- Sump Depth – Depth of sump
- Control structure both: fixture could be both a hydraulic and pollution control fixture
- Value (Y/N) – Valve present in the structure
- Weir (Y/N) – Weir present in the structure
- Weir High Water – What is the high-water level of the weir
- Weir Low Water – What is the low-water level of the weir

P IN.3 through P IN12

We understand separating yes/no for all the 3-12 field options, however, we as a management entity, would still find it valuable to retain a "type" field;

P OUT.10 – Outlet Type

Would prefer to have flapgate, ditch underground in the Type field

Additional P OUT attributes to consider

- Apron Material (Material of apron)
- Riser (Y/N)
- Submerged (Y/N)
- Trash Guard (Y/N)
- Erosion Control Method (*Denotes what type of erosion control method (if any) has been installed with the outlet: e.g. riprap or cabled concrete.*)
- System Flow (*Potential to maintain all of our aprons in one Feature Class and then designate in a field if those aprons are inlets or outlets*)

P MH.6 – Manhole Control

We place these in the control structure Feature Class. No matter if they're a manhole or something else. We don't see value in having it in our system twice.

P MH.7 – Manhole Trap

We place these in the Pollution Control structures Feature Class. No matter if they're a manhole or something else. We don't see value in having it in our system twice;

P MH.8 – Manhole Split

We would place these in the control structure Feature Class. No matter if they're a manhole or something else. We don't see value in having it in our system twice;

P MH.40 – Manhole Ownership Name

Ensure "Private" is included in this Ownership field

Additional Manhole attributes to consider:

- Manhole type (establish a domain of values)
- Manhole diameter (diameter of manhole)
- Restrained cover (Y/N)
- In Street (denotes if manhole is in the street or not)

P LS.3 – Lift Station Type

Maintain a LS type called "Emergency Lift Station" for temporary/emergency pumping stations

Additional Lift Station attributes to consider

- High alarm level (level where alarm sounds)
- Low alarm level (level where alarm sounds)
- Wet well diameter
- Pump gallons per minute
- Total dynamic head of the lift station
- Emergency pump station suction size
- Emergency pump station discharge size
- Generator back up present (Y/N)

BMPs:

We will not have a separate BMP feature class; we view this term (BMP) as being too nebulous as it too broadly encompasses features. Technically the pollution control structures are BMPs, Hydraulic Control Structures are BMPs, as well as encompassing education or other outreach or training to the public or staff. Internally, as a whole we find the term BMP to be poorly understood despite years of recurring education within the City.

Our path forward as a city is going to be encompassing these features within the specific structures/assets they most closely resemble - most notable including a dry ponds field within our Ponds Feature Class - or Basins as referred to in this document).

We also view the area for many of these features as being just as important as the location – i.e. we want to know the total square footage of permeable pavement in the City.

When viewing asset data, we as a City, prefer to view the associated data on the polygon and will move forward with that as our standard. Any points needed will be solely artificial points instead of as a BMP or Basin as point - we would consider adding a more comprehensive list of point types within the artificial points feature class for clarity; but want to avoid duplication of data as much as possible.

Monitoring Components

We do not currently have any representative assets of this feature type.

We would consider this standard monitor format if the City ever acquired any of this asset type.

Jon Røstum
Chief Strategist, Powel Environment, Oslo, Norway

In Norway we have worked on a related project on documentation tool of nature-based storm water solutions as a part of a national research program in Norway. I am especially interested in how far you have come **to develop a standard for documentation and asset management of different blue-, green- and grey-stormwater solutions such as green roofs, swales and infiltration systems.**

Kim Soulliere
City of Golden, Colorado

Did your group discuss MS4 requirements such as the **number of BMPs and which construction site they serve?** We are having trouble modeling the issue of one BMP serving several sites, causing a one-to-many relationship. Another piece we are challenged by is the one-to-many in translating the GIS model to Cartegraph where data collection takes place.

Kellie Thom
Minnesota Department of Transportation

- Pipe Width – should be the interior width;
- Pipe Equivalent Diameter – should match MnDOT specs;
- Pipe Length – Add disclaimer that entities might measure length differently (including or not including end sections);
- Pipe Condition – I asked that the inspection information not be included as we all inspect our features differently;
- Pipe Consequence of Failure Rating, Probability of Failure rating, Pipe Criticality to the system – These should not be included in the standard as how do we measure;
- Channels – open flume would be our most similar but not something we'd typically collect unless it was constructed. Most cities have a network of both designed and natural features which they depict how everything works together. I do not have enough experience to comment;
- Artificial Paths – Again used to create a water flow network by most cities but not something we use so cannot comment on;
- Basin Components – these include both our ponds and basins. Again, same comments about condition and failure rating and criticality;

- Hydraulic control Structures - same comments about condition and failure rating and criticality;
- Pollution Control Structures – The types were hard to pin down and I think need to be re-visited. Again, same comments about condition and failure rating and criticality;
- Artificial Points – Not something we use and cannot comment on;
- Inlets and Outlets – MnDOT does it by type not if it is an inlet or outlet like most cities and counties do. This will be the hardest for us to get our data into for sharing as it is something we do not check. For end sections we do have upstream and downstream but for structures it will be hard. Same comments about condition and failure rating and criticality;
- Manhole – this is another difference between us and others. Manholes are not inlets or outlets so would be separated out. Same comments about condition and failure rating and criticality;
- Lift Stations – These currently fall under special features for MnDOT and would not be able to fill out most of the information that is asked for. Same comments about condition and failure rating and criticality;
- Best Management Components – To me this is a repeat of the basins for some and we would not be using this;
- Monitoring – We do not collect this information so cannot comment;
- Basins – polygons – same comments as before;
- BMP – polygons – same comments as before;

Lisa Sayler
Minnesota Department of Transportation
Hydraulic Engineering

Thank you for the opportunity to review the standard. A lot of thought and effort have been put into it. The documentation is well done for adding clarify to the standard and having the sample data set is very helpful for starting to understand how the data works together.

As with any collaborative product, there will be parts of the standard that MnDOT will be able to meet for transferring data to other agencies, and other parts of the standard will be infeasible for MnDOT provide data. I've included some specific comments and suggestions in the attached document.

My primary concern overall is that regulatory agencies may have the expectation that MnDOT will have data in this format to transfer and there will be an expectation that the owner does have this data for the data attribute fields listed available. The documentation is very careful to repeat multiple times that this is a data transfer standard and not a requirement for individual agencies, but if this is adopted as a statewide standard, regulatory agencies may choose to require.

Another concern is the potential cost to develop data conversion tools so that we can convert our data to transfer. I think it would be helpful to address data conversion in the discussion, especially if there may be any tools or resources planned to be available. At a minimum, this discussion may be helpful for us to lobby within the agency to commit resources to develop the conversion tools.

Overall Concerns with the Standard

Mandatory vs Optional/Available:

We have concerns on potential impacts of adoption of this as a statewide data standard. The documentation is very careful to repeat multiple times that this is a data transfer standard and not a requirement for individual agencies. The data definitions are clear on what data is mandatory vs. not. However, once adopted there may be agencies that we work with or get permits from that try to require some of the parts of the standard that it may be difficult and costly for MnDOT to conform to.

The data field included in the standard are extensive and it is unlikely that MnDOT would either have all of them or be able to fully populate them. Also, because of the attribute definitions/domains, there will not be a direct conversion for some of the data that MnDOT does collect.

Data Conversion Costs

MnDOT has an “in place” database for storm drain features and inspections that it has been using for over 20 years. It will take resources and expertise from MnDOT beyond what our unit has available to develop the necessary “cross-walk” and processes to transfer are data into this standard (if requested) and to be able to use other agencies data. There will be some data fields where it may not be able to transfer data because definitions/schema don’t match exactly.

I recommend some content be added to the Overview, Context and FAQ section on what resources may be necessary to export/import data from the standard and if there are/will be any tools developed. If potential grant money becomes available as suggested in EQP State Water Plan, would be nice if could be directed to conversion development as well as data digitization as suggested.

Inlet, Outlet, Pond Inspection Schemas

I have concerns about including the inspection schema as part of an overall package for a standard. This would be very difficult to use as a transfer standard because of the different ways that agencies describe potential condition/problems. A lot of inspection data that MnDOT collects could not be transferred because we use Yes/No flag ratings that don’t transfer to the domains in the standard.

With regards to adopting this as the data standard, MnDOT already has an inspection schema that does not match this, and it would require a lot of time, training and expense to change as well as making historical data much less useful. If this is approved as a standard, then there may be requirements and expectations on the part of other regulatory agencies that everyone they regulate must provide data in this format.

If this is intended to be used as a data transfer standard as well as an inspection data collection standard, need to plan for data fields where agency does not collect and store data by have null or unknown as options.

- Recommend against including a suggested condition rating – many agencies may have their own or be using PACP – difficult to translate between rating codes and gets confusing since may have different scheme for numbers;
- As applicable, domains/attribute fields should be options for None and Unknown. If this is used to transfer data, agency may not have collected that data, or may not have collected it in a way to allow transfer;
- What should be input for rainfall amounts if unknown – field should not default to zero, null should be allowed

Stormwater Standard - Components

Component Overlap – multiple records for individual features required?

Is it intended that an agency's stormwater feature needs to have a record for each component type that it might be part of? It is common that stormwater ponds and infiltration features will be both basins and BMPs. Less common but possible is that an inlet may have a sump/SAFL baffle and so also be a pollution control structure. If an agency only tracks these as one type of feature, do they needed to be included in both data sets, or is this only for when the data owner tracks them separately (appears in sample data set there are different IDs when a pond vs a BMP). Recommend more explanation on how to include where matches multiple component definitions.

Component Definitions

I think it would be helpful to add some more discussion on what defines whether a feature is a BMP vs a Pollution Control Structure. I think the domain list is helpful, but it would also be helpful to have a descriptive comparison. I also think it would be helpful to go into more detail in this overall description of components of where different types of underground detention/retention/filtration structures fit into rather than making people search through the domains.

Are stormwater tunnels pipes? If so, recommend that tunnel be added as a pipe type. Otherwise, need to define how/where they are included. Also, would be helpful to address in general component definition at beginning of documentation.

Component types common data

Federated ID – Not sure how this ensures a unique ID if only prefix only based on location/CTU where located. Other agencies/entities will be supplying data and have their own way of naming but could have a convention – such as just a number – which matches another agency with features within same CTU unit. Unlikely but possible that MnDOT *feature_ORID* will match a local agency *feature_ORID* for different features when they are in the same jurisdiction.

Ownership Name/Maintenance Authority Name

AgencyOwnName domain should include MnDOT/Minnesota Department of Transportation rather than lumping in with State of Minnesota, seems likely that there are other state agencies that also own or are responsible for maintenance of stormwater features that should be included specifically. Some of the sample data uses MnDOT for attribute data that standard shows used AgencyOwnName.

Data Producer/Source Name is listed as attribute name twice for each component. One based on using AgencyOwnName and the other a text field without domain. Having the same attribute name is confusing and the definitions are not real clear as to what is the difference between the two fields.

Consequences of Failure, Probability of Failure and Criticality to the System. The rating domain for these fields is very subjective and not well defined. Will be difficult for those that do rate these attributes to be able to combine data from other agencies that may use a different definition.

Pipe Components Field Definitions

Pipe Diameter – what is expectation if the fixture is not circular – null, 0?

Pipe Equivalent – in order to get consistency, recommend more precise definition. Suggest following MnDOT standard plates since many agencies use. MnDOT standard plate definition for equivalent diameter (if that is intention) as: EQUIVALENT DIAMETER EQUALS DIAMETER OF CIRCULAR PIPE WITH APPROXIMATELY EQUIVALENT CROSS-SECTION AREA. Figure 1 definition for Pipe Equivalent Diameter is what MnDOT Standard Plates call out as Span.

Why is **pipe height** the inside, and **pipe width** the outside? More consistent to see both as inside dimension and then add pipe thickness as attribute. For utility conflict, probably want to be able to get both outside width/height. For hydraulic modelling, want to be able to get shape, inside width, inside height in order to figure out hydraulic properties.

Pipe Type – because domain values so specific, may lose some data in transfer. For instance, we may not always know if drain tile is perforated or not because may be lumped together. Given the list of attribute values, would need to transfer as other type. I don't know if this is national standard or not but may have been better to have perforated as own attribute field where values are perforated, nonperforated or unknown.

Pipe_Mat domain should consider additional value and/or more description. Most **Corrugated Metal** pipe used is **Galvanized** – which is preferred? For asset management purposes, important to know if metal pipe has Aluminized or Polymeric Coatings.

Vertical accuracy value included for structures but not for pipes – this seems inconsistent.

Basin Components Field Definitions

Basin Type – confusing to include Culvert (centroid) as a basin type. A general definition of a culvert is an open-ended pipe that conveys water from one side of an embankment to another. Is this meant to be used for underground storage consisting of pipe segments?

Hydraulic Structure Components Field Definitions

Hydraulic Control Structure Type – not clear why Deck Drain listed under HCS when it also a data field in inlets. Confusing to me, needs for description to understand when a deck drain is a HCS and is it either an inlet or HCS, or is it both. With Detention and Retention tanks listed as HCS, does this mean underground storage? If so, would be helpful to include that in the overall description.

Pollution Control Structure Components Field Definitions

Pollution Control Structure Type – Definition includes example types and description for hydraulic control structure

Outlet Components Field Definitions

Outlet Type: Since outlet type includes culvert, is it expectation that there will be an outlet created for every culvert?

John Gulliver, Ph. D, PE
Department of Civil, Environmentla and Geo-Engineering
University of Minnesota

I have some comments on the draft standard allocation of BMPs, and your question to the users about whether they should be listed as points, lines or polygons:

Here are the practices that I feel should be a line:

- bioretention-rain garden (most of the time)
- filtration bench/shelf (no underdrain)
- filtration bench/shelf (with underdrain)
- filtration swale (no underdrain)
- filtration swale/shelf (with underdrain)
- infiltration trench,
- tree box,
- permeable pavement road
- planter
- porous paver road
- porous concrete road

Here are the practices that I feel should be a polygon:

- amended-composted soils
- dry pond
- filtration basin (no underdrain)
- filtration basin (with underdrain)
- green roof
- iron enhanced filter
- infiltration basin
- sand filter
- stormwater pond/wet pond
- offline basin
- permeable pavement parking lot
- porous paver parking lot
- porous concrete parking lot

2D Pilot Model Build Model Build Report

Prepared by:
Kimley-Horn

Prepared for:



MINNEHAHA CREEK
WATERSHED DISTRICT
QUALITY OF WATER, QUALITY OF LIFE

Date:
April 2023

Kimley»»Horn

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Document History

| Date | Version | Description of Change |
|---------------|------------|--|
| December 2022 | 1.0 | Initial Draft |
| January 2023 | 2.0 | Revised Draft |
| February 2023 | 3.0 | Revised Draft |
| April 2023 | 4.0 | Revised Draft |
| April 2023 | FinalClean | Revised-Final with MCWD final comments addressed |

Definitions

- ArcGIS – a desktop and cloud solution provided by Esri for GIS analysis, storage, data management, and data processing
- DEM – Digital Elevation Model, represents ground surface in a grid (raster) format. Elevations are assigned to each individual grid cell
- GIS – Geographic Information System
- ICM – InfoWorks ICM software package developed by Innowyze to perform 1D and 2D hydrologic/hydraulic simulation modeling
- ICPR – Interconnected Channel and Pond Routing (ICPR) software package version 4 developed by Streamline Technologies to perform 1D and 2D hydrologic/hydraulic simulation modeling
- MetroGIS (MGIS) – a GIS format designed for use by Twin Cities Metropolitan-area municipalities for the standardization of infrastructure data
- Geodatabase (GDB) – GIS file and data format that allows for standardization and template creation
- File Storage – cloud or physical file storage location utilized as a central repository for raw, input, and output datasets
- Shapefile – Spatial data file format that includes attribute data for individual shapes. May be in a point, line, polygon format and includes the file extension “.shp”.
- Simulation – Collection of input parameters from various hydrologic and hydraulic processes as well as tolerances that culminate in the calculation of the flow of water over a defined period.
- Scenario – Situation that incorporates changes to the input parameters that represents a real-world condition.
- Lidar – Light Detection and Ranging. A portion of the remote sensing information that is gathered via aerial methods that includes numerous points of data that can be classified into types for ground elevation modeling.
- Master database (ICM) – File extension .icmm that includes all model information.
- Model group(s) (ICM) – Individual file/object folders within a master database that contain objects
- Refinement Elements – Breakpoints and Breaklines that are used to refine the 2D mesh

1 INTRODUCTION

The Minnehaha Creek Watershed District's (MCWD) current modeling tools are outdated and do not provide the required granularity and features necessary for the District to effectively characterize and quantify the impacts of climate change. Therefore, District staff identified the need to develop a new modeling tool that has greater granularity that can better evaluate a range of scenarios towards informing decisions relating to climate adaptation strategies, programmatic policies, and specific projects. MCWD began the process to select a better tool/model by completing a cursory assessment of the full range of two-dimension modeling software systems currently available. This screening-level assessment, along with vendor information sessions and consultation with agency experts, led the District to narrow their focus to ICPR and ICM. Both were selected to be built within two distinct subwatershed areas (parts of City of Victoria and City of Edina) for the pilot model build analysis, giving the District an opportunity to comprehensively compare the two software packages. The District chose to pursue a pilot model build, ahead of the full watershed-wide build, to mitigate for the relational and technical risk that is often associated with large-scale, high-resolution models, such as selecting the right software for the intended use.

This memorandum provides an overview of input datasets, model build process, and challenges that were uncovered during the model build process for each software package. The information gathered during this portion of the project will be critical to the understanding of the benefits and challenges each modeling platform presents and ultimately for selection of a future watershed-wide modeling platform. Upon selection of a modeling platform, this information will also inform future implementation of the watershed-wide model build.

The specific model version used for this pilot model process were:

- ICM version 2023.2.0 with an unlimited license; and
- ICPR version 4.07.08 with an expert license.

2 MODEL INPUT DATASETS

The following subsections are formatted in the following way: Background Information, ICM import and defaults, ICPR4 import and defaults, and takeaways. Additionally, both software packages allow for user creation of new features directly within the software using the hand delineation tools and user inputs of parameter data.

2.1 Data Import Processes

2.1.1 ICM

ICM allows for import of data using one of three pathways depending on data type and desired use within the software. The main pathway utilizes the Open Data Import Centre (ODIC). The ODIC allows for import of data in the following formats: MapInfo TAB, GeoPlan Layer, CSV, Tab Separated Data, Access Database, Oracle, SQL Server, Raw Shape File, and XML. The ODIC allows for configuration files to be saved and loaded to set the import fields and default values. The ODIC also allows for import of spatial and lookup table data. Figure 1 shows the ODIC dialog box, the various inputs, and settings that can be applied to the input. The MGIS data was imported through the ODIC by individual file specification along with the appropriate configuration file for each data type.

The second data import pathway is utilized for import of the Digital Elevation Model (DEM). The DEM is imported directly into a Model Group as a Ground Model grid InfoWorks object. The ground model grid import allows for specification of horizontal and vertical unit type (feet, meter), cell size, and clipping boundary. Other unique spatial objects or simulation controls can also be directly imported into a model group. Appropriately defining the model groups within each master folder allows for consistency with model updates and efficiency when reviewing results. Model groups can hold a single data type or multiple data types depending on the folder structure that is required. For the pilot model build, master groups for Edina and Turbid-Lundsten subwatersheds were created with model group folders in each to hold to respective model data types and entries. Model group data can be referenced from outside of the master group during simulation runs (e.g., Edina rainfall data can be referenced into a Turbid-Lundsten simulation run).

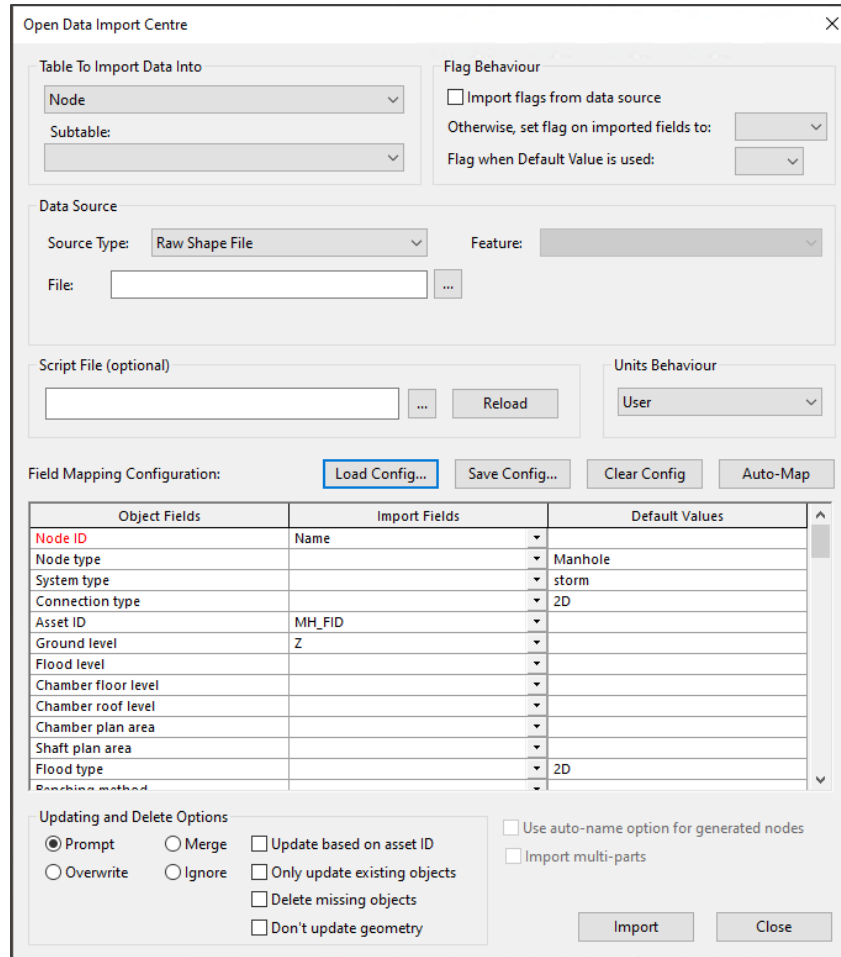


Figure 1. Open Data Import Centre Dialog Box

The third data import pathway is utilized for referencing outside data through the GIS Layer control manager. The GIS Layer control allows for the import of background reference files, aerial imagery, and any other desired shapefile import. The raw shapefiles can be in a different projection than the model, but it is recommended that they are the same projection for faster processing and viewing speeds. Shapefiles that are loaded in through the GIS layer control can be selected and attribute fields displayed directly within ICM for reference. The GIS layer control is used to view data and not to be included within the model simulation calculations. This can be helpful to reference in different background aerial imagery layers, verification of shapefile import, and result analysis based upon highlighted areas.

2.1.2 ICPR

ICPR allows for the importing of data from two primary sources and accepts multiple formats. The primary method of importing data is through the menu tab, using CSV files in the GWIS format. Data can also be imported from the data tree in graphic view, data can be imported using CSV files or shapefiles. It is important to note that any data imported using shapefiles will only include the element shape, placement, name, and upstream/downstream connections (if relevant). Importing pipe network data through shapefiles will result in the pipes missing invert, size, material, length, and loss coefficient data. This data would then have to be entered manually.

ICPR does not allow manual mapping of data fields from imported files. All data fields need to be named in the ICPR standard GWIS format before importing the data. Data was imported into the model using CSV files produced from the MGIS data for all 1D hydraulic elements including junctions, pipes, and outfalls. The model boundary, soils, and LULC data were imported using shape files. The DEM was imported from a raster.

2.2 Surface Datasets

2.2.1 Model Boundary

The model boundary files for the subwatersheds were supplied in layer format within a geodatabase. The layers were exported to individual shapefiles. The shapefiles did not have additional attribute data. ICM designates the 2D mesh and flow area as a 2D Zone and ICPR designates them as an Overland Flow Region.

ICM uses the model boundary for the 2D Zone input and assigns a vertical wall type to the boundary by default. The vertical wall does not allow water to leave the 2D Zone anywhere. The boundary type can be set to vertical wall, critical condition, supercritical condition, dry, or normal condition. The selected boundary type applies to the entirety of the boundary. The 2D Zone condition can be changed by using 2D boundary objects along a section of the 2D Zone boundary. The 2D Zone boundary can be adjusted within an individual scenario in ICM.

ICPR uses the model boundary for the overland flow region and takes a shapefile as an input. ICPR treats the model boundary as a vertical wall. The model boundary for ICPR was simplified and expanded to encompass a larger area. The simplified boundary was used to reduce the small mesh elements along the boundary of the model area. The expanded area was used to allow the groundwater flow to transition in and out of the original model area. The model boundary cannot be adjusted to incorporate different edge conditions without the additional incorporation of boundary stage lines or boundary stage points as discussed in Section 2.5.3.2.

The model boundary is slightly more flexible in ICM with the ability to assign different boundary conditions directly within the model boundary parameters where ICPR requires additional 2D features be created to vary the overland flow region boundary condition. Both models allow for the model boundary condition to be varied across its length using boundary stage lines and/or boundary stage points. The boundary stage lines can be applied to a particular portion of the model boundary and the boundary condition can be varied depending on the desired simulation scenario. It is recommended that model boundaries contain as few vertices as possible to represent the outline of the desired area. Vertices and shapefiles can be simplified by the user within GIS through manual and automated processes. This reduces mesh building and triangulation errors that can occur when multiple boundary points are spaced closed together.

2.2.2 Digital Elevation Model (DEM)

The digital elevation model (DEM) was built from MnTOPO lidar data collected in 2010 and 2011. The lidar data was collected using the UTM Zone 15 horizontal datum and the NAVD88 vertical datum. The horizontal and vertical units are in meters. The mean post (lidar point) spacing for the lidar collected was set at a maximum of 1.5 meters (4.92 feet). The lidar was resampled into a raster format for import to the models. The raster cell spacing was set at 0.5 meters (1.64 feet) for the import. For comparison, three cell spacing scenarios were developed to demonstrate future storage needs. The smaller the cell size, the greater the number of cells to cover an area. Assuming a total watershed area of 178 square miles and a 0.5-meter cell spacing, the overall DEM file would include approximately 6 billion cells. Generally, there is an inverse squared relationship between cell spacing and number of cells (e.g. reduction in cell spacing by ½ equates to a 4-fold increase in number of cells). The DEM raster cell size cannot be varied across the model area. Data presented in Table 1 illustrates how the cell spacing relates to the file size and number of cells for the surface in the Edina subwatershed model.

Table 1. DEM Cell Spacing vs. File Size vs Number of Cells

| Cell Spacing (meters) | File Size (KB) | Number of Cells (million) |
|-----------------------|----------------|---------------------------|
| 0.5 | 94,857 | 32.8 |
| 1.0 | 22,229 | 8.2 |
| 2.0 | 6,363 | 2.0 |

It is important to note that the 2D mesh grid is set to a lower resolution than the underlying DEM, the methodology for the mesh grid is discussed further in Section 2.5.2. The limiting factors in reducing the DEM resolution are the corresponding increase in required storage space, the resulting slowness during viewing a larger raster file in GIS and in the modeling software packages, and significantly increased simulation run times. There is also a diminishing return due to the 2D grid methodology used by both software packages to set the grid element elevations.

ICM imports the DEM as a ground model grid. There is a second option to import a ground model tin format instead. The ground model grid import allows the user to set a name, units for ground elevation and x,y coordinates, base cell size, and clipping the import to a polygon. Multiple ground model grids can be imported to a single master database. An individual ground model can be added and viewed in a single or multiple GeoPlan viewers. The ground model symbology can be adjusted to be transparent, shown as contours, or opaque. The color ramp can also be adjusted to specific colors or to be based upon only the area shown in the GeoPlan viewer.

ICPR imports the DEM into the surface manager menu. Vertical units are assumed to be feet, and the cell size is equivalent to that of the imported DEM. Once imported, the DEM can be viewed, the opacity can be edited, and the color scheme can be based on the entire surface, or the portion of the surface shown in the graphic view. Multiple surfaces can be included within the surface manager and be applied to different scenarios. The surface manager also holds the initial water table surface and confining top layer surface for use with the groundwater module.

The Edina subwatershed includes two bridges that are shown as a berm or dam in the lidar data. The bridge crossings of Minnehaha Creek were removed from the lidar points during the lidar preprocessing work but the relatively short span of the bridges result in triangulation issues during the DEM creation. Figure 2 shows the crossing of Minnehaha Creek at Wooddale Avenue S along with the “missing” lidar point area where the bridge elevation data was removed. Lidar points are classified by return number and different types of rasters can be created by using different selections of the return numbers. The lidar obstructions created by these bridges (or the lack of lidar points) were removed through terrain edits within GIS. Removal of the bridges was necessary to more accurately simulate flow along Minnehaha Creek. Additional cleanup of the bridge crossing locations were required to accurately simulate flow through these areas within ICM and ICPR. The updated terrain file was included in both the ICM and ICPR model builds.

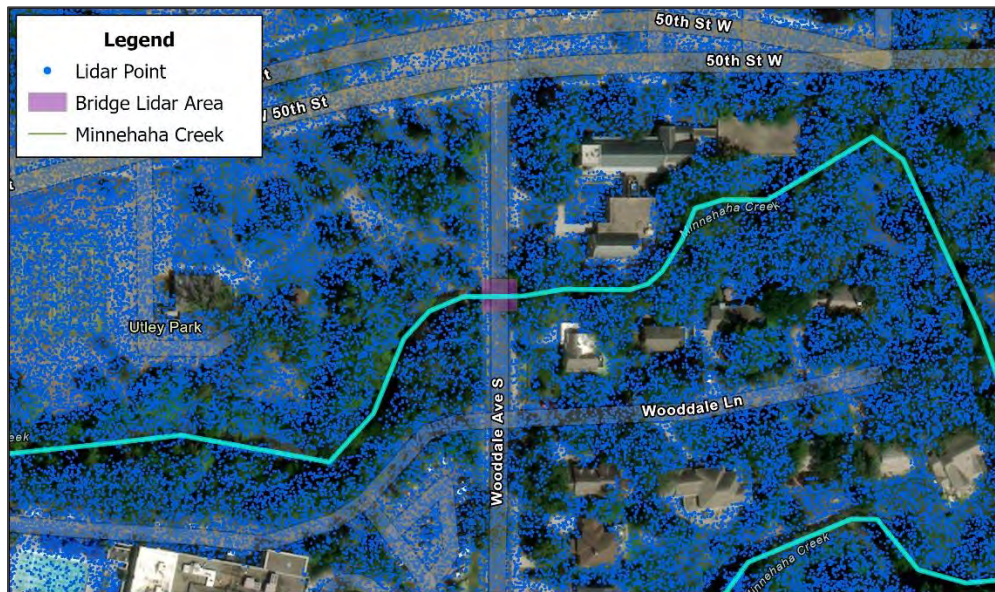


Figure 2. Bridge Lidar Triangulation Area

During the watershed-wide model build process, crossings of Minnehaha Creek, tributaries, and major lakes by bridges should be reviewed to determine the required data needed to accurately model significant crossings. In areas where the creek is completely spanned by a bridge and the flood waters do not reach the low member of the bridge, then modifying the underlying terrain and removing any inconsistencies would be a reasonable approach. If a bridge crossing has piers, sizeable

abutments in the floodplain, and/or is likely to be impacted by the flood waters, then the bridge data should be included in the model. If as-built information is not available, bridge data may need to be surveyed to be included in the modeling packages.

The required bridge information includes upstream and downstream cross-sectional data of the creek, upstream and downstream bridge cross-sectional information and the bridge deck profile. This data should include any additional obstructions in the creek and floodplain that would inhibit the flow of water downstream, as-built data for the bridge deck, upstream and downstream faces of the bridge, assuming that the cross-sectional data for the creek is included in the as-built data. A surveyed reference point may be required to spatially place the bridge depending on the robustness of the as-built dataset.

Due to the horizontal datum being set to UTM, ICM defaults to meters for elevation and sizes after each import. While the model is functional using the UTM projection in ICM, it creates nuisances that if not caught and can cause larger issues down the road with inconsistencies between units of different model parameters. Therefore, if this software is selected the project team strongly recommends that a model spatial projection of Minnesota State Plane – South be used to reliably and repeatably import data to the modeling software, which is based in feet and inches instead of meters and millimeters.

2.2.3 Land Use/Land Cover (LULC)

Manning's n roughness values are derived from the associated land use/land cover (LULC) layer. LULC was taken from the Twin Cities Metropolitan Area Land Cover Classification dataset. The dataset is generated by the University of Minnesota at a 1-meter resolution using high-resolution multispectral National Agriculture Imagery Program data including leaf-on imagery, spring leaf-off imagery, lidar data, multispectral derived indices, National Wetland Inventory, lidar building footprints, and other thematic data (2015). This LULC dataset does not break out road sections individually.

ICM imports the LULC data in two parts. The first part is the Roughness Zone, and the second part is the Roughness Definition. The zone is used to delineate the boundary of each landuse type. The definition is used to reference the roughness parameters for each LULC zone. A single roughness definition may be applied to multiple roughness zones. Each roughness definition may have up to three roughness parameters with unique phase-in depths. This is important to modeling areas that experience minor inundation followed by more extreme inundation. Varying the Manning's n values based on flow depth accurately models the reduction in roughness that is experienced at deeper flow depths. A single roughness definition can be applied directly within the roughness zone. An overall roughness value can also be applied to the whole 2D Zone or to areas without a Roughness Zone. All areas should be incorporated within a roughness zone from the input landuse file, although small gaps may occur during editing by the modeler. These gaps would be filled automatically by the underlying application of an overall roughness value. The gaps can also be removed through GIS processes prior to importing the GIS shapefile to ICM. The LULC delineations should be individual shapes, not multipart shapes within a shapefile.

ICPR imports the LULC data from a shapefile, then creates a raster from this data. Manning's n roughness values are assigned to the raster through a user-defined input table. ICPR uses deep and shallow manning's n roughness values based on user defined depth ranges. ICPR can also apply one roughness value to the entire 2D mesh which can be beneficial during initial model build processes or smaller model areas that are generally covered by consistent landcover.

Both software packages treat roughness zones and roughness definitions as separate entities, and both allow for depth varied roughness values. ICM has a more robust depth varied system, allowing three values as opposed to ICPR's two-value system. ICPR converts the LULC data to a raster while ICM LULC data remains in polygon format. To edit the ICPR LULC delineations, the data must be reimported and converted to a new raster while the ICM LULC shapes can be edited directly within the software. Building footprints can also be incorporated within both software packages. Building footprints can be used as obstructions to block overland flow paths. Within ICM, the obstruction can also be varied to allow flow through at a user-defined inundation depth. Increased roughness values within a building footprint as part of the LULC delineation is an alternative way to model the building obstructions. Within ICPR, buildings can be modeled as extrusions or exclusion areas depending on the desired effect to the ability for water to flow. Extrusions may be used when a building may collapse during an extreme flood event and allow flow through the area above a defined elevation. Exclusions remove the area completely from the 2D overland flow region.

2.3 Subsurface Datasets

2.3.1 Soils

The main Green-Ampt Parameters include suction, conductivity, and deficit (porosity). The values of these inputs range for each soil type and can be manually edited as part of a calibration process to reduce or increase the amount of surface runoff.

Similar to the LULC data, ICM takes the soil data in two parts, the Infiltration Zone and the Infiltration Surface. The infiltration zone contains the spatial delineation for each soil type and the infiltration surface contains the soil parameters for the individual soil types. ICM utilizes the following three soil parameters: suction, saturated hydraulic conductivity, and deficit. The moisture deficit value is set to the porosity value of the soil. This correlates to dry soil and allows for maximum amount of infiltration to occur in the simulation. As rainfall is infiltrated the saturation of the soil and effective infiltration rates vary within the model space. As rainfall recedes, the soil becomes unsaturated at a rate that is calculated through the saturated hydraulic conductivity parameter. All of these values can be adjusted to more accurately simulate in place conditions. The soil areas must be imported as separate features within the shapefile. ICM does not allow for multipart features to represent the delineation of multiple soil areas with the same soil parameters.

ICPR takes soil data the same way it takes LULC data. The first part is a shapefile import that is converted to a raster in ICPR, and soils data is entered into a table that gets paired with the raster file. ICPR can use Green-Ampt, Curve Number, or Vertical Layer methodology to model infiltration, this model build utilizes Green-Ampt methodology. ICPR takes 9 soil parameters: vertical saturated hydraulic conductivity (ft/day), saturated moisture content (decimal), field moisture content (decimal), initial moisture content (decimal), wilting moisture content (decimal), residual moisture content (decimal), pore size index (decimal), bubble pressure (in), and depth to water table (ft).

ICM and ICPR both treat soil zones and soils values as separate entities, but they handle them differently. ICM uses an infiltration surface to define individual soil properties while ICPR stores soil properties in a table that is linked to the soil zones before running the simulation. ICPR takes all relevant NRCS soil parameters for calculating Green-Ampt infiltration while ICM uses suction, conductivity, and deficit. These extra parameters allow soils in ICPR to recover more accurately in multi-event rainfall simulations. Similar to the LULC data, ICPR creates a raster of the soil type delineation. To edit the delineations of the soil layer in ICPR, the soil layer must be edited outside of ICPR and reimported to create a new raster. ICM soil data remains in polygon format and can be edited directly within the software. ICM allows for individual infiltration surface delineations to be modeled with different hydrologic methodology and input parameters. This is useful when the modeler desires to eliminate infiltration over impervious surfaces or has other predefined soils data.

2.3.2 Groundwater

ICM does not allow for import and use of groundwater data within the modeling software. One potential way to mimic groundwater levels is through the manipulation of the Green-Ampt parameters to simulate various soil conditions. Increasing or reducing the initial moisture content within the soil parameters would mimic wet and dry conditions at the beginning of the simulation.

ICPR can model two-dimensional groundwater flow using a triangular mesh similar to the two-dimensional overland flow mesh. The groundwater mesh and the surface water mesh can interact with each other through recharge, infiltration, seepage, and leakage. The model build incorporated recharge, infiltration, seepage. The leakage portion was not included as this portion of the groundwater model relates to the loss of groundwater through the confining (bedrock) layer. Groundwater modeling in ICPR requires multiple inputs to set initial conditions and soil parameters. Surfaces representing the ground surface (same as overland flow region), initial water table, and confining layer are required for developing the scenario. Single elevations can be specified in place of a surface for each initial condition input. Soil parameters must be specified for fillable porosity and conductivity zones. Typically, the zones for the soil parameter zones match the Green-Ampt soil infiltration zones.

The initial groundwater table was set based upon the average depth below ground surface at the groundwater monitoring wells for the entire model area. The corresponding initial water table surface was created by offsetting the ground surface by the average depth. The confining layer surface was created by clipping the countywide bedrock elevation raster contained within the county geologic atlas datasets. The fillable porosity parameter was assumed to be 0.3 for areas below the surface

and set to be 1.0 for areas above ground. This dimensionless parameter correlates the available porosity within the soil layer to the location and is given as a percentage of the total available volume (0.3 = 30% void space). Conductivity was conservatively set equal to vertical saturated hydraulic conductivity from existing Green-Ampt data. Typically, measured horizontal hydraulic conductivity is greater than the vertical hydraulic conductivity. Multiple groundwater regions can be delineated, and they will interact along any face that is wet in the overland flow region. ICPR recommends breaking the groundwater regions at creeks and lakes to reduce the overall size of each groundwater region and increase computational efficiency.

ICM does not allow for groundwater modeling in a 2D simulation while ICPR supports combined surface water and groundwater 2D model and surface water only 2D model setups.

2.4 Infrastructure Datasets

2.4.1 Junctions

Junctions are used within the software packages as end points for conduits, connection points to the 2D overland region, and discharge locations. The junctions are taken as point features. All junctions from the input datasets were included in the model build. There are no limitations for the number of junctions included in either modeling software with the software license that was used. Lower license levels of a software package may have limitations on input and model datasets.

The 1D junctions within ICM can be used to model a variety of situations and interactions between the 1D system and 2D region. The junctions can be set to set as one of multiple different flood types as well. For the model build, the junctions were set to 2D as the flood type. The 2D flood type methodology uses a weir equation to calculate flow from the 2D mesh into the junction. The weir length is taken as the circumference of the junction which is calculated based on the diameter of the largest pipe that is connected to the junction. The size of the junction can also be manually edited by the modeler. Interaction between the junction and 2D mesh is set to depth (by default). This parameter can be changed under the 1D-2D linkage basis parameter to elevation to minimize oscillations within the calculations during the simulation. If there is a large discrepancy between rim elevation and ground elevation or if it is desired to model an in-place condition then using the elevation setting can produce the desired effect. The other main flood type for inlets is set to Inlet 2D. Inlet 2D allows for additional inlet parameters to be set based upon user input. A head-discharge table, flow efficiency relationship, custom equation, or HEC-22 data may be entered to increase the level of detail within the inlets in the model.

Junctions in ICPR can be set to one of multiple options to simulate conditions at the junction. Junctions can be setup to include stage-area, time-stage, or stage-volume data. The junction may also be setup as a 1D Node Interface element as part of the overland flow region. Junctions are part of the 1D network and the 1D Node Interface is setup as part of the 2D overland flow region. The 1D node interface forms a connection between the 1D network and the 2D overland flow region allowing water to pass between the two systems. These junctions have no area, and their elevation is assumed to be the same as the DEM at the insertion point. 1D node interface elements default to assigning a starting water elevation equal to the ground level. This default is appropriate for 1D node interfaces that represent outlets of culverts or pipe systems into the overland flow region but is not correct when modeling storm sewer inlets. The user can set the 1D node interface elements to start with a water elevation equal to the invert through import settings and/or through manual editing of elements. Time-stage nodes were used to represent outfalls which are further described below. Stage/Area Nodes connect to pipes and can have an assigned initial depth and stage/area relationship to represent storage at the node. When no initial depth is assigned the lowest invert from a connecting pipe is used. The Stage/Area Nodes in the model build did not have an initial depth or stage/area relationship.

The main difference between ICM and ICPR is that ICM utilizes the node rim elevation as the default setting and ICPR utilizes the DEM to set the rim elevation for each junction. ICM also allows for additional detail to be added for inlet capacity or known inlet rating curves to be specified by the user. The starting water elevation needs to be set within ICPR for all 1D node interface elements or the software will introduce additional water to the simulation and become unstable. The specification of the starting water elevation needs to be completed prior to data import or nodes will have to be individually adjusted within the model which can become time consuming.

2.4.2 Pipes

All pipes were imported to the model and are utilized to convey flow underground through the 1D storm sewer system. The pipe dataset has the greatest number of data gaps that were filled through various processes prior to import to each model and post-import to each model. Pipes with missing diameters were set to 12.1 inches, missing invert data was set with a user-defined DEM offset, and missing pipe material data was filled with an unknown-type place holder with the associated Manning's n set to 0.016. Furthermore, numerous pipes contained data but were shown to be incorrect when included in the software packages (invert elevations that differed by 80+ feet or were missing a number [80.76 versus 880.76] for invert elevations). This erroneous data appeared to either have duplicate or missing numbers in elevation when compared with surrounding pipes and the DEM. Filling the invert data with a DEM offset for the main trunkline storm sewer system caused issues with oscillations when the models were built. The DEM offset was overwritten for various pipes within the system through linear interpolation of upstream and downstream inverts of neighboring pipes to allow the system to function properly and allow drawdown of ponding locations. The DEM offset automation may need to be reviewed during the watershed-wide build process to determine the overall applicability to the full dataset or to targeted areas.

ICM utilizes pipes as the conduit input. The junctions (inlets, manholes, outfalls) must be imported prior to the import of the pipe layer to allow for snapping of the upstream and downstream ends of the pipe to the associated nodes. Any pipes with updated sizes, post-import, were changed to have a suffix of .2 for differentiation. The pipes were reviewed against the original dataset. This process was revised and included in the challenges section of this memo.

ICPR also utilizes pipes as the conduit input. Junctions can be imported before or after importing the pipe layer because the pipes are not snapped to nodes based on their proximity. Pipes are connected to their relevant junctions based on "From Node" and "To Node" name fields in the pipe attribute table. Some pipes needed to be manually assigned node names even after pipe data was processed in GIS. This process is more difficult in ICPR than in ICM because the property table for the pipe link needs to be manually edited as opposed to snapping pipes to nodes in ICM. Scaling this to a watershed wide build, the process would increase the time needed to manually edit pipe links but would not increase the difficulty of the process.

Both software packages needed additional manual processing to finalize the pipe import process and successfully run the models after the import was completed. Some pipes included upstream or downstream node names that did not match the nodes. They appeared to be from a previous naming convention. Due to the presence of erroneous data in these attribute columns, the preprocessing in GIS did not reassign new upstream and downstream node names for these pipes.

2.4.3 Outfalls

Depending on the robustness of the infrastructure dataset, outfalls may need to be imported separately to complete the 1D portion of the model build process. The Turbid-Lundsten subwatershed did not have outfalls as a separate input dataset and no additional import was required. The Edina subwatershed did have a separate outfall shapefile that was imported into the models. The outfall file was needed to form the connection at the outlet of pipe runs to the 2D mesh.

ICM treats outfalls as nodes with either the node type set to Outfall 2D or Outfall. The Outfall 2D allows for connection back to the 2D mesh while Outfall allows for free discharge from the end of the pipe and out of the model. The Outfall is meant for areas where the model is clipped and no tailwater condition is assumed. If a tailwater condition is assumed, then a level line can be applied with an appropriate level for the duration of the simulation to mimic the downstream conditions.

ICPR treats outfalls from the overland flow region as nodes with node type set to Time/Stage. A Time/Stage node with no table of values attached represents a free outfall with no tailwater condition assumed. A tailwater condition can be modeled by adding a table of time/stage values to the node. For outfalls connecting back to the 2D mesh, a 1D Node Interface connects pipes back to the 2D mesh.

Both ICM and ICPR allow for free outfall conditions, tailwater conditions for outfalls, and outfalls that connect back to the 2D mesh.

2.5 Model Build Datasets

2.5.1 Rainfall

The models were initially built and tested using the MSE-3 distribution and a rainfall depth of 7.4 inches. This rainfall distribution is the standard design rainfall distribution for the region for a 24-hour rainfall event. A second rainfall file was used to simulate the 100-year, 10-day rainfall event. This rainfall event uses a nested Atlas-14 distribution and has a rainfall depth of 10.1 inches. The 10-day rainfall event was used to match the inflow hydrograph taken from the watershed-wide XPSWMM model. The 10-day rainfall event has been assumed to be the critical duration event for Minnehaha Creek. The critical duration event for a watershed depends on the size and flow pattern of the watershed. Typically, a larger watershed will have a longer critical duration event. Additional recorded rainfall events were incorporated to the models during additional model refinement processes.

ICM takes in rainfall data as a time series dataset. The data can be entered by a user or imported by a CSV file. The timestep for the rainfall data can be changed to match the required timestep. The rainfall data is entered using inches per hour units. Once a rainfall file is utilized in a simulation run, the rainfall data cannot be edited.

ICPR has 18 non-dimensional rainfall distributions built into the software that only require a rainfall depth and storm duration to be specified. These distributions do not include MSE-3 so they are not relevant for this model build. Custom rainfall data can be added to ICPR in a variety of formats and storms can be applied globally or in local rainfall zones. Custom rainfall data can be input from a historic rainfall event, a dimensionless rainfall distribution, or a constant rainfall rate. Rainfall depth is measured in inches in ICPR. Rainfall files must come in a txt file format with tab delimiting, this is usually done by saving a CSV file as a txt file then moving the folder containing both files into the Resources>Rainfall directory in the ICPR file structure. Sample formats for custom rainfall files from the ICPR Help System are included below to show the differences for each. Rainfall data can be edited in its source file at any time a simulation is not running. Figure 3 details the rainfall information from the ICPR help menu.

ICM uses rainfall intensity as the input for the hyetograph while ICPR uses rainfall depth as the unit for the rainfall data. ICPR takes a wider variety of formats for custom rainfall data and allows for data editing after a simulation run. ICM does not allow dimensionless rainfall hyetographs as input while ICPR does.

Historical Rainfall Event [top](#)

An example of an historical rainfall file in English units is shown below. This file is set up in absolute time (e.g. the year, month, day and hour are specified). If you prefer to work in relative time, then set all year, month and day values to zero and use cumulative hours from the onset of the simulation. The time values correspond to the beginning of a rainfall packet. For example, beginning at year 2012, month 4, day 7, and hour 4, 0, 1.125 inches of rain falls evenly over the next 60 minutes (the rainfall packet time increment). Entries are not required for zero rainfall packets. The last record must contain the word "END" in uppercase.

```

0
60
2012 4 3 23.000 0.001
2012 4 6 6.000 0.002
2012 4 7 2.000 0.033
2012 4 7 3.000 0.911
2012 4 7 4.000 1.125
2012 4 7 5.000 0.566
2012 4 7 6.000 0.093
2012 4 18 16.000 0.035
2012 4 18 17.000 0.013
2012 4 18 18.000 0.006
2012 4 18 23.000 0.001
2012 4 19 9.000 0.015
2012 4 19 11.000 0.043
2012 4 19 16.000 0.192
2012 4 20 23.000 0.004
2012 4 21 0.000 0.002
2012 4 21 12.000 0.001
2012 4 21 13.000 0.001
2012 4 21 15.000 0.010
2012 4 21 17.000 0.012
2012 4 21 18.000 0.003
2012 4 21 19.000 0.005
2012 4 21 22.000 0.007
2012 4 21 23.000 0.010
2012 4 22 0.000 0.007
2012 4 22 1.000 0.050
2012 4 22 2.000 0.082
2012 4 22 3.000 0.194
2012 4 22 4.000 0.010
2012 4 22 8.000 0.010
2012 4 22 9.000 0.001
2012 4 22 11.000 0.003
END
    
```

Annotations in the image point to: File Type (0), Rainfall Packet Time Increment (60), Year (2012), Month (4), Day (3), Hour (23), and Rainfall Amount (0.001).

Dimensionless Rainfall Distribution [top](#)

An example of a dimensionless mass curve (English units) is shown below. The file type is set to 2 (or 3 for a metric units), the storm duration to 24 hours and the total rainfall amount to 10.6 inches. *Non-zero values of storm duration and rainfall amount in the simulation control form (global rainfall) override the storm duration and rainfall amount in the data file.* Or, if you are using multiple rainfall zones instead of global rainfall, then the storm duration and the total rainfall in the text file are multiplied by the non-dimensional distribution to obtain the temporal distribution of rainfall for the simulation.

These are followed by dimensionless time-rainfall pairs. The dimensionless time is equal to the cumulative time divided by the storm duration. And, the dimensionless rainfall is the cumulative rainfall divided by the total rainfall amount.

```

2
24.0
10.6
0
0.0417 0.00046
0.0833 0.00178
0.125 0.00451
0.1667 0.0089
0.2083 0.02026
0.25 0.03573
0.2917 0.06157
0.3333 0.09602
0.375 0.14664
0.4167 0.21059
0.4583 0.29448
0.5 0.4392
0.5417 0.64992
0.5833 0.74509
0.625 0.82
0.6667 0.87693
0.7083 0.91918
0.75 0.95158
0.7917 0.97342
0.8333 0.98623
0.875 0.99406
0.9167 0.99717
0.9583 0.99905
1
1
END
    
```

Annotations in the image point to: File Type (2), Storm Duration (24.0), Total Rainfall (10.6), Dimensionless Time (1), and Dimensionless Rainfall (1).

Constant Rainfall Rate [top](#)

It is possible to simulate a constant rainfall rate using a non-dimensional rainfall distribution. For example, a custom hydrograph was created for a constant rate of 7.5 inches over 24 hours. The following text file is placed in the rainfall resources folder.

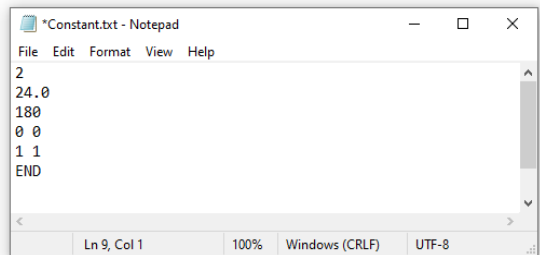


Figure 3. ICPR Rainfall Data Entry

2.5.2 2D Mesh

The 2D Mesh is a critical piece to accurately simulate inundation and flow direction. Multiple mesh parameters can be set to manipulate the cell size and orientation. Through the development of the mesh in both software packages, different mesh development pathways were tested.

In ICM, the maximum triangle mesh area was set to 1,000 square feet, minimum mesh element area to 200 square feet, and minimum angle to 30°. The mesh was built using the terrain sensitive meshing feature along with a maximum height variation set to 3.28 feet. Adjusting the height variation will increase or decrease the rate at which new triangles are created during the mesh development. ICM aggregates small triangles to create a single element that is above the minimum mesh element size during the mesh creation process. This feature adjusts the mesh layout and spacing within the previously user-defined parameters to introduce additional detail into the mesh in areas with high rates of elevation change. Breaklines and refinement regions can be added to a mesh to enforce hydraulically significant features within the elevation data. A breakline is a polyline that aligns the cell edges with the polyline alignment. Refinement regions are polygons that change the mesh spacing parameters to either introduce additional detail into the model or reduce detail by aggregating cells. The ICM model included a single breakline along the centerline of Minnehaha Creek created manually by the model user. This breakline was the same between ICM and ICPR. Additional discussion on mesh refinement and the effect on runtimes is included in the scenario analysis and calibration memos. Within both software packages, each mesh element is assumed to be at a single elevation that is assigned from the DEM by sampling the ground model and taking the average of the sample point elevations.

The 2D overland flow mesh can be created using terrain-sensitive meshing or through the placement of breakpoints. When using the groundwater module using breakpoints to create the mesh is recommended. The overland flow breakpoint layout must be transferred to the groundwater mesh to eliminate continuity issues that result in model crashes and negative aquifer errors during simulation runs. The overland flow breakpoints were placed in a triangular pattern with 100-foot spacing to create the base overland flow mesh. The groundwater mesh was then created using 200-foot triangular spacing to align the overland and groundwater meshes. The 2:1 ratio is recommended based upon the honeycomb creation of both meshes and transfer of water between the meshes. Water flows only along the edges of the triangular mesh and the orientation of triangle edges must follow the direction of flow. Breaklines were added along principal flow paths to align the overland triangular mesh with the direction of flow. The overland flow breaklines were transferred to the groundwater mesh to align the meshes to each other. The terrain-sensitive meshing option is not recommended for use with combined overland and groundwater models. After placement of the breakpoints and breaklines, the mesh must be preprocessed. The preprocessing allows for the review of short triangle edges and mesh build errors. Triangle edges less than five feet should be removed through the adjustment of breakpoint location and breakline alignment. There is a search tool within the ICPR graphical viewer to find short edges but the refinement elements must be hand edited to remove the short edges. Discrepancies between the overland flow and groundwater mesh refinement elements can cause model crashes and model errors during simulation runs.

ICM and ICPR can build their triangular mesh using similar methods, except that ICPR does not include a maximum height variation parameter. The ability to specify the maximum height variation in ICM allows for a larger range between minimum and maximum mesh size to better capture flat and hilly areas within the same overall parameter and less manual user input to refine the mesh later. While ICPR can use terrain sensitive meshing, it is recommended to use breakpoints to generate the base mesh and breaklines to refine the mesh further. While the creation of the base mesh through breakpoints is done using an internal ICPR tool, the refinement of the mesh further must be completed through user edits. ICPR requires that the overland and groundwater meshes be refined in essentially the same manner and encounters routing issues when the internal mesh creation tools are used. ICM and ICPR were able to run all simulations at the original 2D mesh range of 200-1,000 square-feet. ICM was able to reduce the minimum cell size to 20 square-feet and successfully complete the simulations. ICPR was refined through hand edits to reduce cell size and was able to complete the simulations. Both models benefit from the use of 1D objects such as pond volume and river control areas that remove portions of the overland flow model from the 2D calculation by using the 1D solver.

Both software packages encounter issues during the simulation when there are extremely short mesh cell sides present. Cell face lengths less than 5 feet should be adjusted to increase the length. Typically, the minimum length of a cell face is proportional to the maximum depth that a cell may experience by a factor of 10 (e.g. 1 foot of depth = minimum of 10 feet

of cell face length). This is a general rule of thumb and the larger the ratio between cell face length and maximum inundation depth in the cell, the more efficiently and stably a model will perform a simulation. During the mesh development, average and minimum element sizes can be reviewed through log files. The removal and/or adjustment of these small areas and lengths is highly recommended to improve simulation run times and accuracy of results. As a reduction in cell size is achieved, a similar reduction in time step must also occur for the models to perform simulations accurately and completely. Both software packages incorporate variable time steps within model runs. ICPR allows for specification of the range of time steps when using the Fireball solver whereas ICM will reduce the 2D timestep to complete the simulation in a stable manner. Both software packages will perform multiple calculations at a time step if needed to determine the flow and stage between 2D cells. When multiple iterations are needed to perform the calculation, overall model run times increase, sometimes significantly.

2.5.3 Boundary Conditions

2.5.3.1 Inflow Conditions

There were no inflow boundary conditions specified for the Turbid-Lundsten subwatershed model as this area is at the top of the watershed and no offsite flows enter the model area. A single inflow boundary condition was specified manually for the Edina subwatershed model at the upstream (west) end of Minnehaha Creek, where the creek enters the model area. The inflow boundary condition was taken from the watershed-wide XPSWMM model as an inflow hydrograph flow rate from the 100-year, 10-day storm event (Base Flood Elevation run). Using a flow hydrograph to introduce the creek flow produced better results compared to using a stage hydrograph to introduce flow. This was the case using both the base lidar for the channel mesh development and using the refined channel data for the mesh development.

ICM uses a line source to introduce the flow from the flow hydrograph into the model area. The line source is applied at the edge of the model along the channel cross-sectional area for flow to enter the model. A line source can be applied within the model area. Also, a point source can be used to introduce flow at a single point in the model area.

ICPR uses a line source to introduce flow from the flow hydrograph into the model area. The line source was applied inside of the model area due to the expanded size of the model boundary within ICPR. A point source within the model could have been used to introduce the flow at a single point within the model area, however, the line source option was used to maintain consistency between software packages and inflow points.

ICM and ICPR allow for external and internal boundary conditions to be specified for inflow to the models. The boundary conditions can be specified as either point or line inflow to mimic different types of inflow conditions. Upstream boundary conditions should be applied through external line features and be applied as flow hydrographs. Downstream boundary conditions should be applied through modifications to the 2D boundary and applied as stage hydrographs. Other boundary condition setups may be required for unique situations. During the creation of the inflow lines, small discrepancies in the alignment of the line can result in tiny 2D mesh elements. These small elements can increase simulation run times dramatically for both software packages. It will be important to review the minimum mesh size and remove tiny 2D mesh elements for the future watershed-wide build.

2.5.3.2 Outflow Conditions

The outflow boundary condition for the Turbid-Lundsten model area is free discharge at the downstream end of the MnDOT pipe that crosses Highway-5. There are multiple locations where pipes discharge from the Edina subwatershed and all of the pipe discharges were set to free discharge. The only user-specified outflow boundary condition is along Minnehaha Creek. The boundary condition was set to either the FEMA base flood elevation (BFE) or to free discharge depending on the scenario being modeled. The FEMA 100-year flood BFE level at the edge of the model is at an elevation of 861.

ICM uses level lines to introduce stage data. The level line is applied at the 2D Mesh boundary and overrides the previous 2D zone boundary type for the portion of the 2D zone that is colinear with the level line. For the simulation, the level line elevation was set equal to the BFE of 861 and held constant throughout the simulation time frame.

ICPR uses the same line and point elements to define outflow locations that it uses for inflow. Multiple stage boundary condition lines were used along the model boundary to allow runoff to flow out of the model. Minnehaha Creek boundary was set to 861 and all pipe discharge locations from both subwatershed models were set to free discharge.

ICM and ICPR allow for outflow boundary conditions in similar ways to be specified along the boundary of the mesh.

3 MODEL BUILD CHALLENGES

Challenges listed below are unique to each software package and should be considered in the evaluation matrix and the software decision.

3.1 ICM

If the District decides to move forward with ICM as the software for the full watershed-wide build, the following model build challenges should be considered and addressed:

Coordinates

Units are recommended to be changed from UTM Zone 15N projection to one that is based in the units of feet. ICM wants to revert to meters as the default units due to the underlying UTM projection. This causes issues when importing data through the ODIC as inches will be imported as millimeters and elevations as meters instead of feet. The imported data can be converted back to feet and inches through data manipulation within the internal ICM tables. The conversion marks all changed values with an “updated” flag and makes the model updating tricky to manage and track. Also, to export the results back to GIS, the overall model units must be switched back to meters to align with the underlying spatial projection of UTM. While this occurrence isn’t a fatal flaw within the model build and simulation run process, the necessity for a streamlined and user-friendly process is key for long term success of the watershed-wide model build.

Mesh Size

Determining proper mesh size will be critically important to successfully implement the watershed-wide model. The mesh element elevation is calculated by taking the average elevation of multiple sample points. In areas where large variations in elevation occur in a relatively short distance, multiple mesh elements are required to accurately simulate the change in topography. Though the size of the mesh elements must be carefully adjusted as the size of the element decreases, the number of computations at each time step that the software must complete increases. The reduction in element size will also increase time for post-processing results and increase size of storage requirements. There are multiple ways that ICM allows for mitigation of this challenge. By using the terrain-sensitive mesh generation technique, a larger range between minimum and maximum element sizes can be specified. Determining the correct size of mesh will be important to developing results at a scale that is desirable for the final use.

1D Datasets

All pipe and node parameters are read directly from the pipe/node data, no information is taken from the DEM during the import process. As a result of this fact, the pipe outlet nodes must either have an elevation from as-built/survey or be recalculated based upon the associated DEM.

Following the pipe import process, pipes were discovered that the scripting process missed the conversion of diameter data with the “ suffix for inches in the pipe size to be solely numbers (from 30” to 30). This process was remedied short-term by parsing the data column with the “ inputs and reloading the data into the model. Th date. The pipe data should be parsed to remove inch (”) data prior to incorporating with the preprocessing tools.

When GIS data has duplicate pipes (i.e., pipes that start and end at the same exact points), neither pipe is imported into ICM. This is due to conflict with placement and no hierarchy being placed on the pipes during import. The missing pipe following import occurred once during the Edina subwatershed model build and did not occur during the Turbid-Lundsten subwatershed model build. The missing pipe was found by using the GIS Layer Manager to bring in the pipe data as a background file and verifying that all pipes were imported. This is a quick process but a necessary one to verify the automation.

Overland Flooding

An issue with the representation of the overland flooding results within ICM was discovered during the initial analysis runs. The representation was reviewed with the staff at Innovyze and modifications to future releases of ICM will include changes to how the representation is shown. The underlying results are accurate but the triangulation between large mesh elements

can be overwritten during the visual representation process and remove areas of inundation. Typically, these areas are located along the edges of the inundation and are visually apparent due to their rectangular nature. The short-term solution is to either reduce the mesh size in areas with large changes in elevation or view the results within the 2D model (triangles), not within the flood map representation (internal raster). Following review and discussion with Innovyze on the issue, it was determined to not be significant since the results are computing correctly and can still be viewed within the 2D model viewer. A results raster can also be created outside of ICM through geoprocessing tools in GIS.

1D-2D Interface

The ICM model encountered some model instability issues during the initial testing. These instabilities caused the simulation to crash within the initial 5% of the run time. The instabilities were most commonly due to 1D-2D connection elevation variances at the end of pipe runs. ICM creates a note during the validation process when there is a discrepancy between the downstream invert in a pipe and the 1D-2D node elevation but can still run the simulation. There are three main causes for this issue to occur during the model-build process and the resulting simulation runs.

1. A pipe discharges below the water level of a creek or pond. This can be due to Lidar's inability to penetrate water, resulting in a falsely high bottom elevation
2. Lack of data within the pipe dataset. This leads to third issue.
3. Pipe invert data being filled through the automated process. Pipes with inverts that do not have data are automatically filled with a DEM offset from the user input parameter. This new downstream invert is then set below the DEM and will need to be adjusted during the model-build and verification process.

3.2 ICPR

In the event the District selects ICPR for the full model builds, the following model build challenges should be considered and addressed:

Coordinates

All component data should be projected into the desired coordinate system and units before importing the data into ICPR. This reprojection was completed during the data processing automation. Failure to do this will result in elements of the model being misplaced. Units cannot be changed within ICPR, so any data with units not converted beforehand will need to have their data manually edited in ICPR after they've been imported. ICPR needs all 1D model elements to be imported as .csv files to maintain all of their input data shapefiles will only import location and connectivity data.

1D Datasets

ICPR determines connectivity in the 1D network based on data within the pipe so when data entry errors are present or name data is missing, ICPR will not be able to connect pipes to nodes regardless of spatial relationship. When these errors occur, the modeler must manually define the names of upstream and downstream connections for each pipe.

Following the pipe import process, pipes were discovered that the scripting process missed the conversion of diameter data with the " suffix for inches in the pipe size to be solely numbers (from 30" to 30). This process was remedied short-term by parsing the data column with the " inputs and reloading the data into the model. Preprocessing the pipe data to consolidate the pipe size attributes will also aid the model develop process to highlight areas that require additional information to fill the gaps created through a lack of pipe size data.

All pipe and parameters are read directly from the pipe data, no information is taken from the DEM during the import process. As a result of this fact, the pipe outlet nodes must either have an elevation from as-built/survey or be recalculated based upon the associated DEM. Node elevations can be manually specified but ICPR will automatically adjust them to the DEM if they are not manually specified.

When GIS data has duplicate pipes (i.e., pipes that start and end at the same exact points), both pipes are imported into ICPR. This causes a fatal error when the model tries to run. This error can be solved by deleting the duplicate pipes from ICPR. The missing pipe following import occurred once during the Edina subwatershed model build and did not occur during the Turbid-Lundsten subwatershed model build.

ICPR allows for creation of 1D nodes and 1D interfaces, these entities are separate types that cannot be swapped out. The 1D nodes live within the hydraulic network and are full 1D entities. The 1D interfaces are 2D features that connect the 1D pipe network and the overland flow region. To swap a junction from a 1D node to a 1D interface, the junction must be deleted from one dataset and created in the other by hand. 1D interfaces assume that the starting water surface elevation is at the ground elevation. This assumption is accurate for culvert inlets/outlets as they are typically dry at the start of a simulation. This assumption is not accurate for surface inlets (catchbasin) as this condition will assume that the manhole/structure is completely full of water at the start of the simulation. The starting elevation for a 1D interface must be specified to be at or below the connecting pipe invert to remove this issue.

Boundary Conditions

The ICPR model encountered instability when boundary conditions were not applied directly to the model boundary, when nodes were left in the model that did not attach to any pipes, and when inlets are placed very close to stage boundary conditions (less than 10 feet in Edina). The combination of these errors results in the model terminating at the first major rainfall (120 hours into the simulation). Fixing the boundary condition to conform exactly to the model boundary and removing pipe inlets close to the downstream stage boundary condition stopped errors from crashing the model. Further cleaning up the model to remove unconnected nodes and altering the boundary condition to ramp up with the rainfall event greatly improved runtime.

Groundwater Model

A fully functioning overland flow model must be created prior to creation of the groundwater model. All edits to the overland flow model must be transferred into the groundwater model, including breakpoints, breaklines, and refinement areas. The groundwater model uses a matrix solver to complete the calculations for groundwater levels and flow. Due to the matrix solver being used, the size of the individual groundwater meshes begins to reach a practical limit around 12,000 groundwater cells within a single groundwater mesh. Multiple groundwater meshes can be used within a single model but the interface line between groundwater regions must be wet (e.g., a lake, pond or creek) to allow flow across the boundary. At a minimum, groundwater mesh elements should be the same size as the overland flow elements. However, the model creator recommends groundwater mesh elements be set at a 2:1 ratio due to the groundwater mesh creation methodology and the interaction between the overland flow and groundwater meshes. For example, a model with surface base mesh size set at 500 square-feet would have a 1,000 square-foot groundwater base mesh at a factor of 2x. Multiple groundwater regions can be created and interact with a single overland flow region.

Model Instability

The 2D flow methodology of ICPR that only allows flow along the triangle faces of the overland flow region significantly impacts the model run times and stability when the faces are not aligned with the direction of flow. The issues become increasingly problematic when triangle faces are misaligned in areas of significant flow (ie. creek flow). Aligning the triangle faces with principal flow paths is accomplished through the creation of breaklines. The breaklines can be created within ICPR or through external GIS applications. For the pilot model build, the breaklines were created within ICPR. When considering a watershed wide build, the recommended approach is to create breaklines in GIS for the. Creating the breaklines within GIS allows for multiple users to create breakline shapefiles that can be joined into a single large file for incorporation within ICPR.

All breaklines created within the overland flow region should be transferred (copied) to the associated groundwater flow region for the area. Breaklines should also be snapped to vertices of other breaklines to reduce the occurrence of short triangle faces that need to be fixed during the model build process. A model can be run with short cell faces but there is a high likelihood that the simulation will crash or errors will occur during the simulation run that force the model to stop the simulation.

4 PILOT MODEL CALIBRATION AND VALIDATION

Since the outset of the project, the team had discussed calibration and validation of the model builds as a critical step towards evaluating each model's ability to meet the Districts defined goals for the updated watershed-wide model. These two important steps in the evaluation process are discussed separately in the Calibration and Model Scenario Analysis Memorandums, respectively.

APPENDIX D – MODEL CALIBRATION REPORT

2D Pilot Model Build Model Calibration Report

Prepared by:
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Prepared for:



MINNEHAHA CREEK
WATERSHED DISTRICT
QUALITY OF WATER, QUALITY OF LIFE

Date:
April 2023

Kimley»»Horn

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Document History

| Date | Version | Description of Change |
|----------------|---------|-----------------------|
| March 2023 | 1.0 | Initial Draft |
| April 2023 | 2.0 | Revised Draft |
| April 19, 2023 | 3.0 | Final |

Definitions

- ArcGIS – a desktop and cloud solution provided by Esri for GIS analysis, storage, data management, and data processing
- Calibration - the process of checking a measuring instrument to a referenced standard. For the purpose of this project, the instruments are the ICM and ICPR models and the referenced standard is a measured watershed response condition. The process involves running the model to produce results that can be compared to known, measured, watershed responses and then adjusting the model physical structure and/or input parameters to a point where the model results are accurate when compared to one or more measured responses.
- DEM – Digital Elevation Model, represents ground surface in a grid (raster) format. Elevations are assigned to each individual grid cell
- GIS – Geographic Information System
- ICM – InfoWorks ICM software package developed by Innowyze to perform 1D and 2D hydrologic/hydraulic simulation modeling
- ICPR – Interconnected Channel and Pond Routing (ICPR) software package version 4 developed by Streamline Technologies to perform 1D and 2D hydrologic/hydraulic simulation modeling
- Shapefile – Spatial data file format that includes attribute data for individual shapes. May be in a point, line, polygon format and includes the file extension “.shp”.
- Validation - process that provides assurance that a system (i.e., model) consistently provides results within the acceptable criteria. For the purpose of this study, validation refers to comparing the calibrated model results to other generally accepted model results or other watershed response conditions that do not have supporting measured data.

1 INTRODUCTION

The Minnehaha Creek Watershed District (MCWD or District) began the process to select a model platform for their upcoming watershed-wide build by completing a cursory assessment of the full range of two-dimension (2D) modeling software systems currently available. This screening-level assessment, along with vendor information sessions and consultation with agency experts, led the District to narrow their focus to ICPR and ICM. The District chose to pursue a pilot model build effort focusing on these two models in advance of the full watershed-wide build.

Previous memorandums prepared throughout the course of this pilot model build project have described the processes to prepare data and build the models and have demonstrated the need to develop a new modeling tool that has greater granularity to answer more specific questions, such as characterizing and quantifying the impacts of climate change. The District desires a model that can evaluate both low water and high-water conditions, along with the ability to evaluate event-based storms and longer-duration runs, to inform decisions relating to climate adaptation strategies, programmatic policies, and specific project impacts. In addition, the District desires to have a higher level of confidence that the results of a selected model will be consistent with (within some level of accuracy) the measured and monitored responses observed in the watershed. An important factor in gaining that confidence comes from having a model that can be calibrated to a known watershed response, or better yet, multiple known responses. The focus of this memorandum is on the calibration process for both models within the Edina geography and how the calibration process completed during this pilot model build can inform the expectations for calibration needs and model build efforts for the watershed wide model build. The models constructed for the Turbid-Lundsten corridor were not addressed during this calibration effort.

This memorandum provides an overview of input datasets, model build process, parameter manipulation, and challenges that were uncovered during the calibration process for each software package. The results of the calibration process have demonstrated that both models can be calibrated to within generally accepted calibration tolerances. The information obtained through the calibration process provides additional critical insight and data into the benefits and challenges each modeling platform presents relative to meeting the District's goals for selecting a watershed-wide modeling platform. The specific model versions used for this pilot model process were:

- ICM version 2023.2.0 with an unlimited license; and
- ICPR version 4.07.08 with an expert license.

2 CALIBRATION OBJECTIVES

The primary goal of calibrating the pilot study models is to further inform the model selection criteria and evaluation matrix. Characterizing each model's ability to replicate measured results and meet the defined calibration tolerance targets is a foundational step that builds confidence in their abilities to simulate future events and scenarios. Furthermore, determining the ease of use in terms of calibration and model adjustment is critical to the sustained usefulness of the model.

Recognizing that hydrologic and hydraulic models are mathematical representations of the physical environment, obtaining a calibrated model is highly dependent on the quality of the data the mathematical expressions use to produce model results. Because it is not possible to obtain all information and data on the actual physical environment, assumptions must be made to complete the model build process. These assumptions represent the physical conditions and qualities within the model and are defined as parameters that are defined and adjusted by the modeler. Changing the parameters allows for manipulation of the results until an acceptable output result is reached. Model calibration almost always requires the adjustment of one or more parameters to align the modeled outputs with the recorded/measured conditions.

Model validation is the process of backchecking the final calibrated model(s) against additional historical records (or other best available data). Typically, the information used in the validation process differs in intensity, timing, and/or length from the calibration events. For example, if the calibration was performed to monitored smaller rainfall depth storms, the validation events can be longer duration, high intensity, or seasonally different event. In general, the more diverse or wide ranging the intended uses of the model are, the more numerous and wide-ranging the validation events should be. In most cases, the reality is that validation process uses the best available data which may be from monitored events, other accepted models, or even photographs showing observed high-water levels for a known event.

2.1 Calibration Metrics and Tolerances

Before the calibration process was started, the project team evaluated potential calibration metrics and corresponding calibration tolerances to help guide when the calibration process reached an acceptable level of accuracy. Primary and secondary metrics are shown in **Table 1** and the calibration tolerances for each primary metric are shown in **Table 2**. The primary metrics of stage, R-squared, standard deviation, and continuity error are numerical results that can be pulled directly from the model or calculated from model outputs. Secondary metrics are essentially graphical observation of hydrograph plots. They represent visual observations of how well the model hydrograph matches the monitoring results. For this pilot study, observations include a visual comparison of how well the peak modeled elevation matches the peak monitoring elevation or the accepted/published flood elevation data for both models and how much the response changes for the ICPR model with and without the groundwater mesh in the model.

Table 1. Calibration Metrics

| Category | Metric |
|-----------|--------------------------------------|
| Primary | R – Squared (Stage) |
| | Standard Deviation (Stage) |
| | Continuity Error (Volume) |
| | Stage Difference (Average, Peak) |
| Secondary | Groundwater Influence |
| | Flood Inundation Levels (Peak Stage) |

The metrics listed in **Table 1** are further defined as follows:

- R – Squared represents the proportion of the variance between a modeled and measured value. High R-squared values represent a higher level of confidence in the results mimicking the observed system. For the model results in this pilot study, R-squared value is based on the model stage results.
- Standard Deviation relates to the differences in the stage (in feet) between the recorded monitoring data and model simulation results. The smaller the standard deviation value, the closer the simulated results are to the recorded data. A high standard deviation indicates the model results are more spread out compared to the recorded data.
- Continuity Error (Volume) is the total error that occurs within the simulation process due to non-convergence and estimation within the equations used to calculate stage and flow within a model. The continuity error is a measure of the total volume and due to computational processes in a model, takes the form of either additional volume that is introduced to the model area or a reduction of volume.
- Stage Difference. Stage corresponds to a measured water level in the pond, storage area, creek, or river. For the model results in this pilot study, stage is expressed in feet of elevation in the NAVD 88 vertical datum. The average metric is the average difference calculated over the full model run time and indicates whether the data overall are higher (positive result) or lower (negative result) than the average stage. We want to see both a lower standard deviation and a lower average stage difference. The peak value is the difference in the peak elevation for the full run time, where the peak model result may occur at a different time than the recorded peak.

The primary metrics are gaged against industry standard tolerances for calibration and validation of hydrologic and hydraulic models. The tolerances allow for comparison between modeled events and the corresponding recorded event data. Tolerances are used to determine the level of fitness to the recorded data. The tolerances that correspond to level of fitness for a calibrated model will shift depending on the length of the simulation, severity of the storm, and size of model. Generally, the longer duration, low-intensity storm events will require a higher level of fitness to be rated higher (i.e., Very Good) than a short-duration, high-intensity storm event. **Table 2** outlines the sliding scale of fitness that was applied to the modeled results for the long duration events. A search for comparable tolerances for short-duration events did not yield usable

information. Using the long-duration tolerances for the short-duration events provides a conservative fitness rating for the short-duration events.

Table 2. Primary Metric Calibration Tolerances for Long-Duration Events

| Metric | Rating | Tolerance Level | Description |
|---------------------------|---|-----------------|---|
| R - Squared | Poor | 0.60 | A higher RSQ signifies that as the recorded data increases in value, the simulation data also increases in value, and increases at a similar magnitude. |
| | Fair | 0.70 | |
| | Good | 0.80 | |
| | Very Good | 0.90 | |
| Standard Deviation | Poor | 2.0 ft | The smaller the standard deviation value, the closer the simulated results are to the recorded data on average. If the simulated data fit the recorded data stage hydrograph perfect, the resulting standard deviation would be 0.0. |
| | Fair | 0.5 ft | |
| | Good | 0.1 ft | |
| | Very Good | <0.1 ft | |
| Continuity Error | Poor | > 5% | Lower continuity error totals indicate model stability, smaller volume addition/loss, and improved simulation times. |
| | Fair | 2% – 5% | |
| | Good | 1% – 2% | |
| | Very Good | < 1% | |
| Stage Difference | Metrics are reported as Average and Peak. | | As the stage difference reduces to a value of 0.0 feet, the recorded and modeled hydrographs become aligned to a greater degree. Both metrics are to be evaluated in combination with the other factors, with more emphasis on having a smaller average stage difference and lower standard deviation to represent a tighter overall fit between modeled and recorded data. |

3 CALIBRATION PROCESS

Success of a calibration effort is largely based on the availability of known results (i.e., recorded data) as a basis for model adjustments. The larger and more robust recorded datasets allow for a tighter calibration effort to be completed. The recorded data availability for the Edina subwatershed includes groundwater stage-time series data, Mill Pond outlet stage-time series data, Mill Pond outlet flow-time series data, 56th Street stage-time series data, and 5-minute (and select 1-minute) interval rainfall data for 2021 and 2022. Typically, model calibration is done using relatively small rainfall events as these are more common and a sufficient period of recorded data is available from which to base the calibration on.

The recorded data was used to create inflow hydrographs at the upstream limits of Minnehaha Creek in the pilot model and rainfall hyetographs for the various storm events. The storm events included a July 2022 event (6 days), a Sept 2021 event (1.6 days), and the Summer 2021 event (77 days). Other published or accepted model results were reviewed and used as additional validation runs, including the city of Edina localized flood inundation mapping from the city’s XPSWMM model and the FEMA Base Flood Elevations (BFEs). These results are discussed within the subsequent scenario analysis technical memorandum.

3.1 Approach

The same general process was followed while calibrating both models. An overview of the calibration approach is provided below:

Step 1: Evaluate Base Model Performance

Calibration starts with understanding the correlation between the recorded data and the modeled results. The selected calibration events are run through the base model. Generally, discrepancies in runoff and discharge volumes have different origins than a discrepancy in high water levels. Understanding the basis of where the discrepancy is coming from is the first step to an efficient model calibration and validation process. The calibration datasets are needed to compare the pilot study model runs against recorded data. These datasets for river calibration need to include rainfall, downstream stage, and upstream inflow hydrographs. Additional stage and flow datasets throughout the study reach are beneficial to determine calibration quality and variability at multiple points along the river or creek reach.

Step 2: Adjust Physical Components

Incorporating better base physical watershed data (surface and infrastructure) should be the first consideration during the calibration process. Large discrepancies observed in step one is a strong indication that physical improvements to the model are likely needed. Updates to any surface feature, such as refined channel geometry, requires additional review of breaklines and mesh refinement to verify that they continue to represent the features that are hydraulically important within the model area. A shift in a channel thalweg that is not adjusted for correctly can remove the expected detail due to incorrect breakline placement based upon the new surface data. When breaklines and mesh refinements are done correctly, they can also reduce continuity errors within the model runs and reduce model run times by reducing convergence error occurrence.

Step 3: Reevaluate the Physically Adjusted Model's Performance

Once physical updates have been made to the model, a rerun of the simulation should be performed to determine the effect of better data on the modeled results. The physical updates can change the watershed response in many ways. Areas of inundation may appear or be reduced, and flow rates and flow paths can change drastically. The modeled results should be compared to the recorded data to determine the effect of the physical model updates and whether bias or performance issues still exists.

Step 4: Model Parameter Adjustment

If additional calibration is required, the next phase is to review the scale at which the calibration adjustments need to be made. If the modeled results match the curve of the recorded hydrograph but are off vertically, then an adjustment of the roughness values may be required to better match the recorded data. If the modeled results don't match the gaged hydrograph or are off by multiple feet, then a review of hydrologic parameters may also be required to better match the gaged data. There is a subset of model parameters that are typically utilized during calibration, which include:

- Hydraulic Parameters: Channel and Overland Roughness (Manning's n), Land-use delineations, Porous Polygons (Building Footprint Representation);
- Hydrologic Parameters: Initial Moisture Content, Hydraulic Conductivity (maximum and minimum), Pore Storage Volume (ICPR Only), Impervious Percentage;
- Groundwater Parameters (ICPR Only): Fillable Porosity, Hydraulic Conductivity, Leakage Conductivity.

Calibration of a model based on input parameters is typically accomplished through multiple iterations to gauge the effectiveness of the changes to the desired model output. It is not recommended to adjust multiple parameters during a single calibration run due to the general lack of knowledge during the result review process of which parameter facilitated the corresponding change in results. Changes to the soil parameters should be taken with caution as changing the hydrologic parameters may be adjusting for hydraulic differences that are not being represented within the model correctly. For example, if the depressional storage areas in the model are not being represented due to lack of detail in the elevation dataset or the 2D mesh, then adjustment of the infiltration parameters to include more infiltration will have a similar result in detaining runoff from the downstream system. However, when the storm event changes, the amount of infiltration that occurs will vary and may not represent the localized depressional storage that the parameter was originally adjusted to

represent, and the calibration will be inaccurate. Adjustment of the infiltration parameters needs to be done with caution and an understanding of the overall system response.

Step 5: Documentation

All physical and parameter adjustments within the final model version should be documented to provide a clear picture of: (1) where the model initially struggled to meet recorded data; (2) what adjustments were made to bring the modeled results closer to measured; (3) what limitations or bias remains; and (4) recommendations for additional calibration efforts in the future.

4 CALIBRATION RESULTS AND DISCUSSION

The calibration of both software packages starts with an evaluation of the base model’s performance against the measured data. Discussion in the next three subsections details the refinement and additional updates that were completed to achieve models calibrated to within the desired tolerance ranges.

4.1 Base Model Evaluation

The base model development used the infrastructure and spatial data that was created as part of the automated data development process. These datasets included pipe, structure, outlet, land use, soils, and elevation information. The automated data development processes used standard values for their outputs and included data gap filling for the infrastructure datasets. A single breakline was incorporated into both ICM and ICPR along the channel bottom of Minnehaha Creek to align the adjacent cells with the flow direction. Initial water surface elevations in the ponds and wetlands in the northern portion of the Edina Subwatershed were set based on aerial imagery and MnDNR data, where available.

The 77-day calibration event was run through the base model for ICPR and ICM. The modeled results for ICM and ICPR reflected results that were substantially higher than the gaged data as illustrated in **Figure 1**. ICM was able to simulate the increases and decreases of flow and stage within the creek but at a higher elevation while ICPR experienced extreme amounts of continuity error and was not able to represent the general hydrograph curve.

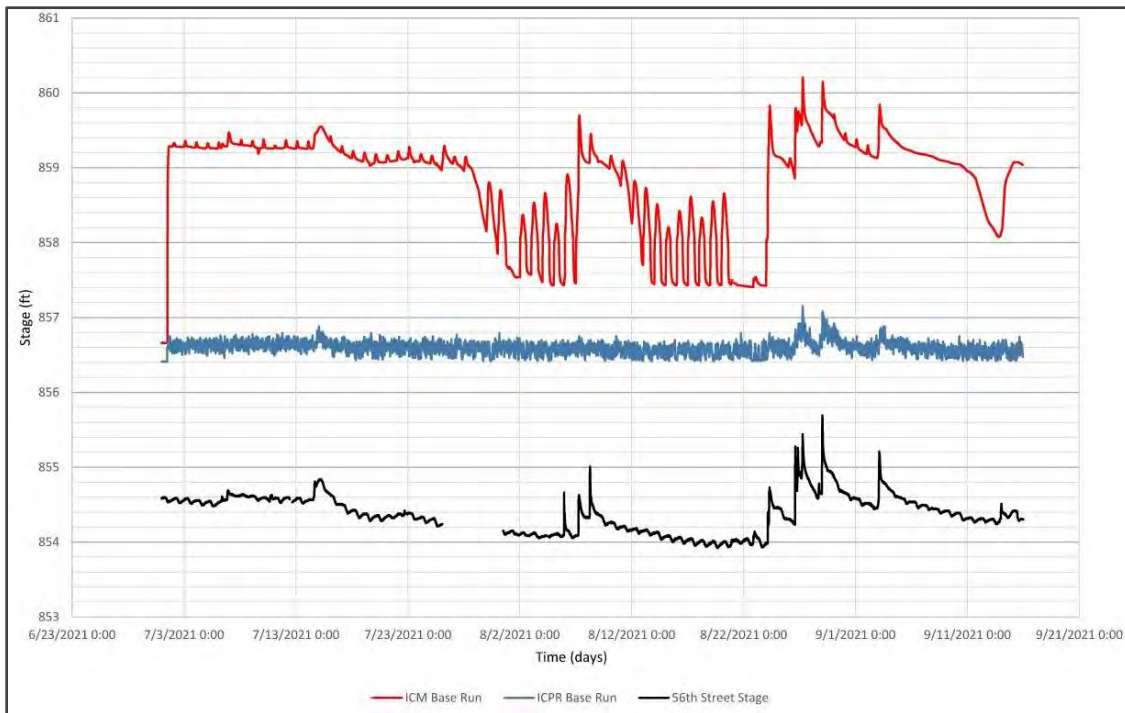


Figure 1. Base Model Comparison

The differences between the modeled and measured results within ICM and ICPR indicated that physical corrections to both models were possibly needed since there wasn't a practical way to reach the measured results through parameter adjustments. **Figure 2** shows the relationship between the ICPR mesh elevations and the DEM elevation when the cell sizes are too large to represent the flow capacity of the creek channel. The ICPR model overestimates the size of the channel while underestimating the elevation of the corresponding node. In this case, the lack of resolution within the 2D mesh allowed for an overestimation of lower elevation channel flow capacity. The location of the creek section shown in the graph is represented as a red line that crosses the creek channel perpendicular. Each step along the ICPR line represents the corresponding elevation that ICPR uses to perform the calculations. The result of this overestimation is the lack of response within the stage hydrograph from ICPR during the base model run.

The base model development used the best available data that could be incorporated through an automated process. However, the lidar data that was used to develop the elevation dataset was originally taken in 2011. In addition, the process of gathering the lidar points does not allow for elevations below a water surface to be obtained. This means that areas that contain water are set at the elevation of the water surface at the time the lidar data was collected, not at the bottom of the channel or pond. The effect of this is that a channel or storage area may have more depth, and therefore more volume capacity, when compared to conditions represented in the lidar data. The result is often a higher inundation level than the known or measured elevation since the model does not have the actual storage capacity for the runoff volume. Knowing this limitation LiDAR, in combination with the modeled results, it was deemed necessary to reconstruct the geometry of the channel and floodplain using alternative datasets.

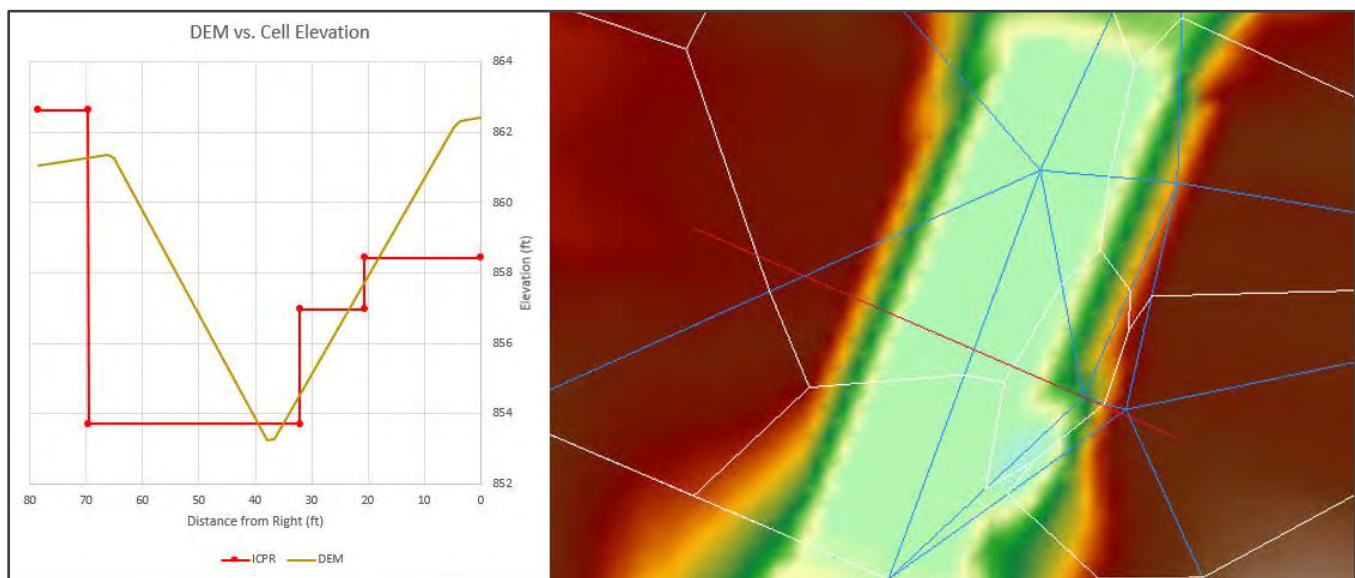


Figure 2. ICPR Mesh Comparison

Cross-sections for segments of the Creek were pulled from the District's current XP-SWMM model. Cross-section profiles were digitized as points across the channel, with the lowest elevation placed at the Creek centerline. These point profiles were repeated along the general length of the Creek as indicated by the XP-SWMM cross-section link.

For the Arden Park area geometry, the as-built elevation survey that was previously created as part of the District's work at Arden Park in the past 5 years was used to create the basis for the terrain. Elevations were captured both on land and at multiple transects across the creek. These elevation points were used to generate a TIN, which was combined in the TIN generated from XP-SWMM cross-sections. Some final clean-up of the combined cross-sectional and TIN surfaces was done to remove areas of incorrect triangulation during the meshing process. This final updated TIN was overlaid on the original lidar surface for the remainder of the Edina subwatershed model.

The impact of the modified terrain on the modeled results is described in the model specific calibration sections below.

4.2 ICM Calibration

Table 3 includes a summary of the primary metric results starting from the base model through progression of the calibration process. **Figure 3** provides a visual for the reported calibration event progression. Significant steps in the calibration process are summarized in the following paragraphs.

Table 3. ICM Long Term Run – Tolerance Results

| ICM Long-Duration Event | R-Squared (Stage) | | Standard Deviation (Stage) | | Continuity Error (Volume) | | Stage Difference (ft) | |
|-------------------------|-------------------|--------|----------------------------|--------|---------------------------|-----------|-----------------------|-------|
| | Value | Rating | Value (ft) | Rating | Value (%) | Rating | Avg. | Peak |
| Base | 0.62 | Fair | 0.497 | Fair | -0.04 | Very Good | 4.45 | 4.50 |
| Terrain Modification | 0.81 | Good | 0.113 | Good | -0.01 | Very Good | 0.54 | 0.62 |
| Mesh Refinement | 0.78 | Good | 0.128 | Good | -0.09 | Very Good | -0.18 | -0.04 |
| Final | 0.85 | Good | 0.138 | Good | -0.07 | Very Good | 0.05 | 0.61 |

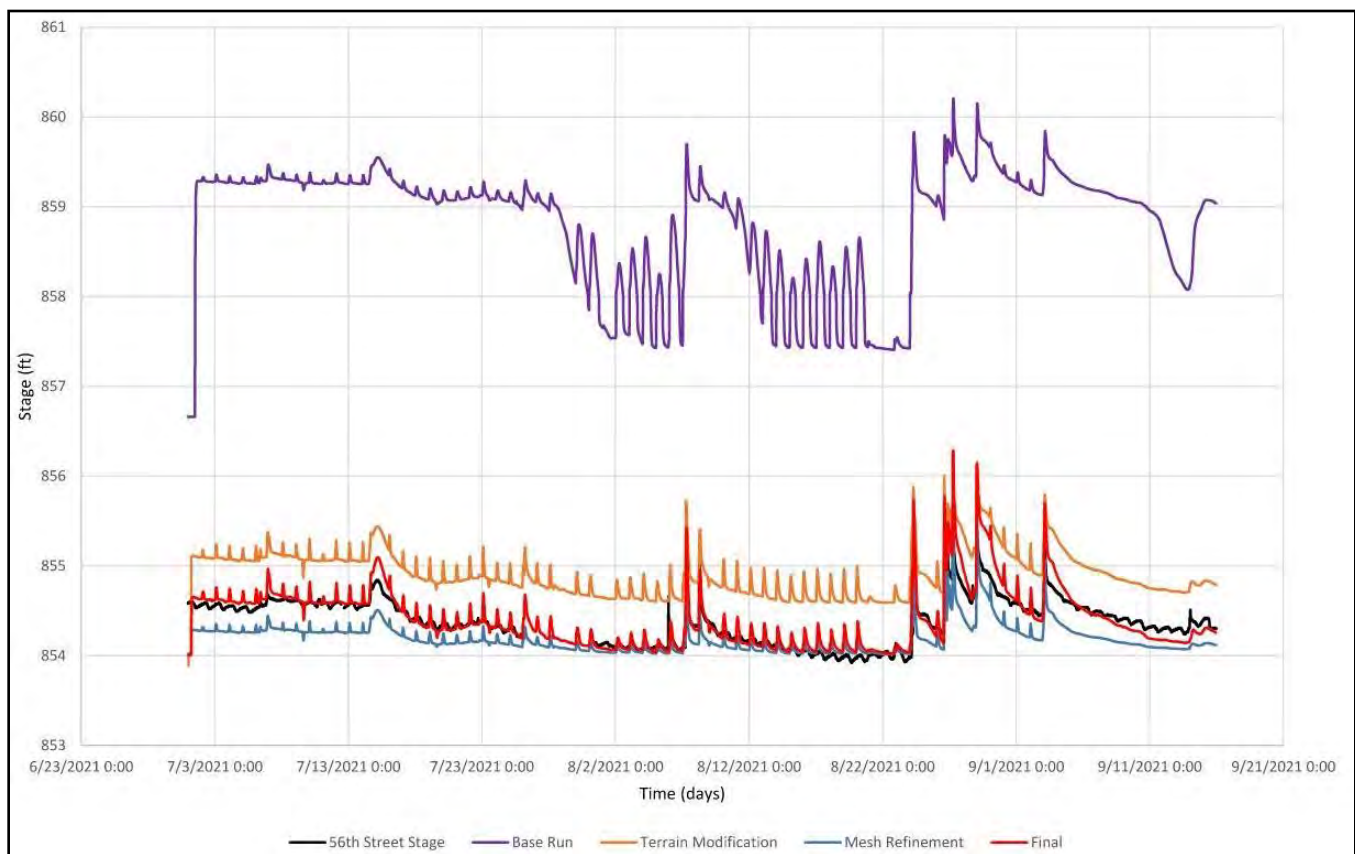


Figure 3. ICM Long Term Run – Calibration Steps

Terrain Modification: The modified-terrain version reduced the stage discrepancy to an average of 0.5 feet higher during the 77-day run, which was a substantial improvement from the base model that was 4.5 feet higher on average. The terrain modification also reduced the peak stage discrepancy between simulated and recorded data to 0.6 feet from 4.5 feet for the Base run. As part of the new terrain inclusion, a new breakline was delineated along the creek centerline to align the 2D mesh to flow downstream. The next step was to refine the mesh to increase the tolerance ratings shown in **Table 3**.

Mesh Refinement: The lowering of the minimum mesh element size was completed next as part of the ICM calibration process. The minimum mesh element size was reduced from 200 square feet to 20 square feet. The 20 square feet introduced excessive run times (30 hours). This was due to small mesh elements being located within the main channel bed of Minnehaha

Creek. The minimum mesh size was increased to 50 square feet. This increase continued to allow for additional detail along the banks of the creek and floodplain to be captured and represented more accurately while resulting in similar stage results along the section and reducing run times for the simulation to around 12 hours (similar to Base and Terrain Modification runs). The R-Squared results reduced slightly but remained within the good rating while the average and peak stage results decreased significantly.

Final: The final step of the ICM calibration process included the adjustment of Manning’s n roughness values. The values were increased from the standard to the higher values for the land-use types. This was done to increase the stage results within the creek. The results from the final adjustment increased the baseflow to be in line with the recorded data to a closer degree as shown in **Table 3**. The R-Squared value increased while the average stage difference decreased. The peak stage difference did increase back to 0.6 feet similar to the Terrain Modification run. Through all the simulation runs, the continuity error for ICM remained at Very Good rating level. **Table 4** lists the revisions to input parameters that were adjusted as part of the calibration process within ICM and ICPR. Within ICM, only the shallow and deep Manning’s n values were adjusted from the initial values to the final values in **Table 4** as part of the final calibration step.

Table 4. Revised Parameters (Initial vs. Final)

| Landuse | | Impervious ¹ | | Shallow Manning’s n | | Deep Manning’s n | |
|---------|--------------------------|-------------------------|-------|---------------------|-------|------------------|-------|
| Code | Description | Initial | Final | Initial | Final | Initial | Final |
| 100 | Agricultural | 0.00 | 0.00 | 0.100 | 0.100 | 0.070 | 0.090 |
| 111 | Farmstead | 0.00 | 0.12 | 0.050 | 0.050 | 0.050 | 0.050 |
| 113 | Single Family – Detached | 0.10 | 0.25 | 0.050 | 0.065 | 0.030 | 0.050 |
| 114 | Single Family – Attached | 0.10 | 0.38 | 0.040 | 0.065 | 0.030 | 0.050 |
| 115 | Multifamily | 0.50 | 0.65 | 0.040 | 0.060 | 0.030 | 0.040 |
| 120 | Retail/Commercial | 0.85 | 0.85 | 0.030 | 0.045 | 0.030 | 0.030 |
| 130 | Office | 0.85 | 0.85 | 0.030 | 0.045 | 0.030 | 0.030 |
| 141 | Mixed Use Residential | 0.50 | 0.85 | 0.030 | 0.045 | 0.030 | 0.030 |
| 143 | Mixed Use Commercial | 0.50 | 0.85 | 0.030 | 0.045 | 0.030 | 0.030 |
| 151 | Industrial | 0.50 | 0.72 | 0.035 | 0.060 | 0.030 | 0.040 |
| 160 | Institutional | 0.00 | 0.30 | 0.040 | 0.065 | 0.030 | 0.050 |
| 170 | Park/Open Space | 0.00 | 0.00 | 0.080 | 0.100 | 0.050 | 0.080 |
| 173 | Golf Course | 0.00 | 0.00 | 0.060 | 0.090 | 0.040 | 0.060 |
| 210 | Undeveloped | 0.00 | 0.00 | 0.090 | 0.100 | 0.080 | 0.090 |
| 220 | Open Water | 1.00 | 1.00 | 0.030 | 0.045 | 0.030 | 0.030 |

1. ICPR model adjustment only.

4.3 ICPR Calibration

Table 5 includes a summary of the primary metric results starting from the base model through progression of the calibration process. **Figure 4** provides a visual for the reported calibration event progression. Significant steps in the calibration process are summarized in the following paragraphs.

Table 5. ICPR Long Term Run Calibration Results

| ICPR Long-Duration Event | R-Squared (Stage) | | Standard Deviation (Stage) | | Continuity Error (Volume) | | Stage Difference (ft) | |
|--------------------------|-------------------|--------|----------------------------|-----------|---------------------------|--------|-----------------------|-------|
| | Value | Rating | Value (ft) | Rating | Value (%) | Rating | Avg. | Peak |
| Base – GW | 0.30 | Poor | 0.212 | Good | -16.1 | Poor | 2.22 | 1.48 |
| Terrain Modification | 0.87 | Good | 0.130 | Good | -7.69 | Poor | -0.29 | -0.47 |
| Mesh Refinement | 0.86 | Good | 0.094 | Very Good | +3.36 | Fair | -0.09 | -0.42 |
| Impervious | 0.83 | Good | 0.106 | Good | +1.24 | Good | -0.29 | -0.30 |
| Final | 0.83 | Good | 0.103 | Good | +1.46 | Good | -0.26 | -0.18 |

Terrain Modification: The modified-terrain version reduced the stage results to an average of -0.29 feet lower than the Base Run model during the 77-day run, which was a substantial improvement to the base model that averaging 2.22 feet higher. The terrain modification version also increased in R-Squared value and rating while the continuity error remained extremely high and poor in rating. The continuity error was a key issue that helped drive the success of the overall calibration effort while reducing the uncertainty of the results with the continued reduction in continuity error. One additional note on the Terrain Modification version is that while the visual fit of the model results shown in Figure 4 are the best fit to the actual monitored data, the continuity error rating required additional work to move the calibration metrics into an acceptable range.

Mesh Refinement: Refining the mesh was the next step at calibrating the ICPR model. Breaklines were added to the creek and overbank areas, where most of the error was occurring. It is critical in ICPR for the triangular base mesh to be oriented so that the faces of the triangular mesh align with flow direction. To align the edges of the triangular mesh with the flow direction, multiple breaklines were used to contour the mesh to the flow direction. This contouring is illustrated in **Figure 5** as well as the general increase in mesh cell refinement. The left image shows the original 2D mesh with the single breakline along the thalweg of the creek. The right image shows the refined mesh with the breaklines shown in red. The refined mesh has more cells, and they are better positioned to transition flow from upstream to downstream and across the floodplain area. Breakpoints were added following the breakline delineation in the overbank areas to mitigate triangulation issues between the refined area in the creek and the original mesh density outside of the creek channel. The inclusion of additional breaklines and breakpoints reduced error by an additional 63% from the updated terrain version. The mesh refinement also reduced the average stage difference to -0.09 feet while the peak stage difference reduced slightly when compared to the Terrain Modification model run. Graphically, the stage results in the creek decreased at the gaging location when compared to the Terrain Modification model run. The next step was to increase flow to the creek within the subwatershed with an increase in impervious values.

Impervious: The next step to increase the impervious value included adjusting the impervious values from the standard values to the higher-end values. This was done to facilitate additional runoff from infiltrating and instead runoff to the creek. The increase in impervious values increased the average stage difference while lowering the peak stage difference to -0.30 from -0.42 feet. The reduction in peak stage difference is due to the flashy or direct runoff nature of the subwatershed to the creek during an intense rainfall event versus the increase in average stage difference is due to a reduction in baseflow entering the creek channel from the groundwater portion of the model. The difference was surprising and was noted in the following Constraints and Future Recommendations section of this memo. To increase the stage levels in the creek, the Manning's n roughness parameters were increased next. **Table 4** lists the increase in impervious levels within ICPR as part of this step within the calibration process from the initial values to the final values for each land use type.

Final: The final step from the calibration was to increase the Manning's n roughness values. This was done in a similar manner to the ICM calibration process. The standard Manning's n values were increased to the corresponding high values to simulate increased resistance to flow and a higher stage in areas of flow. By increasing the roughness values, the water should move at a slower rate within the channel and increase in stage. The change in elevation resulting from the adjusted n value was minor compared to the desired result while the overall stage results remained within the acceptable range for the calibration process. The R-Squared value remained the same from the Impervious model run but the average stage difference decreased slightly, and the peak stage difference reduced by 60% as shown in **Table 5**. **Table 4** lists the increase from initial to final values for the Manning's n parameter. The increase from initial to final values were the same for both the ICM and ICPR models.

As a follow-up to the discussion in the Terrain Modification section, while the numeric values and rating of the calibration metrics all fall within the acceptable range, the visual fit of the Final version results shown in Figure 4 is clearly not as good as the visual fit of the Terrain Modification curve. We expect that in a full model build process, the visual fit could and would be improved through additional calibration adjustments.

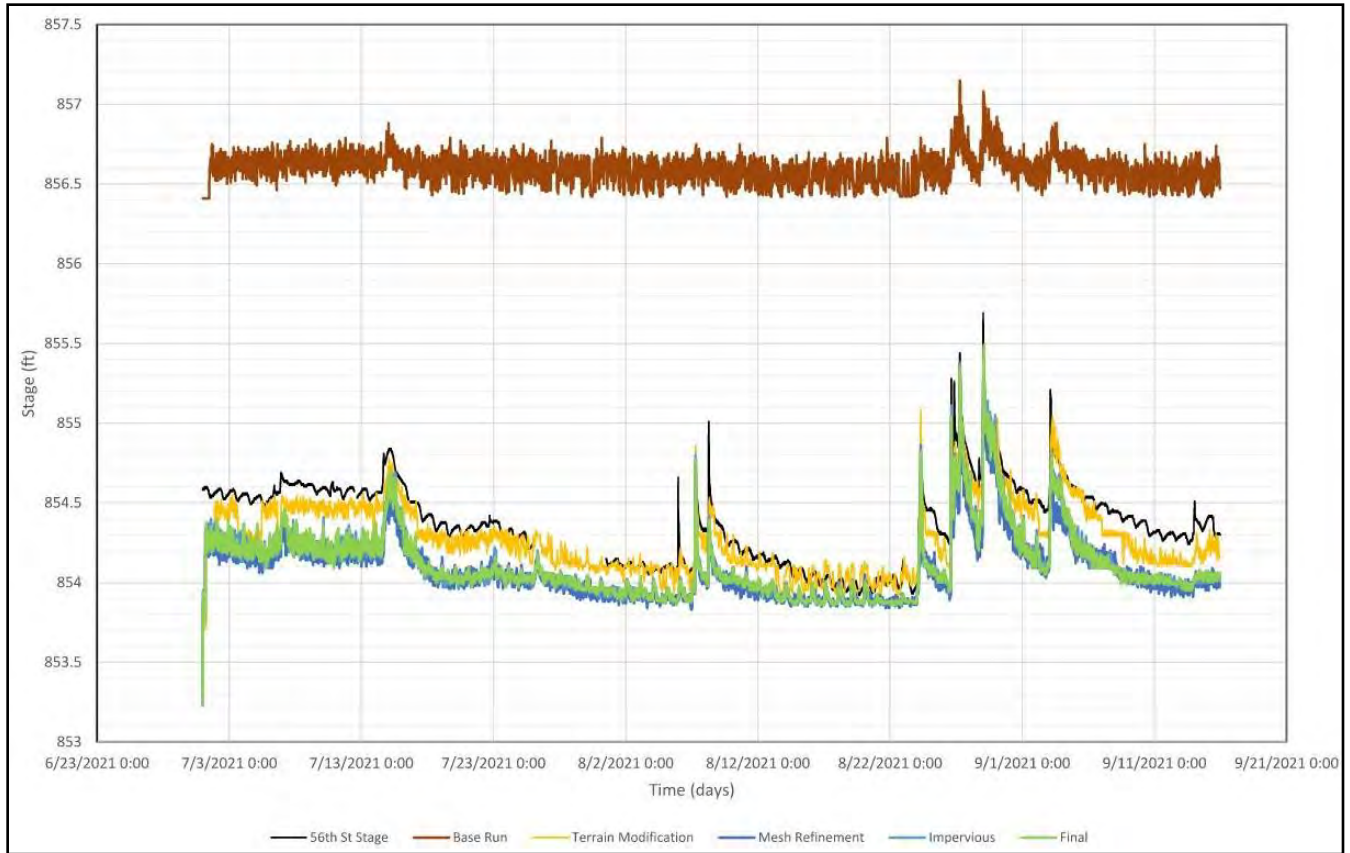


Figure 4. ICPR Long Term Event Calibration

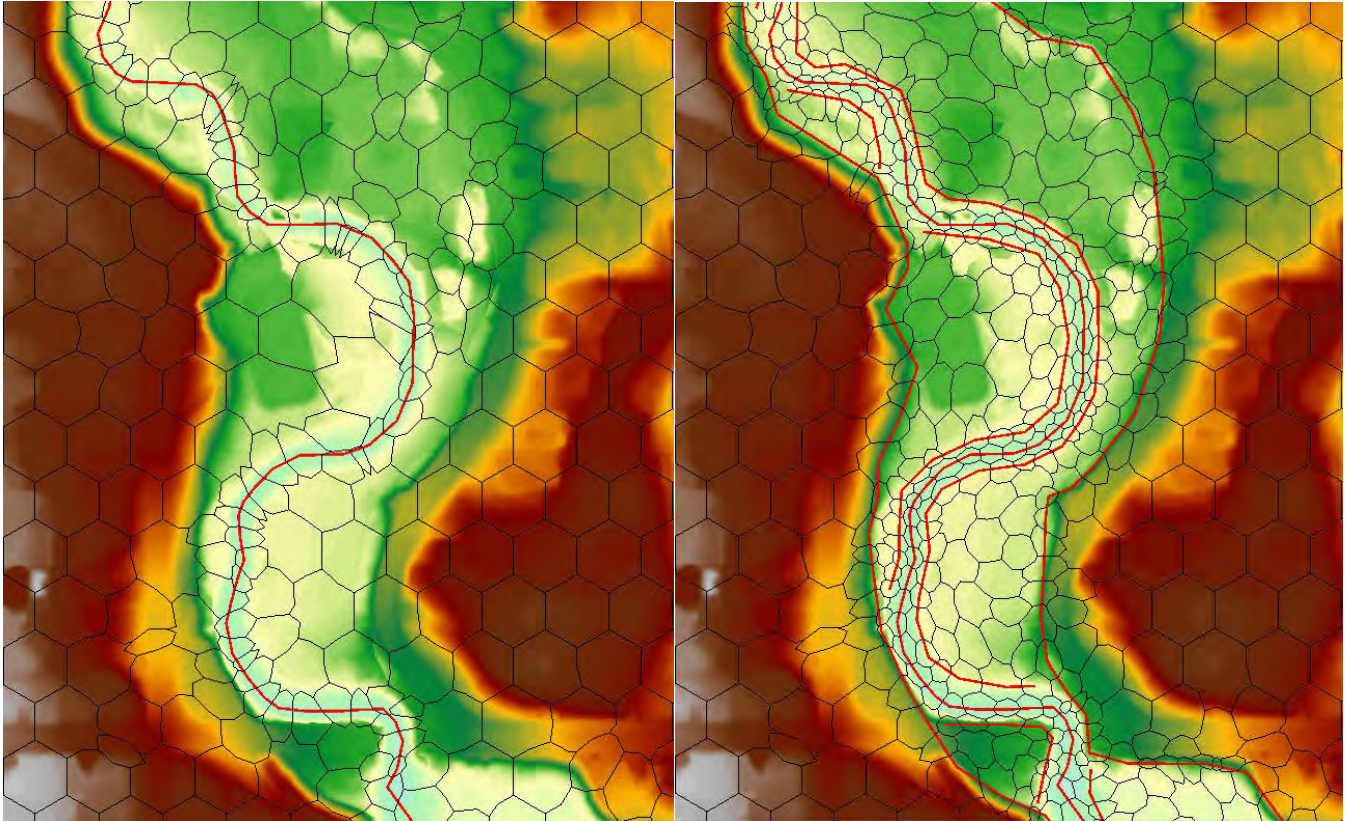


Figure 5. ICPR Mesh Refinement

4.3.1 ICPR Groundwater Component

ICPR's ability to model 2D groundwater mesh is a distinguishing feature from other 2D H&H models. To better understand the level of influence the groundwater component plays within the software, a non-groundwater version of the model was maintained to gauge the differences in results between the groundwater and non-groundwater versions. **Figure 6** illustrates the final differences between the groundwater and non-groundwater ICPR model versions. The largest discrepancy between the two versions is the two time periods during the month of August 2021 where the creek flow drops to zero in the non-groundwater version. The groundwater version demonstrates baseflow conditions within the creek during these periods of limited inflow upstream. The creek stage levels are also slightly higher with the groundwater version than the non-groundwater version. The groundwater component allows for transfer for infiltrated water back into the overland mesh during times of minimal flow in the creek.

During the calibration process, adjusting some parameters may have an adverse effect on the results than what was desired. This phenomenon was seen during the calibration of the ICPR model. Typically, increasing impervious values within the runoff portion of the model results in additional flow in the downstream receiving water body. There was a slight lowering of the downstream stage when the impervious values were increased within ICPR. This was theorized to be due to the groundwater module within ICPR. In a strictly 2D overland model, any water that is infiltrated into the ground is lost from the simulation. In ICPR with the groundwater module, the infiltrated water is allowed to accumulate and flow back into the overland system during times of low flow/stage within the creek. By increasing the impervious parameter in ICPR, the groundwater system was not able to recharge to the same degree as previously seen in earlier model runs. The resulting stage levels downstream were lower than previous iterations and had the opposite effect than anticipated.

The groundwater influence is greater during long-duration simulations than short-duration simulations. This is due to the relatively long time for groundwater to travel through an area versus surface water. The groundwater influence also becomes more pronounced during extended periods of wetness and drought. A high-groundwater table limits the amount of water

that can be infiltrated during future rainfall events. A predefined groundwater initial water surface can be defined within ICPR to mimic high and low groundwater levels for short duration events. Additionally, the groundwater module can be used to calibrate a model further but also introduces additional dials to turn. Having a strong understanding of how a system work is key before beginning to calibrate a model, the groundwater system is typically very difficult to understand at a small scale but can be represented well at a large scale. This means that analyzing scenarios to determine the effects of groundwater at a parcel/site scale may not be appropriate given the quality and robustness of the groundwater data that the results are based on but determining effects at a regional or larger scale may be appropriate.

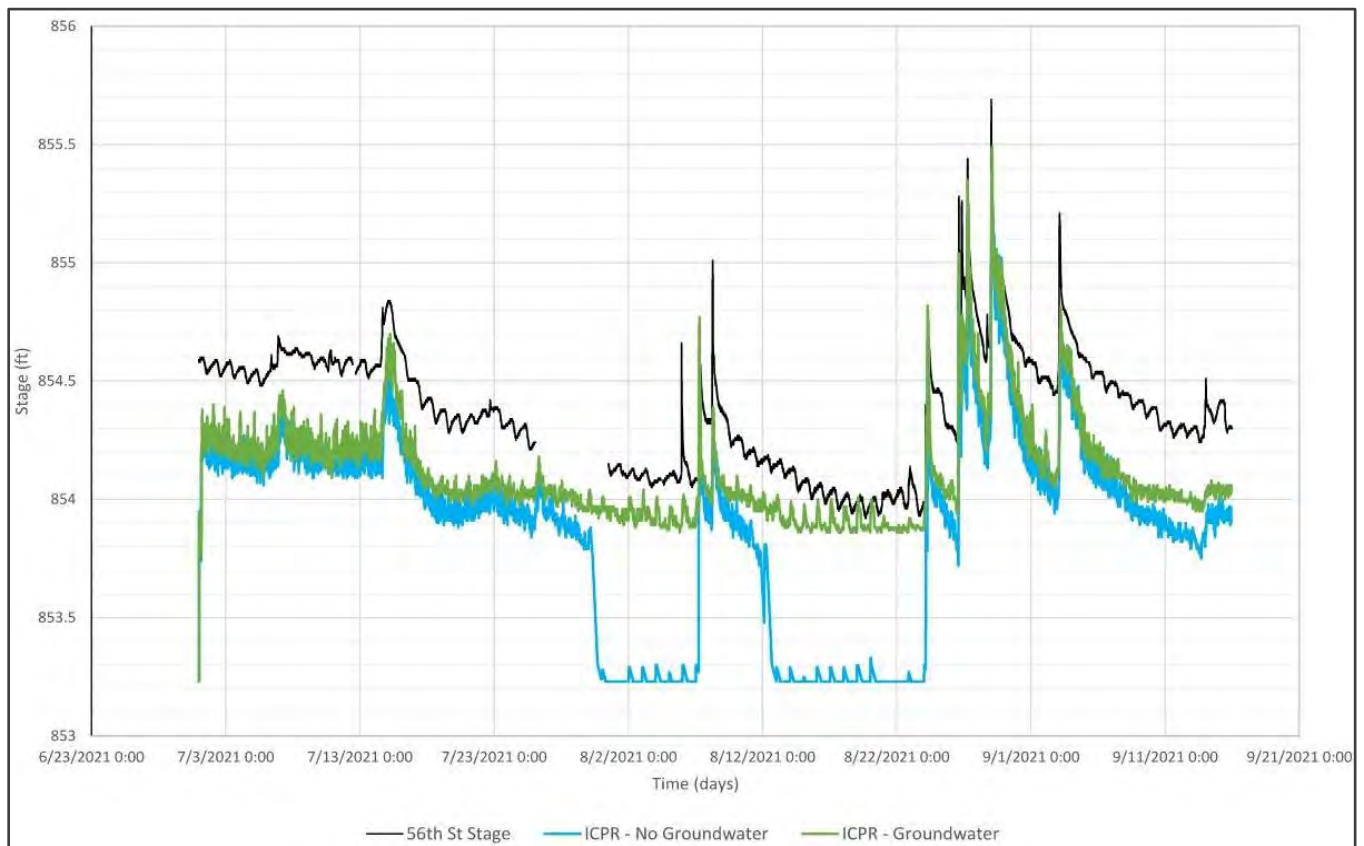


Figure 6. ICPR Groundwater Influence

5 COMPARISON AND CONCLUSIONS

As a core objective of the pilot model, it is important to compare the modeled results of ICM to ICPR and call attention to major differences. **Figure 7** shows the 77-day simulation for both calibrated models, **Figure 8** shows the September 2021 simulation results, and **Figure 9** shows the July 2022 simulation results. Both ICM and ICPR models shown on **Figures 7, 8, and 9** represent the final model versions that were developed through the calibration process. Throughout the calibration process, ICM remained generally above the recorded data at the 56th Street gage location. The baseflow followed the rising and falling sections of the hydrograph but the peaks during the individual rainfall events overestimated the increase in stage. This was seen in **Table 3** as the average stage difference reduced throughout the calibration process, but the peak stage results were overestimated continually. This trend continues in **Tables 5 and 6** where ICM continually overestimated the stage results in the short-term events as well. ICPR remained generally below the recorded data for the long- and short-term events as shown in **Tables 4, 5, and 6**.

ICM draws down at a slower rate after a peak stage than the recorded data and ICPR draws down much quicker than the recorded data. ICM also appears to drawdown at a consistent rate, where ICPR has a high degree of noise and doesn't seem to draw down in a natural manner. This noise, in contrast to ICM's smooth results, can make it difficult to interpret what is happening precisely.

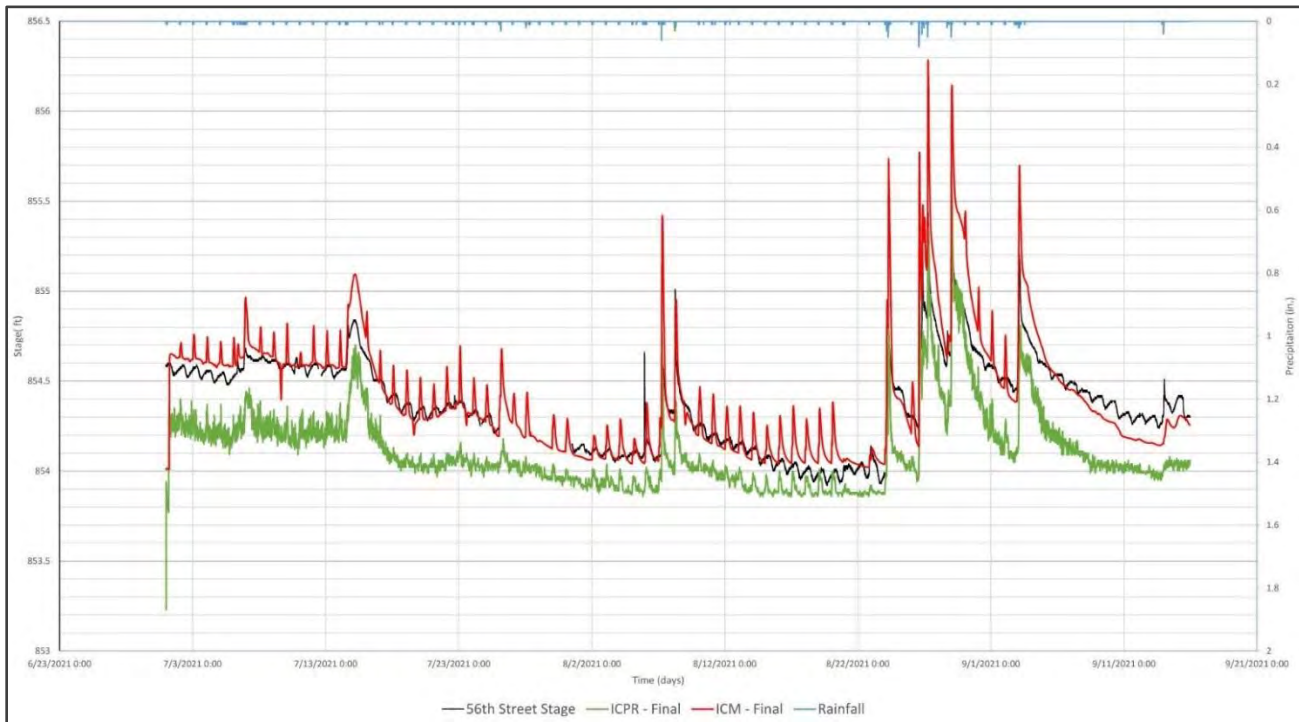


Figure 7. ICM – ICPR Long Term Stage Comparison

Table 6. September 2021 – Model Comparison

| September 2021 Event | R-Squared (Stage) | | Standard Deviation (Stage) | | Continuity Error (Volume) | | Stage Difference (ft) | |
|----------------------|-------------------|--------|----------------------------|--------|---------------------------|-----------|-----------------------|-------|
| | Value | Rating | Value (ft) | Rating | Value (%) | Rating | Average | Peak |
| ICM – Final | 0.680 | Fair | 0.351 | Good | +0.06 | Very Good | 0.06 | 0.43 |
| ICPR – Final | 0.559 | Poor | 0.287 | Good | -1.55 | Good | -0.35 | -0.43 |

Table 7. July 2022 – Model Comparison

| July 2022 Event | R-Squared (Stage) | | Standard Deviation (Stage) | | Continuity Error (Volume) | | Stage Difference (ft) | |
|-----------------|-------------------|-----------|----------------------------|--------|---------------------------|-----------|-----------------------|-------|
| | Value | Rating | Value (ft) | Rating | Value (%) | Rating | Average | Peak |
| ICM – Final | 0.894 | Good | 0.168 | Good | -0.01 | Very Good | 0.23 | 0.23 |
| ICPR – Final | 0.901 | Very Good | 0.106 | Good | +0.77 | Very Good | -0.15 | -0.22 |

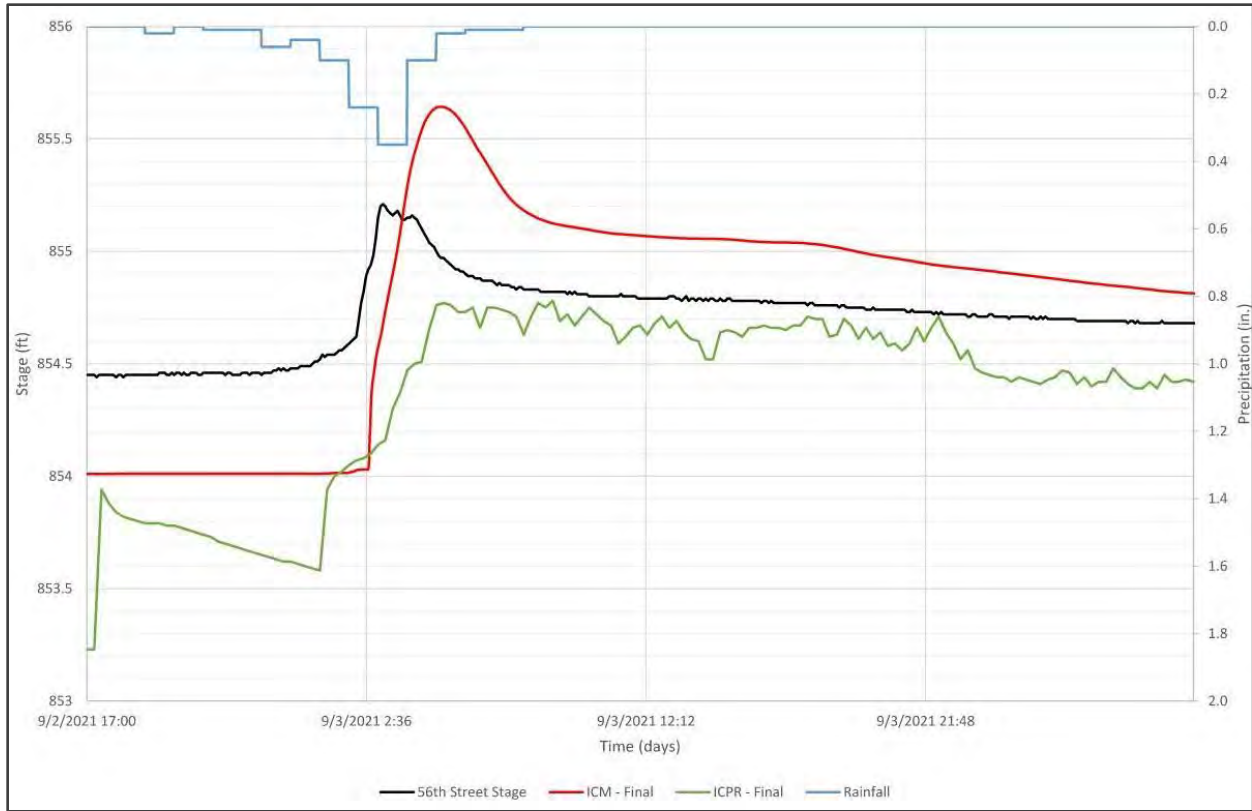


Figure 8. September 2021 Stage Comparison

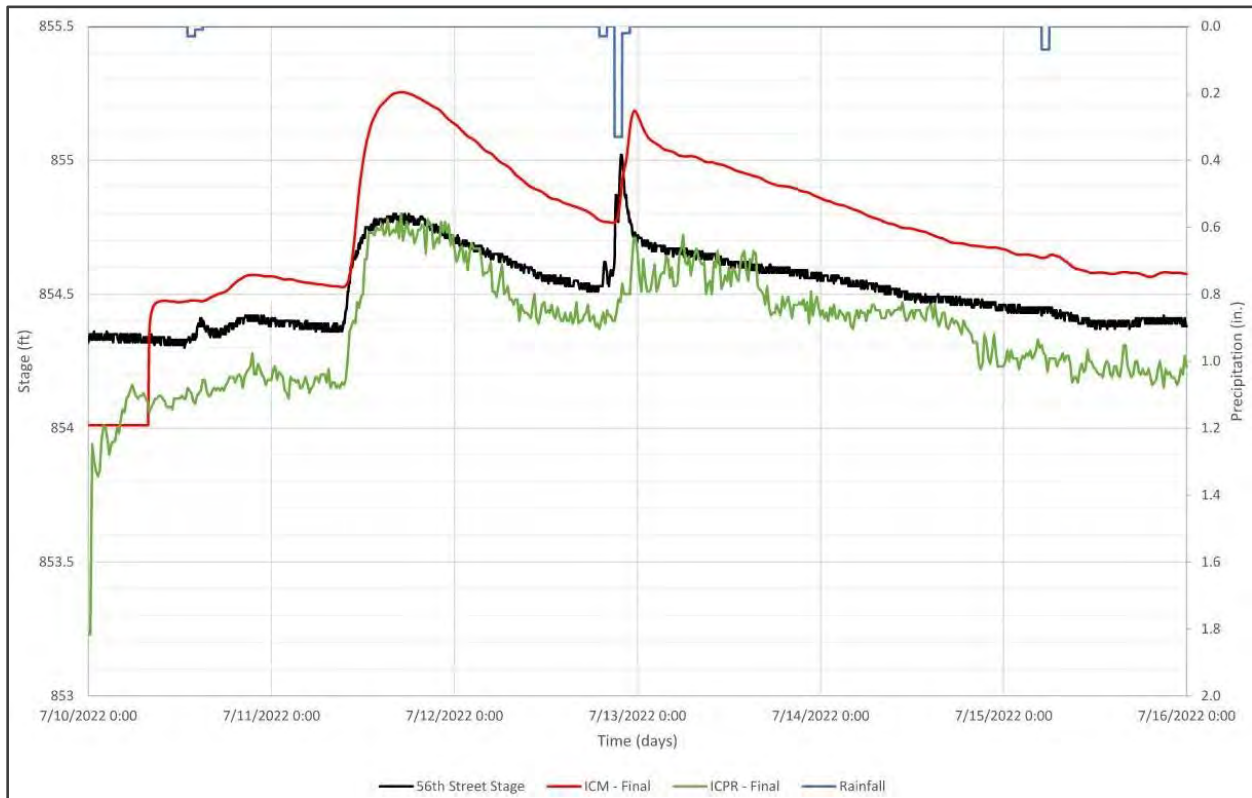


Figure 9. July 2022 Stage Comparison

6 CONSTRAINTS AND FUTURE RECOMMENDATIONS

Modeling constraints and in-depth understanding of the hydrologic and hydraulic system being modeled are uncovered during the calibration process. The effect of changing parameters and the magnitude of the changes is tracked to determine the best tools to perform the calibration. Through the pilot model’s calibration efforts, four key constraints were observed and should be considered as the District scales to the watershed-wide build. These include:

1. Accuracy and Resolution of Terrain Data

The largest constraint to developing a 1D-2D hydrologic/hydraulic model for extreme event analysis is the quality (or lack of) of the terrain information. Terrain files can be generated from survey data points, lidar files, contours, and a combination of all three. Each terrain data source has issues that are particular to the individual source.

Table 8. Strengths and Weaknesses of Elevation Data Sources

| Terrain Source | Strength | Weakness |
|----------------|--|--|
| LiDAR | Very high resolution | Unable to penetrate and represent below water storage |
| Survey Data | Ability to direct and fill elevation gaps where needed | Collected by surveyor discretion; manually intensive |
| Contours | Fill storage gaps that LiDAR can’t represent | Can miss critical hydraulic structures; often not readily digitized/accessible |

Understanding and using all three versions of terrain information will be key to developing large-scale accurate hydrologic and hydraulic models. The importance of this dataset was exemplified during the pilot model calibration when the inclusion of surveyed cross-sections and project as-built data greatly improved stage accuracy. During the watershed-wide model build process, it is critical that the best elevation information be used. This may require additional survey and elevation data to be obtained either through manual processes or partnership with individual agencies throughout the watershed to gather the required data. Channel and pond cross-sectional area will aid in the development of stage-area storage relationships for use with 1D objects or manipulations of the terrain surface to represent the storage within the 2D mesh.

2. Resolution of Monitoring Stations

Additional monitoring station data will be critical to the future calibration of the watershed-wide model build. Increasing the density and accuracy of the recorded data will allow for greater accuracy of the calibration process. Adjusting parameters to meet a single comparison point is valuable to understanding the sensitivity of the model in general and match results at the single location but overestimation of input parameters may occur. When the overestimation of a parameter occurs to match a single calibration point, a change in storm intensity or length can lead to large discrepancies in the results. It’s understood that the District is in the process of implementing its real-time sensor network that is designed to collect continuous water-level and flow from critical locations throughout the watershed. This data source will be extremely useful to aid in watershed-wide calibration.

3. Range of Calibration Events

The bulk of the available monitoring data was collected during the 2021 and 2022 open water seasons. Typically, two years of data provides a range of creek flows and responses to varying rainfall events (small, medium, large events). However, both 2021 and 2022 were drier than normal years for MCWD. In fact, most of 2021 the watershed was under moderate drought designation, with an extreme drought designation reached in the fall of 2022. The

precipitation events from 2021 and 2022 were minor events when compared to overall design storms in terms of rainfall depths and intensities. Calibrating a model to either extreme (drought or flood) can pose unique challenges.

During extreme events, debris may enter the flow paths and clog inlets to storm sewer and culverts, the spatial variability of rainfall is typically much higher, deeper water typically flows much faster than shallow water.

For the watershed-wide build, there ideally will be access to monitoring data that spans a wider range of water-level conditions. This is clearly outside anyone's control, but longer periods of record should help yield a variety of conditions to reference.

4. Vertically Varied Parameters

Within ICM, the Manning's n roughness coefficient can be varied up to three times depending on the depth within a cell. Within ICPR, the Manning's n roughness coefficient can be varied twice (shallow and deep). Both models allow for changes to the roughness values at each inundation level and changes to the inundation level breakpoints by roughness zone. The flexibility to adjust the parameter and level allows for a higher degree of calibration. Manning's n roughness values are reported as typical ranges that are applied to specific landuse categories. These ranges can be significant and should be reviewed for having representative values for the areas in the model. These are helpful when flow depth varies greatly during the simulation timeframe. As the depth of flow increases, the effects of friction on the flow velocity decreases. Developing the transition depths between low depth and high depth Manning's n values can further calibrate the model to various storm intensities and flood events.

ICPR allows for soils layers (and associated infiltration parameters) to be varied vertically. The infiltration parameters are the same as the Green-Ampt parameters with the addition of layer thickness and cells per layer. The use of vertical layers for infiltration allows for specification of known variability as soil depth increases. This can be beneficial when a clay layer is known. The drawback to this approach is the relatively high input data requirement. Typically, soil borings are needed to verify soil depths and associated infiltration parameters to accurately model infiltration using vertically varied parameters. This may be beneficial for smaller areas of interest within the watershed-wide model build but may become inefficient when scaling to the full watershed-wide model build.

While the calibration process allows for additional confidence in the modeled results to be gained, the process is never truly finished. The calibration process can be reevaluated at any point for either model developed through the 2D Pilot Model Project if additional monitoring data is obtained.

2D Pilot Model Build Scenario Modeling Report

Prepared by:
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Prepared for:



MINNEHAHA CREEK
WATERSHED DISTRICT
QUALITY OF WATER, QUALITY OF LIFE

Date:
May 2023

Kimley»»Horn

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Document History

| Date | Version | Description of Change |
|---------------|---------|-------------------------|
| December 2022 | 1.0 | Initial Draft |
| April 2023 | 2.0 | Revised Draft |
| May 2023 | 3.0 | Final Draft |
| May 2023 | 4.0 | Revised Final Submittal |

Definitions

- ArcGIS – a desktop and cloud solution provided by Esri for GIS analysis, storage, data management, and data processing
- DEM – Digital Elevation Model, represents ground surface in a grid (raster) format. Elevations are assigned to each individual grid cell
- GIS – Geographic Information System
- ICM – InfoWorks ICM software package developed by Innowyze to perform 1D and 2D hydrologic/hydraulic simulation modeling
- ICPR – Interconnected Channel and Pond Routing (ICPR) software package version 4 developed by Streamline Technologies to perform 1D and 2D hydrologic/hydraulic simulation modeling
- MetroGIS (MGIS) – a GIS format designed for use by Twin Cities Metropolitan-area municipalities for the standardization of infrastructure data
- MSE – Midwest and Southeast states, used in the NRCS Rainfall Distributions
- Geodatabase (GDB) – GIS file and data format that allows for standardization and template creation
- File Storage – cloud or physical file storage location utilized as a central repository for raw, input, and output datasets
- Shapefile – Spatial data file format that includes attribute data for individual shapes. May be in a point, line, polygon format and includes the file extension “.shp”.
- Scenario – Updates to a model that represent a past or future condition to simulate conditions

1 INTRODUCTION

The Minnehaha Creek Watershed District's (MCWD or District) current modeling tools are dated and do not provide the granularity and features necessary for the District to effectively manage and adapt to climate change. District staff identified the need to develop a new modeling tool that has greater granularity that can characterize and quantify the impacts of climate change and evaluate a range of scenarios to shape climate adaptation strategies, programmatic policies, and specific projects.

The District chose to pursue the pilot model build ahead of the watershed-wide build, to mitigate for the relational and technical risk that is often associated with large-scale, high-resolution models such as selecting the right software for the intended use. The pilot model compares two modeling platforms, ICM and ICPR, which were chosen based on findings from an initial screening of available two-dimensional models. A key step within the project is conducting various scenario and model runs to help evaluate and understand the strengths and weaknesses of each platform.

This memorandum provides an overview of the selected model runs, results, and learnings. Three categories of model runs were conducted, each aimed at learning something different about the two platforms. The objective of each category is described below:

- **Rainfall Scenarios:** These runs look to compare the results of ICM and ICPR to identify where we see differences and whether observations seen during calibration hold consistent in other areas of the watershed and under a wider range of rainfall conditions.
- **Geospatial Scenarios:** These scenarios look to reveal the differences and challenges associated with (1) incorporating adjusted spatial data and (2) model functionality and performance.
- **ICPR Groundwater Sensitivity:** These runs look to examine the level of influence ICPR's 2D groundwater component has on surface water results.

Model run times were tracked during a majority of the scenario runs as a performance metric and is included within this memorandum. In addition, the range of scenario runs provide a comparison of the output capabilities of each model. The specific model version used for this scenario analysis were:

- ICM version 2023.2.0 with an unlimited license; and
- ICPR version 4.07.08 with an expert license.

2 RAINFALL SCENARIOS

Prior to completing the scenario analysis model runs, both models were calibrated as described in the Model Calibration Memorandum. Calibration focused on the simulated stage results within Minnehaha Creek, as this portion of the subwatershed had the most robust recorded dataset. The calibration was also performed against small, low-intensity storm events; the recorded datasets captured numerous small, low-intensity storm events but no major rainfall events were captured within the subwatershed. The simulations included within this section aim to compare the modeling results of ICM and ICPR across a wider range of typical and intense rainfall conditions. It's important to characterize where differences were observed, explain potential reasons those differences may exist, and evaluate whether consistent patterns or biases are noticed across all the simulations. To provide additional context to the results comparison, additional outputs from other accepted models and datasets were included for a subset of the simulations.

Five simulations were run to support the objective of rainfall scenarios, which include:

1. FEMA BFE: Compares ICM and ICPR results to FEMA Base Flood Elevation (BFE) results;
2. Localized Flood Mapping: Compares localized ICM and ICPR flood inundation results to City of Edina's model output;
3. Turbid-Lundsten Discharge Rates: Compares ICM and ICPR discharge rates across four event-based simulations;

4. 2014 Flood of record: Compare ICM and ICPR under an extreme event; and
5. Design Storms: Compare ICM and ICPR results under four event-based design storms.

2.1 FEMA BFE – Edina Subwatershed

Data provided in **Table 1** represents a comparison between the documented FEMA Base Flood Elevation (BFE) results and the calibrated ICM and ICPR modeled high-water levels along Minnehaha Creek within the Edina subwatershed area. Model results data is taken from the calibrated base model-build versions of both models and the FEMA BFE results are based on the data from the published Flood Insurance Study (FIS) that was completed in 2016. The Minnehaha Creek XPSWMM model was originally certified by FEMA in 2003 and then a major update was completed in 2012 per FEMA documentation.

Results in the “Elevation” columns of **Table 1** represent each model’s results using the 100-year, 10-day event that was taken from the XPSWMM model for inflow conditions. The downstream boundary condition was set to mimic the stage elevation of the downstream BFE of 861. For ICM, the average difference in elevation at the BFE cross section lines is +1.0 feet with a standard deviation of 0.7 feet. For ICPR, the average difference in elevation at the BFE lines is +0.5 feet with a standard deviation of 0.9 feet. The average and standard deviation results were taken using only data from cross section represented as BFEs 872 through 862 in **Table 1**. Reported stage results at the upstream two and downstream cross-sections represents somewhat erroneous data for both models due to model boundary condition influences. It is generally not recommended to use simulation results that are close to boundary conditions due to potential for influences that overpower the actual simulated results. This can be seen in the results for both models near the model boundaries.

Table 1. Modeled Elevations Compared to FEMA Base Flood Elevations

| FEMA Cross-Section (27053C-) | FEMA BFE (ft) | ICPR | | ICM | |
|---------------------------------|------------------|----------------|------------|----------------|------------|
| | | Elevation (ft) | Delta (ft) | Elevation (ft) | Delta (ft) |
| 1547 | 875 ¹ | 878.1 | +3.1 | 878.0 | +3.0 |
| 1546 | 873 ¹ | 876.6 | +3.6 | 876.4 | +3.4 |
| 1544 | 872 | 874.8 | +2.8 | 874.0 | +2.0 |
| 1581 | 871 | 871.0 | 0.0 | 871.8 | +0.8 |
| 1545 | 869 | 869.1 | +0.1 | 869.3 | +0.3 |
| 1573 | 866 | 867.3 | +1.3 | 867.5 | +1.5 |
| 1542 | 865 | 865.9 | +0.9 | 866.9 | +1.9 |
| 1543 | 865 | 865.1 | +0.1 | 866.3 | +1.3 |
| 1541 | 864 | 864.4 | +0.4 | 865.0 | +1.0 |
| 1623 | 864 | 864.2 | +0.2 | 864.6 | +0.6 |
| 1574 | 863 | 862.5 | -0.5 | 863.0 | 0.0 |
| 1459 | 862 | 862.0 | +0.0 | 862.5 | +1.5 |
| 1539 | 861 ¹ | 865.0 | +4.0 | 861.3 | +0.3 |

1. Cross sectional results Impacted by boundary conditions

As shown in **Table 1**, the ICM model results are generally a bit higher than ICPR, except near the upstream and downstream boundary sections, while both models produce results slightly higher than the FEMA BFE results. This difference is likely a result of several factors, with the greatest influences being: a) not having calibrated the models to an extreme event; and b) differences in channel geometry between the FEMA model having a 1D cross section and the two pilot study models having variable mesh sizes. Additional refinement of input parameters may be beneficial during the watershed-wide build to more closely align the simulated results with the recorded data. Additional refinement may include finer delineation of changes in land cover in the overbank areas, modeling bridge crossings that impact the flow of water within the creek, and further

refinement of the 2D mesh to enforce all hydraulically significant features. Another approach that may be beneficial in the watershed wide build would be to create a 1D channel section throughout the critical reaches of the creek.

Overall, the results illustrate a reasonable validation of the model results, especially when considering that the model was not calibrated to a larger event. Both models show a slightly higher than FEMA result, which may relate to a combination of the additional runoff volume generated by the models that is not captured in the FEMA model that simulates a flow value through the creek channel, and the variation of the channel geometry between the 2D models and what was used in the FEMA model.

2.2 City of Edina Localized Flooding Maps

Localized flood maps based on results from the City of Edina’s previously calibrated XPSWMM model were used for comparison to the results produced by the calibrated ICM and ICPR models. In contrast to the BFE comparison in the previous section, this analysis looks to compare results within overland areas. The Edina XPSWMM model was calibrated based on data from a network of rain gauges and flow gauges that were installed throughout the Morningside Neighborhood. The Edina flood inundation boundaries for the 10-year and 100-year flood events were compared against the respective ICM and ICPR inundation results. Exhibits showing the modeled inundation (depths and boundaries), City of Edina flood extents, and FEMA Flood Zone data are included in Appendix A of this memorandum. High-water levels at four locations were taken from ICPR and ICM to compare results between the software packages and the City of Edina’s flood data. **Table 2** lists the high-water levels as well as the corresponding inundation depth in parenthesis.

Table 2. Peak High-Water Elevation Comparison

| Location | City of Edina Model | | ICPR | | ICM | |
|--|---------------------|----------|-------------|-------------|-------------|-------------|
| | 10-year | 100-year | 10-year | 100-year | 10-year | 100-year |
| Weber Pond | 868.2 | 869.3 | 864.8 (1.2) | 866.2 (2.6) | 865.3 (3.1) | 867.4 (5.2) |
| Townes Rd. – West (Pond/Wetland) | 872.8 | 878.2 | 871.2 (3.4) | 872.3 (4.5) | 870.8 (3.2) | 872.6 (5.0) |
| West 51 st Street (Low Area) | 876.6 | 878.5 | 877.9 (2.0) | 879.3 (3.4) | 880.5 (4.7) | 881.6 (5.8) |
| Arden / 50 th Street (Low Area) | 886.2 | 886.4 | 883.5 (1.9) | 884.9 (3.3) | 884.4 (2.9) | 885.8 (4.0) |

When comparing the ICPR and ICM models, the ICM model consistently produced higher peak stage values than ICPR. The only location where ICPR produced higher results was the Townes Road – West area. This was due to a strong influence from the groundwater module in the area of the Townes Road – West. The groundwater module starting elevation was derived from the groundwater monitoring wells, the wells averaged a groundwater elevation approximately 6 feet below the existing ground. Directly east and south of Townes Road – West is a high area that ranges in maximum elevation between 910 and 920 feet. As the simulation begins, this area of mounded groundwater begins to flow to the low area that is Townes Road – West (ground elevation of 867 feet). This difference in groundwater level versus ground elevation allows for large amounts of groundwater to enter the overland flow mesh. This area also does not have a natural or piped outlet within the models.

When comparing the modeled results for ICPR and ICM to the City of Edina data, including a review of Exhibits 1A through 2B in the Appendix, the pond/wetland areas (Weber Pond and Townes Road) have lower peak elevations and smaller flood inundation limits while the low areas (West 51st and Arden/50th) match closer to the City of Edina data.

The ICM inundation results show ponding in many of the smaller areas scattered throughout the subwatershed for the 10-year and 100-year rainfall events, whereas the ICPR inundation results over-represent the extent of deeper inundation and under-represent shallow inundation. This is largely due to the relatively large cell size that ICPR was developed with due to the placement of breakpoints since the terrain-sensitive meshing tool within ICPR cannot be used when also including a groundwater mesh. **Figure 1** illustrates the differences in the mesh resolution of the final calibrated model versions for ICPR and ICM. The ICM mesh was based on the terrain sensitive meshing tool, while the ICPR mesh was based on the defined break

lines and breakpoints. Areas of shallower inundation may be able to be better represented through additional breakline refinement and delineation of mesh refinement areas within the ICPR model. These updates would be done through user inputs and user created features. ICM shows extensive flooding in street areas where the City of Edina data shows no inundation (Intersection of Townes Road and W 48th Street and along Kellogg Avenue and Wooddale Avenue) within the 100-year results. The inundation extents within ICM underestimates some of the shallow ponding areas north and south of Branson Street. ICPR would require additional manual effort to achieve a similar resolution that was obtained in ICM through its automated build process. ICPR is reliant on breaklines and breakpoints for mesh creation when the groundwater module is used; Based on experiences during the pilot model build, this effort may not scale efficiently during the watershed-wide build process.

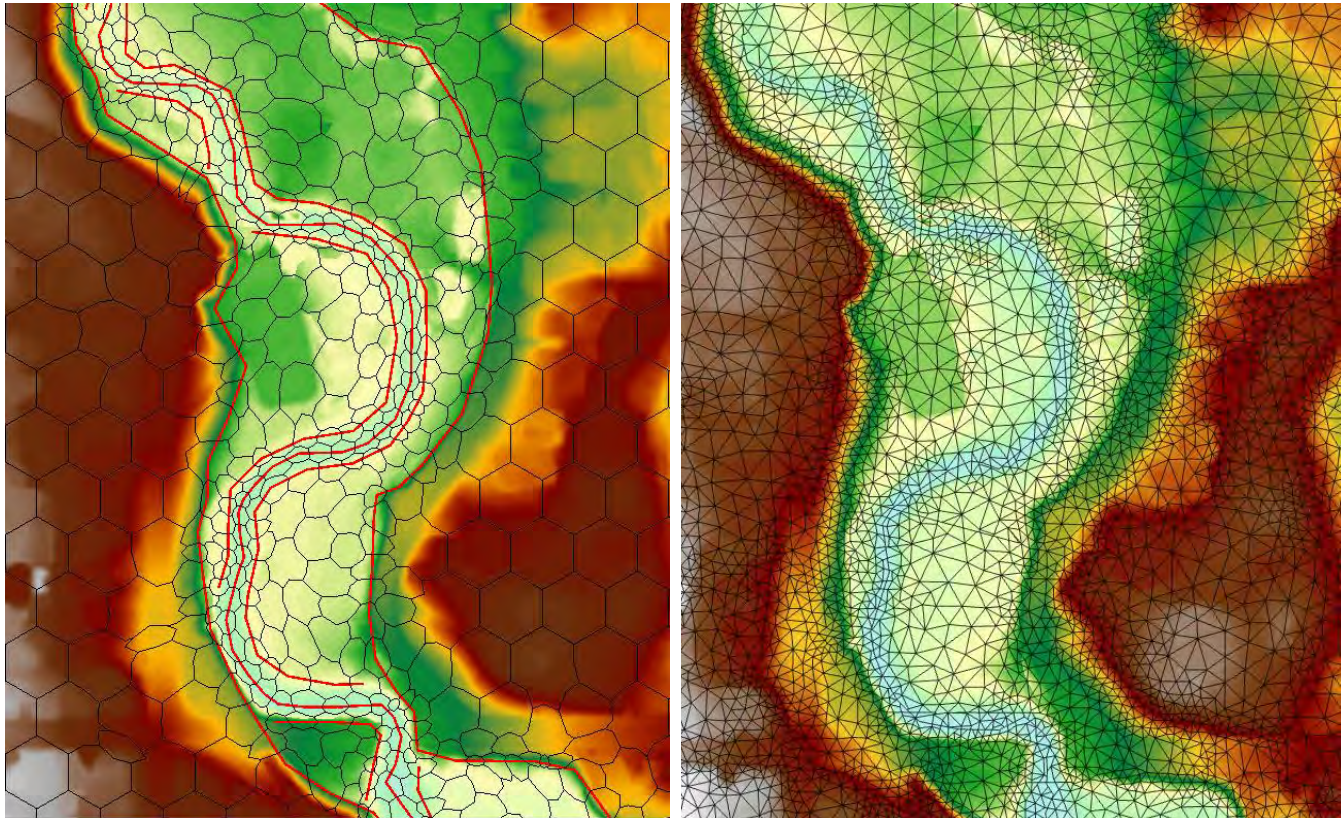


Figure 1. Mesh Development ICPR (left) vs. ICM (right)

The areas of major inundation within the model areas show higher inundation depths/elevations in the ICPR model versus the ICM model. As expected, the corresponding inundation extents are also higher within ICPR versus ICM. The inundation results appear to be more consistent through the Minnehaha Creek channel in ICM versus ICPR, likely a result of the increase in size of the mesh elements in ICPR. The larger mesh element size (i.e., lower resolution) relates to a poorer representation of the creek channel and loss in available cross-sectional area.

Changing the inundation depth cutoff from 0.25 feet to 0.10 feet increases the inundation boundary in ICM to more closely match the Edina localized flood mapping. The level of detail in the infrastructure dataset is assumed to be greater in the Edina (city) model than that of the pilot model builds considering that the intent was to build a model to a specific and very local flood inundation area and to build it manually by adding area-specific features (pipe, pumps, etc.) and making area-specific adjustments within the model to calibrate the model to a known response. Conversely, the 2D Pilot model builds were largely automated model builds using the baseline best available data and making manual additions and adjustments only where needed to reach the established calibration thresholds. An increase in level of refinement in both pilot models (e.g., breaklines along all roads) would allow for finer results and routing of overland flow. Increasing the detail within the

infrastructure dataset (e.g., assigning real values for assumed pipe inverts and pipe sizes) would increase the level of accuracy when simulating low to mid-level intensity storm events.

Mesh refinement will be an important effort to the accuracy of model results during the full model build process. While mesh elements can be decreased in size to limit the amount of manual mesh refinement that is required, the trade-off will be model run times. Smaller cells result in shorter time steps increased run times. The key will be to find the right balance between the higher-resolution auto-generated mesh and the impact the higher resolution has on run times.

2.3 Turbid-Lundsten Discharge Rate

The Turbid-Lundsten subwatershed model area was run with the 2-year, 10-year, 100-year 24-hour and the 100-year 10-day rainfall events. The four rainfall events simulated are typical events that are analyzed as part of developing a proposed site or development plan. While this model didn't follow the same level of calibration process that the Edina models followed, scenario's run within the Turbid-Lundsten area offer an opportunity to evaluate if ICM and ICPR behave in a similar way within an undeveloped area that has different underlying features.

Table 3 shows the discharge rate from the model area through the existing 48-inch culvert under Highway 5 for each event for both models. While the District has monitoring data at the downstream end of the culvert under Highway 5 for short periods of 2014, 2015 and 2016, there is not corresponding detailed rainfall event data that can confirm which event(s) best matches the short periods of monitored flows for the outfall. However, daily precipitation totals were pulled for 2014-2016 from the Minnesota State Climatology Office (MSCO) website, using the Nearest Station Precipitation Data retrieval tool to use as the best available data for comparison.

Table 3. Peak Discharge Rate Comparison

| Model | 2-year, 24-hour (2.8 inch) | 10-year, 24-hour (4.2 inch) | 100-year, 24-hour (7.5 inch) | 100-year, 10-day (7.5 inch) |
|-------|-------------------------------|--------------------------------|---------------------------------|--------------------------------|
| ICPR | 67.0 | 99.8 | 176.6 | 176.6 |
| ICM | 38.2 | 61.8 | 152.1 | 177.5 |

ICPR produced the same peak discharge rate for the 100-year, 24-hour and 100-year, 10-day events. This may relate to the interaction between 1D and 2D elements in ICPR that results in limiting flow when a pipe becomes surcharged. Based upon standard engineering practice pipe calculations, as a pipe becomes surcharged there is an increase in pressure and the flow continues to increase through the pipe as surcharge increases. When reviewing the flow hydrograph from ICPR for the 48-inch culvert, the flow increases to 176.6 cfs and then holds constant until the flow draws down below that level as shown in **Figure 2**. The hydrograph flatlines and does not allow a discharge rate above 176.6 cfs to occur in ICPR. We did not observe this flow cut-off within ICM for flows up to 190 cfs in the Turbid future development model runs.

When comparing discharge model results to the MSCO precipitation data, the first observation is that the overall subwatershed is very flashy when reviewing the model results and response of the system. The drawdown time in the 100-year, 24-hour run from the peak discharge of 152.1 cfs to approximately 2 cfs occurred within 24 hours for ICM and from 176.6 to 40 cfs in ICPR in 36 hours. The model run was not extended past 48 total hours for this validation run.

The first observation of monitored flows in early 2014 is that flows appear to have some level of baseflow on the order of 1-3 cfs. The frequency of recording flow values of once every one to two weeks was clearly not sufficient to capture the peak discharge for each of the rainfall events. However, the baseflow results are consistent with what the modeling shows for the tail of the hydrograph after 24-hours at a discharge on the order of 2 cfs. Overall, pilot model flows seem to be in the range of what could be expected for comparable depth events simulated in both models. More frequent monitoring points throughout a given event would be needed to better capture the flashy nature of this watershed.

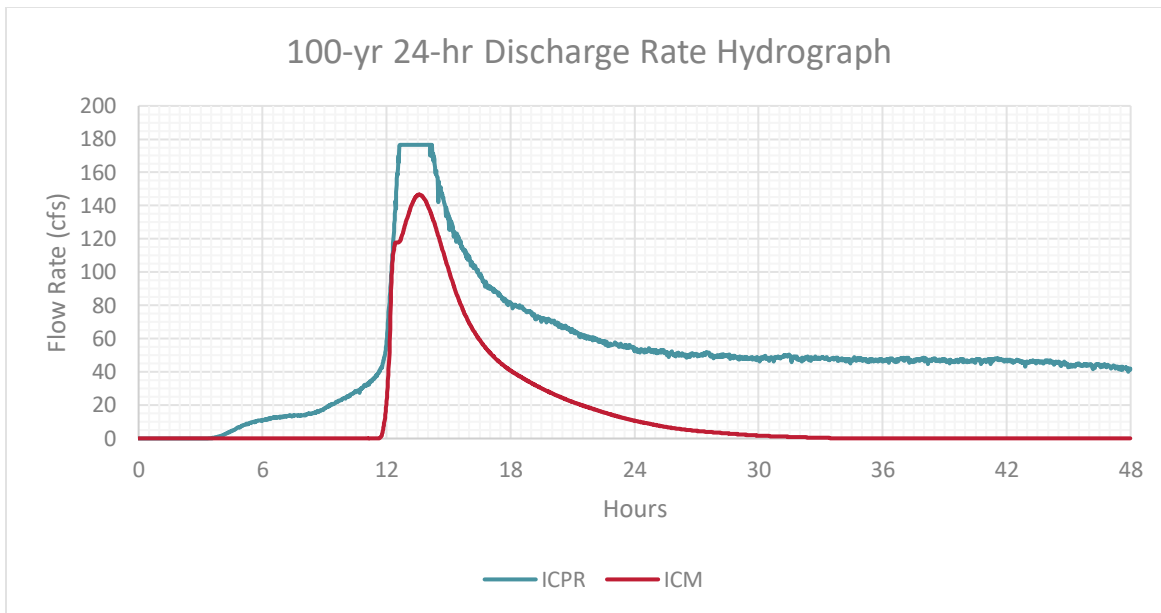


Figure 2. 100-year, 24-hour Discharge Rate – Turbid-Lundsten

2.4 2014 Flood of Record in Edina

The flood of record for the Edina subwatershed occurred over multiple days in the middle of June 2014. The timing of this storm points to the origin coming from a summer thunderstorm event. The event produced approximately 7.5 inches of rainfall over a 6-day (147 hour) period. Based upon the NOAA Atlas-14 point precipitation frequency estimates, this storm ranges between a 25- and 50-year rainfall event. Within the longer storm, three peak rainfall intensity periods occurred. The first peak reached an intensity of 0.57 inches per hour, the second peak reached 0.46 inches per hour, and the final peak reached 1.28 inches per hour. The first and third peak rainfall periods produced the greatest portion of the overall rainfall depth (1st = 2.24 inches, 3rd = 4.19 inches).

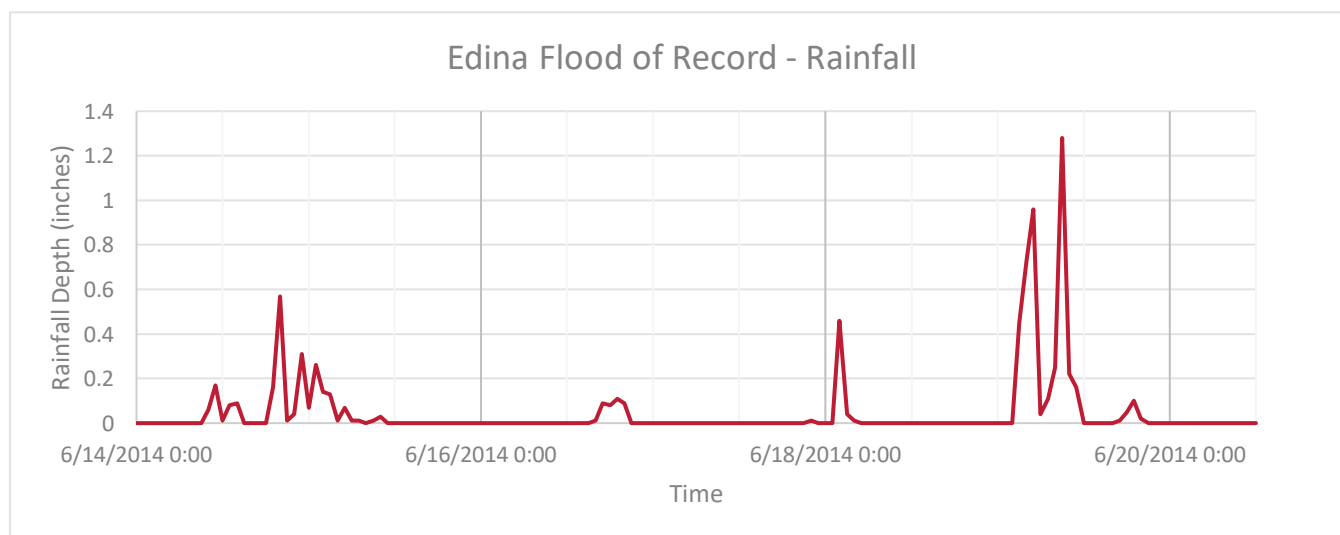


Figure 3. Rainfall Events and Depths During the 2014 Edina Flood of Record

Table 4 presents the peak high-water elevations and peak discharge rates at multiple locations along Minnehaha Creek for both pilot models. This scenario run did not include an upstream flow hydrograph boundary condition due to lack of flow data from 2014. Therefore, all flow and stage levels from the calibrated models within Minnehaha Creek are attributed to runoff from the rainfall event. ICM produced slightly higher peak elevations at all of the comparison locations but produced lower discharge rates compared to ICPR. This relates to the computational approach each model takes and highlights the need to look at multiple parameters when reviewing model results data. In addition, **Figures 4** and **5** illustrate the runoff hydrographs through the system at 54th Street for the ICPR and ICM pilot model runs, respectively.

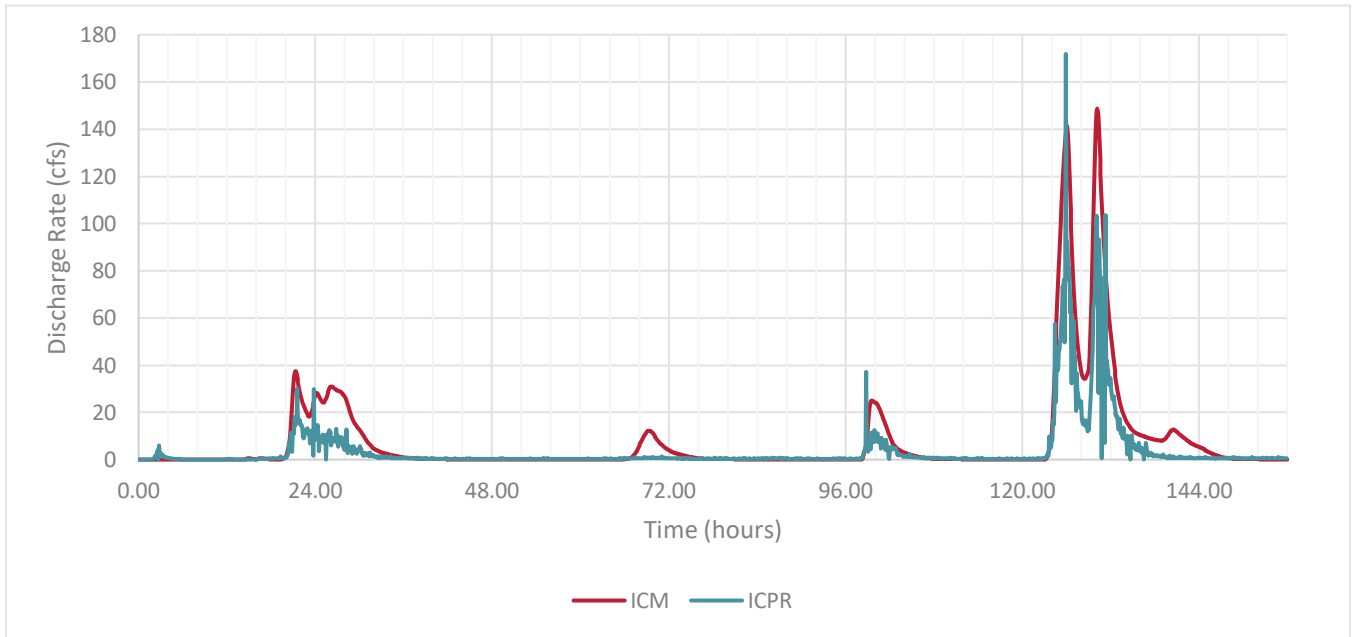


Figure 4. Discharge Rate Hydrograph at 54th Street

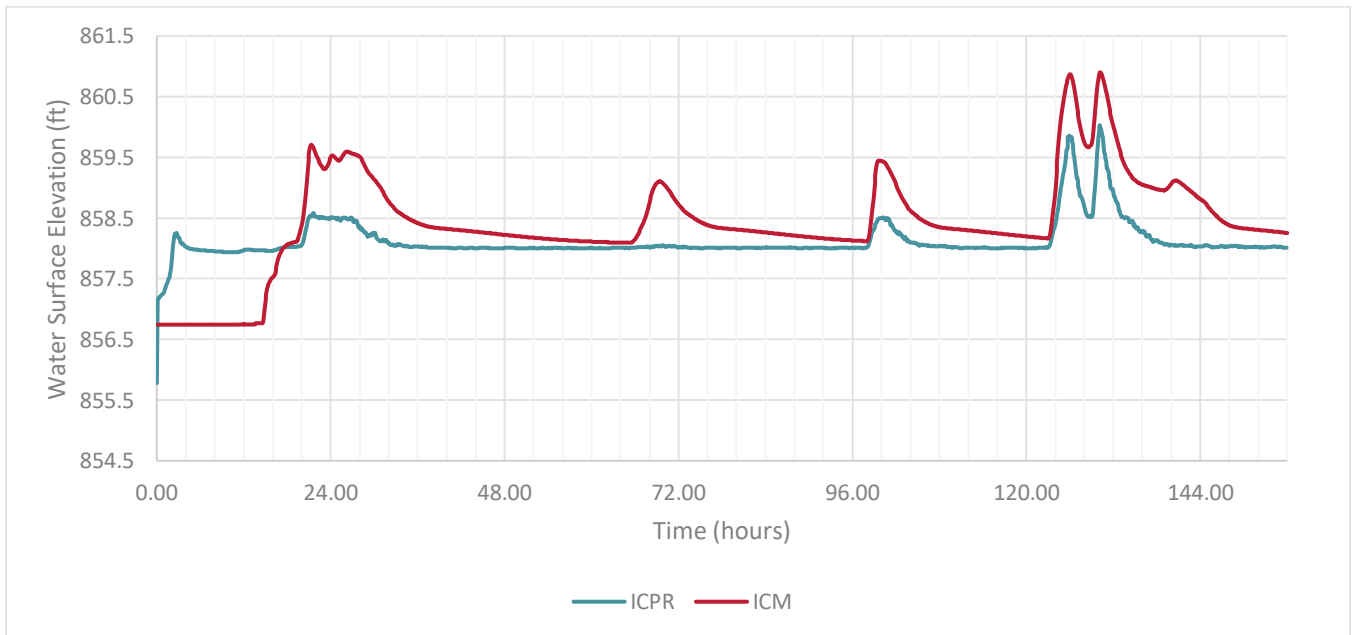


Figure 5. Water Surface Elevation Hydrograph at 54th Street

In the case of this model results comparison, the peak elevations are a much better point of comparison than the peak discharge rates. The peak discharge rates from ICPR were produced from single spikes in the discharge rates in an otherwise smooth plot, very similar to the noise shown in the calibration stage hydrographs for ICPR. This does not indicate that the model results for peak elevation are not reasonable, only that further exploration of the peak discharge spike shown in the result data may not represent actual conditions. As discussed in the calibration memorandum, additional refinement of the channel geometry in both models may substantially address the peak discharge difference between the two models.

Table 4. Flood of Record – Peak Elevation and Discharge Rate Comparison

| Location | ICPR | | ICM | |
|--------------|---------------------|----------------------|---------------------|----------------------|
| | Peak Elevation (ft) | Peak Discharge (cfs) | Peak Elevation (ft) | Peak Discharge (cfs) |
| Wooddale Ave | 870.80 | 42.7 | 871.58 | 42.7 |
| Arden Park | 861.99 | 169.7 | 863.15 | 119.7 |
| 54th Street | 860.06 | 291.3 | 860.89 | 148.7 |
| 56th Street | 856.24 | 336.9 | 857.18 | 150.5 |

2.5 Atlas-14 24-hour Design Storms – Edina

The second scenario dataset uses the Atlas-14 rainfall depths applied over the MSE-3 rainfall distribution to create design storms. The Atlas-14 rainfall depths representing the 2-year, 10-year, and 100-year return periods that are associated with a 24-hour storm event were taken from the NOAA Atlas-14 Point Precipitation Frequency Data Server. The rainfall depths used for this scenario analysis were: 2-year – 2.86 inches, 10-year – 4.28 inches, and 100-year – 7.49 inches. The rainfall depths were then applied to the MSE Type 3 (MSE-3) dimensionless rainfall distribution curve.

Table 5 details the peak high-water level, peak discharge rate, and continuity error for each model. The peak high-water level and flow rates are taken from both models at the 56th Street crossing of Minnehaha Creek. ICM again produced slightly higher peak elevations during each of the simulated storm events than the ICPR model, with all differences for the three events on the order of one foot. The peak discharge rates in **Table 5** are similar to the trend discussed for data in **Table 4** that all ICPR peak discharges are higher than ICM. The continuity errors for both ICPR and ICM are within acceptable ranges, while ICPR continuity errors are consistently higher than ICM. Again, a reflection of the noise seen in the ICPR hydrograph plots.

Table 5. Atlas 14 - Peak Elevation, Discharge Rate and Continuity Comparison

| Event (24-Hour) | Peak Elevation (ft) | | Peak Discharge (cfs) | | Continuity (%) | |
|-----------------|---------------------|-------|----------------------|-------|----------------|--------|
| | ICPR | ICM | ICPR | ICM | ICPR | ICM |
| 2-yr | 856.0 | 857.1 | 258.2 | 142.8 | -1.420 | 0.038 |
| 10-yr | 857.0 | 857.9 | 395.2 | 246.6 | -1.620 | 0.020 |
| 100-yr | 858.2 | 859.2 | 976.6 | 505.3 | 0.150 | -0.001 |

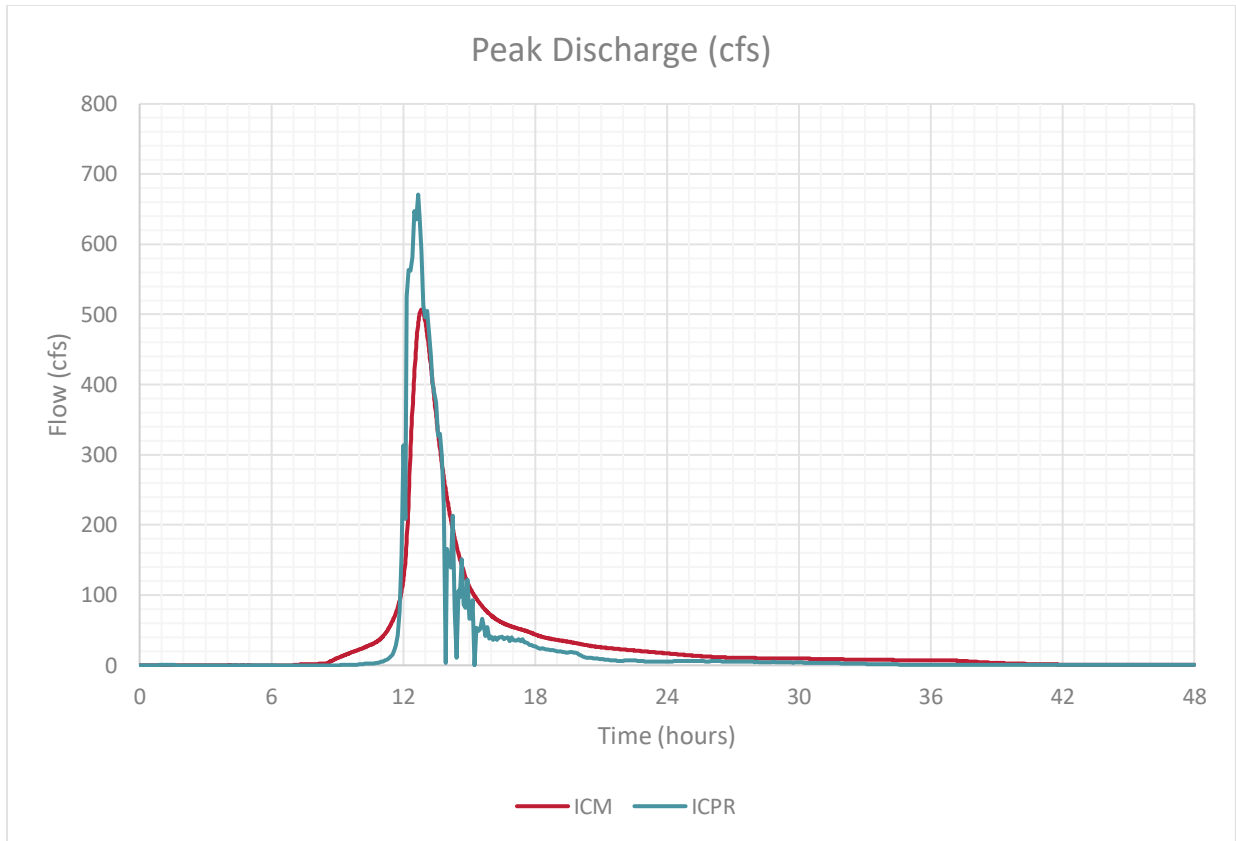


Figure 6. Peak Discharge Hydrograph – Exported Results

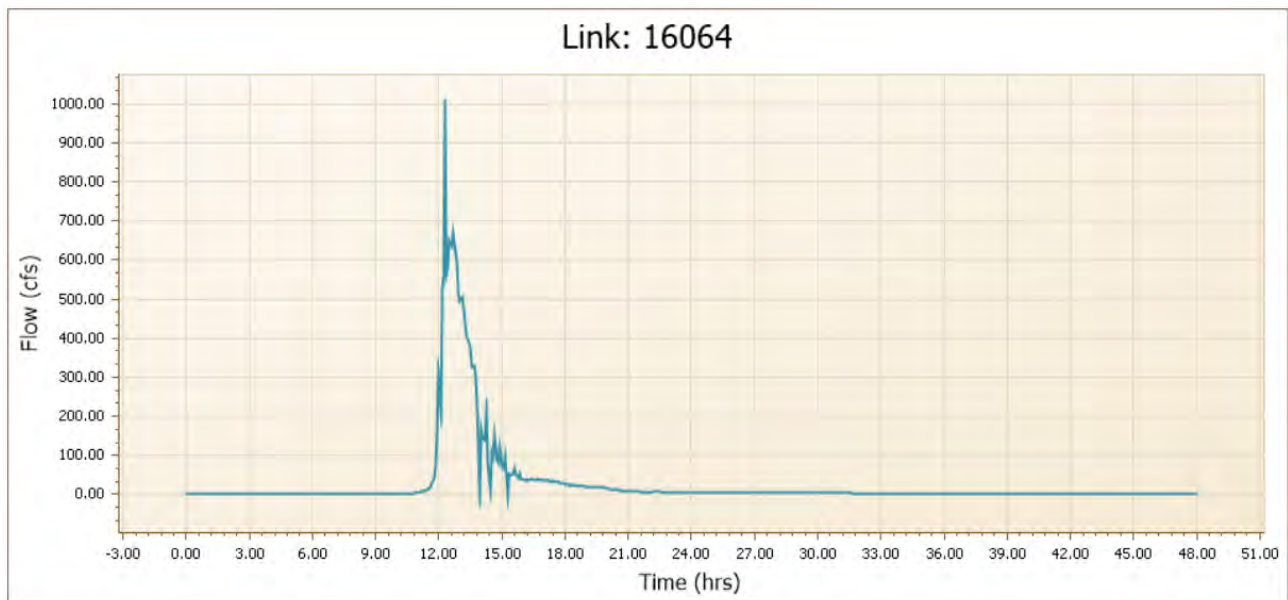


Figure 7. ICPR Peak Discharge Results – Internal Hydrograph

One important takeaway from the working through the data export process within ICPR, is that we observed different peak discharge rates at a given node when viewing data within the model compared to the data that was exported from the model

to be presented or plotted in Excel, for example. As shown in the internal hydrograph in **Figure 7**, the peak discharge rate is on the order of 1,000 cfs, which was not shown in the exported results. In an effort to better capture the peak discharge rate in the exported results, the ICPR model was rerun with the surface hydraulic timestep being reduced from 5 minutes to 15 seconds. The rerun of the ICPR model with the smaller timestep also did not capture the simulated peak discharge rate shown visually within the model, in the exported data. The rerun of ICPR did result in an increase in model run time from 24 minutes to 303 minutes, with no other model parameters adjusted between the two model runs. The variability in results seen within the visual software interface compared to the exported results could lead to inconsistencies in reported results depending on where the model user pulls the data from. At a minimum, we would recommend a standard process to use the values taken from a consistent approach, based on the model users best professional judgement of viewing the hydrograph and reviewing the exported data. There are likely other model creation factors that could narrow the gap between the visual result and the exported data, such as a 1D channel, which we have mentioned previously as an option to reduce some of the noise in the high-water level results.

2.6 Summary

The five rainfall scenario simulations confirm that using a model that was only calibrated to relatively small events is difficult to draw precise conclusions on the validation of the models to larger events due to the wide variations in the rainfall event intensity, duration and antecedent conditions, for example.

ICM generally produced higher peak elevations than ICPR, which was also observed during the calibration process. This trend is not a result of the model computational process; instead, the trend relates much more to where we stopped the model adjustments during the calibration process and the lack of having a larger event to calibrate to. We are confident that the gap in peak elevations could be narrowed by additional adjustments in one or both model in additional calibration efforts and do not see this difference as a limitation or concern in either model. On the other hand, the peak discharge rates tend to be consistently higher for ICPR, which very much appears to relate to the computational processes within the pilot models. As we have discussed previously, ICPR shows much more variation in the peak discharge results with relatively high values shifting to lower values in subsequent time steps while ICM produces a much smoother hydrograph. As discussed in the calibration memorandum, this tends to present itself in a higher continuity error values for ICPR, with both models producing errors within the established calibration tolerance limits.

3 GEOSPATIAL SCENARIOS

The following sections detail two complex scenarios that looked at altering the geospatial data within the original calibrated models. These scenarios are represented of the processes and challenges the District will encounter as it looks to assess future impacts of climate and development activity. This subset of scenarios looks to reveal the differences and challenges associated with: 1) incorporating adjusted spatial data; and 2) model functionality and performance. The two selected scenarios include:

1. Pre-settlement vs. Future Development: Assesses the impacts of regional land-use change.
2. Morningside Flood Reduction Project: Assesses a localized change to pipe infrastructure.

The following sub-sections outline the datasets that were used to setup the scenarios, the results from each model, and a summary of the learnings.

3.1 Pre-Settlement vs. Future Development – Turbid-Lundsten

This scenario dataset includes pre-settlement and future development landuse for the Turbid-Lundsten subwatershed. The pre-settlement landuse file was broken out by vegetation and wetland area. The wetland areas are associated with a storage volume based upon the overall size of the wetland delineation. The wetland area delineations were also used to modify the terrain file by lowering each area with a wetland boundary by 2.5 feet to represent the pre-settlement conditions across the subwatershed. The future development scenario included the updated landuse delineations assuming residential and commercial buildout within the subwatershed according to the City of Victoria – West Growth Area projections from May 2018. Future development terrain was modified to represent future water quality ponding throughout the subwatershed to meet the current development requirements. Breaklines were added to the future development scenario in ICM and ICPR to

enforce the storage areas within the 2D mesh. The 2-year, 10-year, and 100-year 24-hour rainfall events and the 100-year 10-day rainfall event were simulated for the pre-settlement and future development scenarios. These events were modeled using the MSE-3 rainfall distribution applied directly to the model area.

Table 6 provides the pre-settlement, existing conditions and future development peak discharge rates and volume passing through the 48-inch culvert at Highway 5 from the Turbid-Lundsten subwatershed. The ICPR peak discharge rate was limited by the pipe capacity similar to the results discussed in **Section 2.3** and **Table 3**. The peak discharge in future development conditions was restricted to 176.6 cfs. The discharge rates were lower in pre-development conditions when compared to the existing conditions model results (**Section 2.3**). The future development discharge rates are similar to the existing conditions rates in ICM and ICPR, respectively.

Results in **Table 6** point towards an expected increase in overall peak discharge rates due to development within the subwatershed from pre-settlement to existing conditions. In addition, future development and associated pond creation/routing mitigate for the increase in discharge volumes and peak rates throughout the subwatershed relative to the existing conditions. It should be noted that the Turbid-Lundsten model was not calibrated, and results show a wide separation between the two modeling platforms. The difference between models is more pronounced as runoff volumes are compared (**Table 6**). Without a calibrated model, and sufficient monitoring data, it's difficult to discern whether one model is over or under predicting, or a combination of both. This highlights the importance of calibration beyond the baseline model build. As you may recall, a large separation existed between ICPR and ICM results within the Edina base model build. But upon calibration, scenario outputs (peak discharge/elevations) were quite similar to one another. Therefore, watershed-wide model calibration to both events and baseflow conditions will be critical and monitoring data will be needed within each major subwatershed.

Table 6. Pre-Settlement to Future Development – Discharge Rate and Volume Comparison

| Model / Scenario | 2-year, 24-hour (2.8 inch) | 10-year, 24-hour (4.2 inch) | 100-year, 24-hour (7.5 inch) | 100-year, 10-day (10.3 inch) |
|---|---------------------------------------|--|---|---|
| DISCHARGE RATE AT HIGHWAY-5 OUTLET (cfs) | | | | |
| ICPR | | | | |
| Pre-Settlement | 19.8 | 32.7 | 73.7 | 90.7 |
| Existing | 67.0 | 99.8 | 176.6 | 176.6 |
| Future Development | 72.6 | 104.6 | 176.6 | 176.6 |
| ICM | | | | |
| Pre-Settlement | 20.3 | 49.9 | 111.6 | 142.6 |
| Existing | 38.2 | 61.8 | 152.1 | 177.5 |
| Future Development | 28.9 | 58.8 | 168.6 | 190.3 |
| DISCHARGE VOLUME AT HIGHWAY-5 OUTLET (acre-feet) | | | | |
| ICPR | | | | |
| Pre-Settlement | 37.6 | 54.7 | 88.4 | 335.2 |
| Existing | 118.9 | 154.0 | 202.9 | 871.5 |
| Future Development | 149.2 | 164.8 | 191.4 | 1,272.7 |
| ICM | | | | |
| Pre-Settlement | 3.6 | 15.0 | 51.7 | 66.0 |
| Existing | 5.6 | 20.2 | 60.5 | 70.2 |
| Future Development | 7.8 | 23.2 | 64.1 | 79.2 |

ICPR and ICM are similar in effort required to update landuse and swap out DEM files for various scenario runs. Depending on extent of pipe updates required for a scenario, the effort to update varies between software packages. Small updates require similar levels of effort through hand edits to the pipe/node data within the models. Larger updates will require use

of the import tools for each software. ICM allows for multiple options when importing including overwrite, prompt, merge, and ignore when duplicate features are encountered during import. ICPR requires that the import dataset is clipped to only include the new/updated features. This allows for efficient updates and removal of previously created features. Additionally, 1D pond objects could be used within both models to test pond sizing and routing without the need for additional terrain and mesh element refinements. Modifying a single ponding feature to a 1D object would be a very simple and quick process, while for multiple locations the process would be more time consuming to manually enter each 1D object.

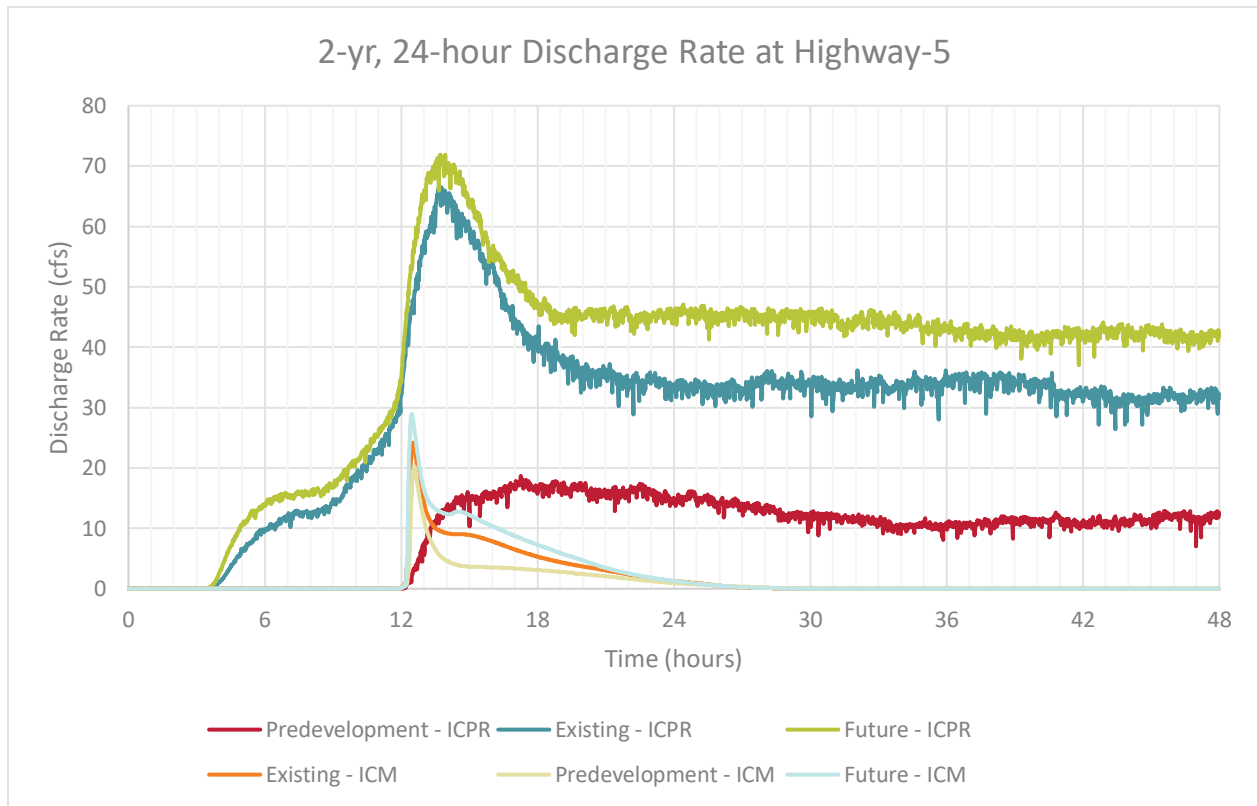


Figure 8. Discharge Rate Hydrographs – 2-year, 24-hour

3.2 Morningside Neighborhood Flood Reduction Project – Edina

The final scenario dataset includes the proposed improvements to the Morningside Neighborhood as part of the regional flood reduction project. The flood reduction project includes upsizing existing storm sewer, rerouting trunklines to maximize existing and proposed storage areas, and constructing a stormwater pump station and forcemain. Weber Pond will also be increased in size to further minimize the impacts from localized flooding in the neighborhood. The proposed conditions were simulated using the 2-year, 10-year, and 100-year 24-hour design storm events. The Morningside model updates included multiple trunk storm sewer updates (taken from a GIS file provided by the District), updated DEM and mesh refinement within both models, and the inclusion of the proposed pump from Weber Pond to allow drawdown and high-water level management. The pump curve was developed from the previous Basis of Design report supplied by the City of Edina for the Morningside Flood Risk Reduction Infrastructure Design (May 2022).

Inclusion of the pump data was unique in each model. ICM specifies pumps as pump links and ICPR specifies pumps as a rating curve link. Both options allow for varying levels of data to be included as part of the pump station. Typically, pump and forcemain hydraulics are calculated outside of the software to develop a pump curve that relates the upstream and downstream head to flow rates while including losses from bends, friction, and others. This allows for the pump system within the model to be simplified to a single link that takes inflow at the inlet end of the pump system and outflow at the outlet point. Both models allow for pump on/off elevations to be specified along with selecting a node within the model for

reference for each. ICM allows for logic and numerical arguments to be created and used to control the pump during a simulation independently of the pump curve (i.e., pump breaks 15 hours into simulation run).

Pond footprints were created within GIS and imported to the infiltration layer for ICM models. This was done to limit infiltration through the pond bottom and allow for specification of starting water elevations for pond features.

Table 7 presents model results for the high-water levels within Weber Pond as a result of the improvements to the Morningside neighborhood. ICPR produced higher peak results for the 2-year and 10-year rainfall events while ICM produced the highest peak result for the 100-year, 24-hour rainfall event. Both models produced comparable results for each of the three events with the difference between the two models being lower for the larger, 100-year, event run.

Table 7. Weber Pond – High-Water Level

| Storm Event | ICPR | ICM | City of Edina Model |
|-------------------|-------|-------|---------------------|
| 2-year, 24-hour | 860.4 | 859.7 | N/A |
| 10-year, 24-hour | 861.2 | 860.5 | 868.2 |
| 100-year, 24-hour | 863.1 | 863.4 | 869.3 |

Both models produced stage results significantly lower than the documented Basis of Design report that used the City’s previously constructed and calibrated XPSWMM model. These results raise the obvious question of why such a difference? There maybe a few possible factors that drive the differences including assumed/assigned pipe sizes used in the automated build process and mesh resolution. We believe the most significant factor is likely in the details of the infrastructure data for the automated model build. While calibration efforts focused on the recorded data at the creek, there was not corresponding calibration efforts focused on smaller localized ponding areas like Weber Pond for the pilot models. The automated model build includes an assumed 12-inch RCP pipe where no data was available in the initial infrastructure dataset in this area. Within the Weber Pond drainage area, the actual pipe sizes routed to Weber Pond may be larger than the assumed/assigned data used during the automated build process. Larger pipes would result in less restriction to the flow into Weber Pond and a higher peak.

Two main take-aways from this scenario simulation are that: 1) getting the most accurate base infrastructure data up front during the watershed-wide build will allow the automated model build to provide better results throughout the watershed instead of just at or near the calibration locations; and 2) depending on the desired use(s) of the model (i.e., creek evaluations versus localized areas such as Weber Pond), the level of calibration may need to vary.

4 ICPR GROUNDWATER SENSITIVITY

ICPR’s 2D groundwater capabilities are a defining feature and a key difference from ICM. ICPR can simulate groundwater flow through the use of a second 2D mesh layer that is aligned to the 2D surface mesh allowing groundwater to enter the surface layer and contribute to surface flows. ICM on the other hand, has a standard infiltration function that removes the water that infiltrates from the system and the infiltrated volume does not contribute to runoff.

During the calibration process for the ICPR pilot model, model runs were conducted to show the influence the 2D groundwater component had on results, by running a scenario with and without the groundwater module activated. Through that exercise, it became clear that ICPR’s groundwater component was responsible for keeping base flow within the channel in periods between storms. To further understand the influence of the groundwater module and its impact on results, three additional runs were conducted. The only change within each run was the starting groundwater level condition, which was:

1. Low: Constant elevation of 853 feet for the entire model area
2. High: Matching the terrain (e.g., water table is at the ground surface level)
3. Varied: 6-feet below the terrain. (the level used for all model build, calibration, validation and scenario analyses)

Results showed that the initial groundwater elevation assumption can have a significant impact for smaller storm event results when assessing high-water level results on ponding and low areas as shown in **Figure 9**. Groundwater level assumptions had a smaller impact on larger events results and on creek peak flow and stage results.

The resulting stage hydrographs are provided in **Appendix B**. Results are reported at Weber Pond and at the 56th Street crossing along Minnehaha Creek. The Morningside scenario model version was used to perform the sensitivity analysis. The influence in surface stage hydrograph levels varied depending on location (pond vs. creek) and storm intensity (2-yr vs. 10-yr vs. 100-yr). In relation to peak stage within Weber Pond, the High groundwater scenario produced extremely high stage results when compared to the Low and Varied groundwater scenarios for all of the simulated rainfall events. In part, this likely result from an additional volume of water being in the model at model start time. For an additional 6-feet of groundwater in a medium having 30% pore space, an additional 15 inches of water depth is available in the model, beyond what is produced in the rainfall-runoff process, and at least a portion of that available volume discharges into the surface features.

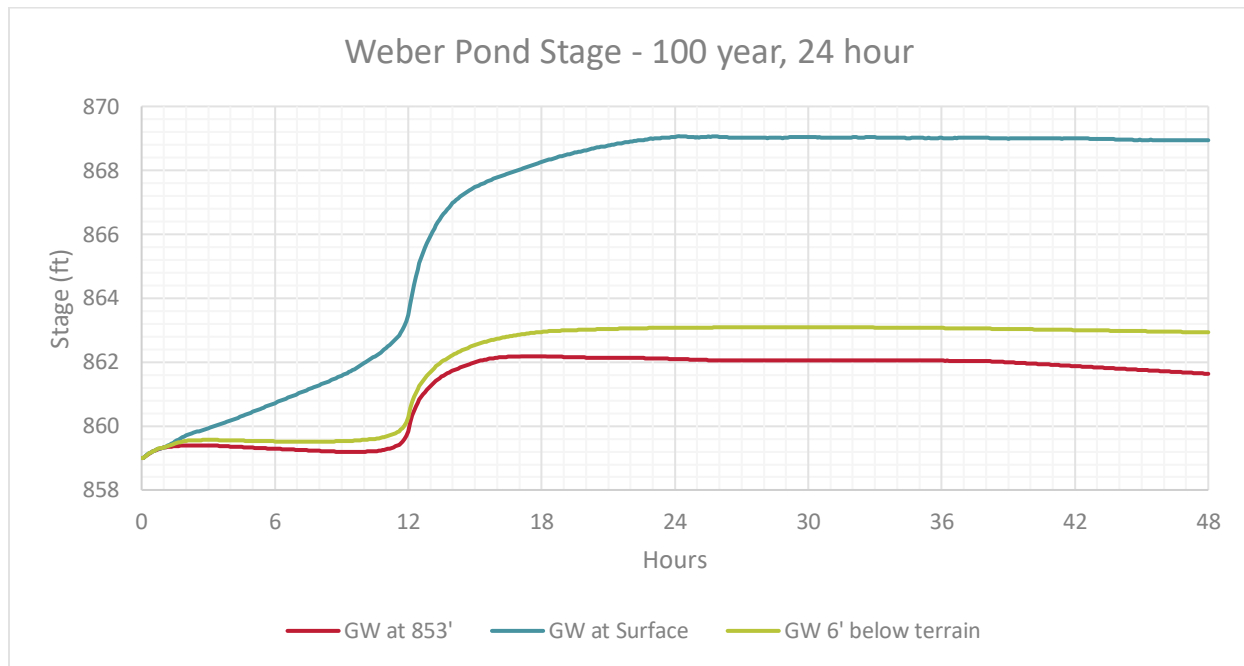


Figure 9. ICPR: Impact of Groundwater Initial Conditions on Weber Pond Stage

The 56th Street Stage results demonstrated a smaller influence from the High groundwater scenario across all storm events when compared to the Weber Pond stage results. The influence from the High groundwater scenario on the peak stage elevation also decreased as the intensity of the rainfall event increased.

At a minimum, the modeled differences in high-water level for both Weber Pond and 56th Street highlight the need for greater emphasis on having confidence in the groundwater elevations throughout the watershed if ICPR is the selected model for the watershed-wide build. Additional monitoring data for groundwater would clearly help to more accurately develop groundwater inputs for the watershed-wide model build process and subsequent simulation runs. Over long simulation time periods (e.g., the 77-day simulation in Calibration memorandum), the influence of groundwater is allowed to even out as the model “warms-up” at the beginning of the simulation time. This allows the ICPR model to more accurately simulate groundwater over long time periods and the influence of the starting groundwater elevation becomes increasingly minimal as the simulation time length increases. When reviewing and simulating shorter storm events, there is a larger impact of the results from the initial conditions.

5 MODEL RUN TIMES

The models were developed and run using various laptop setups to assess the overall usability and processing power needs and considerations. A computer with a good graphical processing unit (GPU) will be beneficial to reduce simulation run times for ICM. A computer with a fast CPU is beneficial for performing ICPR simulations and reduce overall run times. Both software

developers have recommendations for desired computing power and overall computer setups to increase modeling efficiency.

The run times shown in **Table 8** provide a comparison of the calibrated models across selected events. This is not a true apples-to-apples comparison, as the models were constructed at much different resolutions and required different adjustment to reach acceptable calibration tolerances. For example, ICPR has 11,900 triangular elements in the Turbid model while ICM has 96,000 triangular elements for the same model area. Similar differences in resolution are present in the Edina models. Furthermore, the ICPR model includes the groundwater simulation which is essentially a second 2D grid with a second set of computations being completed for each time step. This added computational need is offset within this set of examples by differences in the cell numbers and corresponding mesh size (i.e., resolution).

Table 8. Summary of Scenario Model Run Times

| Scenario | Storm Event | Simulation Length (hours) | Model Run Time (minutes) | |
|---------------------------|----------------|---------------------------|--------------------------|------|
| | | | ICM | ICPR |
| Edina Flood of Record | 2014 | 156 | 54 | 65 |
| Edina Atlas-14 | 2-yr, 24-hr | 48 | 18 | 15 |
| | 10-yr, 24-hr | 48 | 20 | 18 |
| | 100-yr, 24-hr | 48 | 21 (14) | 24 |
| Turbid Pre-Settlement | 100-yr, 24-hr | 48 | 42 | 85 |
| | 100-yr, 10-day | 360 | 223 | 290 |
| Turbid Future Development | 100-yr, 24-hr | 48 | 46 | 74 |
| | 100-yr, 10-day | 360 | 180 | 213 |
| Edina Morningside | 100-yr, 24-hr | 48 | 25 | 22 |

To evaluate run times on a more representative apples-to-apples scenario, model resolution was adjusted to be comparable between the two models. Due to model build challenges and level of effort observed while constructing mesh in ICPR, it was deemed most efficient to bring the ICM model to a lower resolution (i.e., larger mesh size). The purpose of these runs was solely to evaluate run times, and the impact the resolution change had on results was not considered. A computer with NVIDIA Quadro T2000 with Max-Q Design GPU and an Intel Core i7-10850H CPU was utilized for the comparison. Results for a 100-year, 24-hour design storm event run are presented in **Table 9**.

Table 9. Model Run Times with Comparable Model Resolution

| Subwatershed | Resolution | ICM | | ICPR | | |
|-----------------|------------|--------------------|------------------|--------------------|------------------|----------------------|
| | | Run Time (minutes) | # of 2D Elements | Run Time (minutes) | | # of 2D Elements |
| | | | | Overland Only | With Groundwater | |
| Turbid-Lundsten | Low | 20 | 12,053 | 33 | 47 | 11,900 |
| | High | 42 | 92,931 | 78 | 106 | 50,842 ¹ |
| | | | | 169 | N/A | 105,498 ² |

¹ ICPR high-resolution run developed from hand-delineation tools (breakpoint offset, breaklines)

² ICPR high-resolution run developed from automated build tool

The results indicate that longer run times will be experienced with ICPR. These run times will increase if the model is scaled watershed-wide and/or a greater resolution is desired. Modifications to the build of ICPR, such as “phased” groundwater regions, will be critical for watershed-wide scaling and may help workaround long run times. To further evaluate the model run time comparison, a sensitivity analysis was performed on the existing conditions Turbid-Lundsten watershed using the 100-year, 24-hour storm event and a 48-hour simulation length. This sensitivity analysis was completed at the end of the

scenario analysis and following the previous calibration analysis. Thus, all improvements to both models (ICM and ICPR) were incorporated within the analysis as well. Seven scenarios were included within the sensitivity analysis as shown in the list below and detailed in **Table 9**.

ICM

1. Low-Resolution (12,053 2D elements), Automated Mesh Tool
2. High-Resolution (92,931 2D elements), Automated Mesh Tool

ICPR

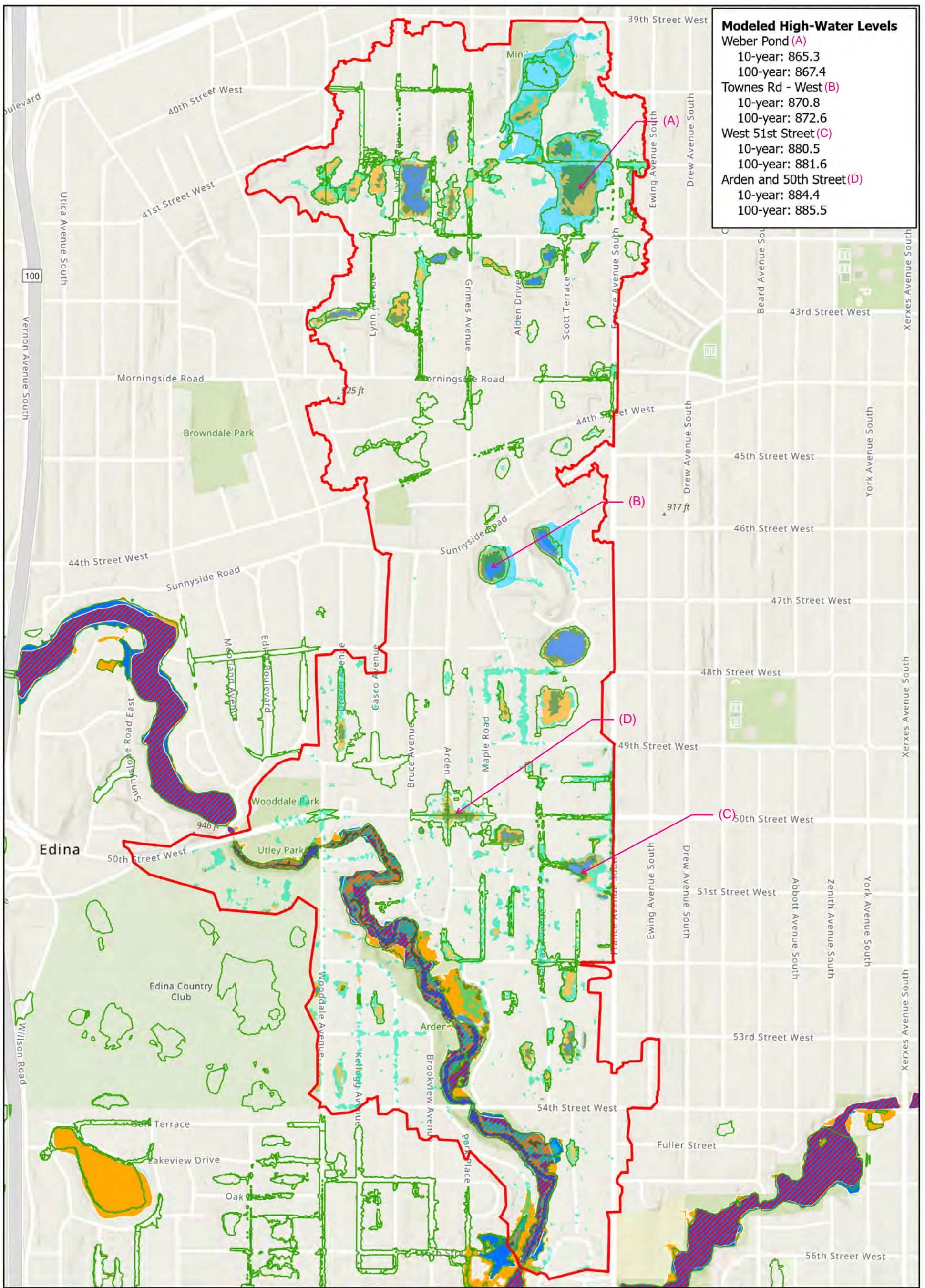
3. Low-Resolution (11,900 2D elements), Hand-Build Mesh, Overland Region Only
4. Low-Resolution (11,900 2D elements), Hand-Build Mesh, Overland and Groundwater Regions
5. High-Resolution (50,842 2D elements), Hand-Build Mesh, Overland Region Only
6. High-Resolution (50,842 2D elements), Hand-Build Mesh, Overland and Groundwater Regions
7. High-Resolution (105,498 2D elements), Automated Mesh Tool, Overland Region Only

ICM still reports faster run times across all scenarios. The groundwater region within ICPR increased run times additionally, although it should be noted that the previously discussed 18,000-20,000 groundwater mesh element limit was not reached in Scenario 6. If ICPR is scaled to the watershed wide build, the District could consider whether the groundwater module is needed for all scenario runs or whether its needed active through the entire watershed to help lower run-times.

Outside of the simulation processing time, ICM and ICPR both need to preprocess the mesh to parameterize each 2D mesh element with infiltration and roughness values for use during the model runs. ICM completes the preprocessing of the mesh quickly and will typically complete the preprocess build in under a minute to five minutes. ICPR completes the preprocessing in under 30 minutes for the low-resolution scenario and between two and five hours for the high-resolution scenario.

APPENDIX A

EDINA FLOOD MAPS



Modeled High-Water Levels

| | |
|---------------------------|-----------------|
| Weber Pond (A) | 10-year: 865.3 |
| | 100-year: 867.4 |
| Townes Rd - West (B) | 10-year: 870.8 |
| | 100-year: 872.6 |
| West 51st Street (C) | 10-year: 880.5 |
| | 100-year: 881.6 |
| Arden and 50th Street (D) | 10-year: 884.4 |
| | 100-year: 885.5 |

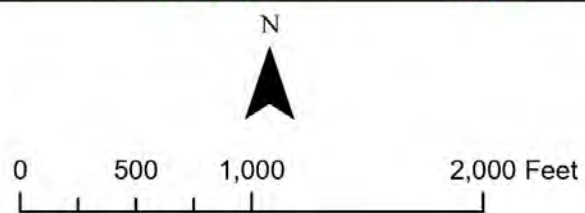
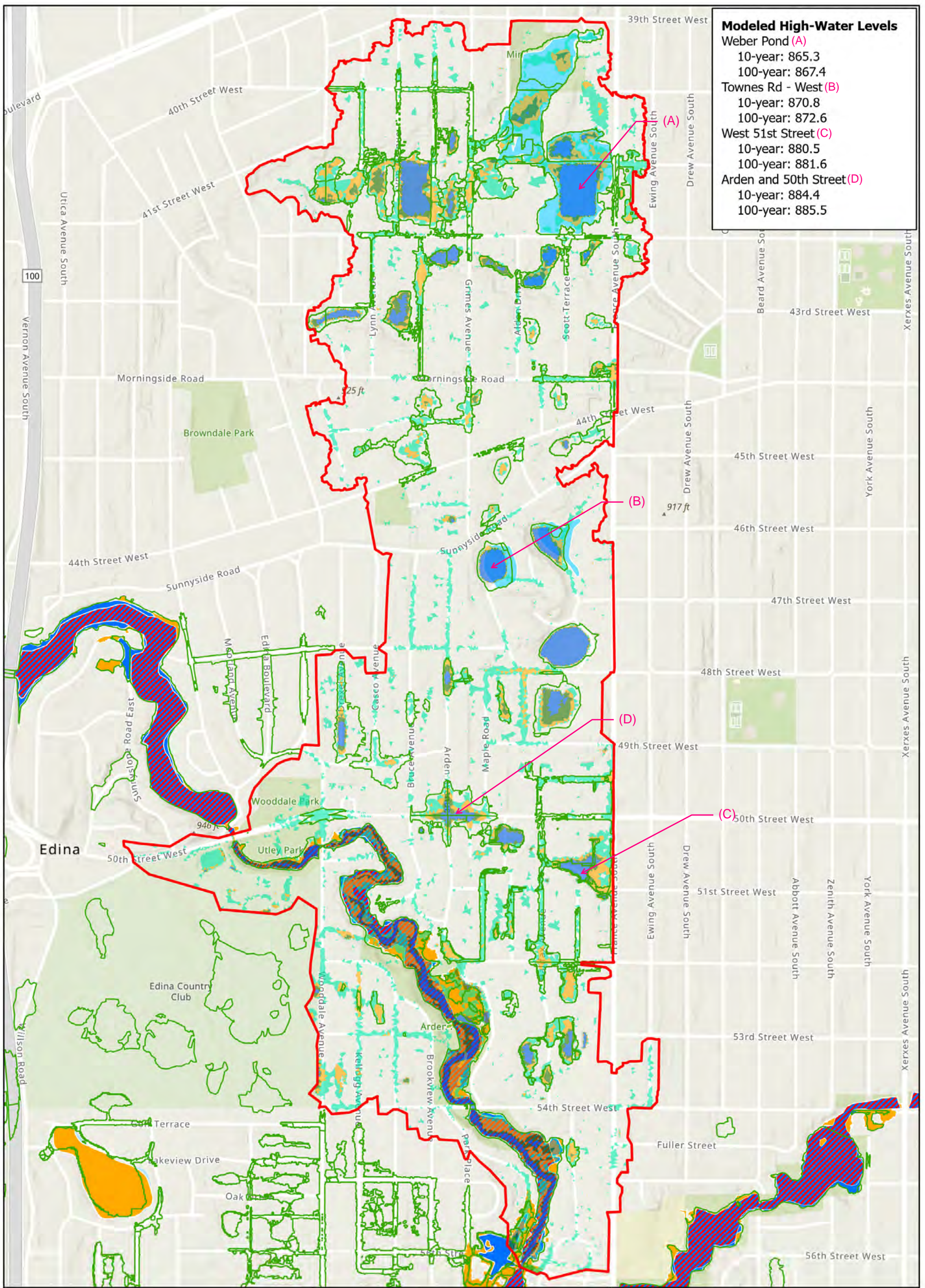


Exhibit 1A
ICM - Flood Inundation Comparison
 10-year, 24-hour Rainfall Event

Legend

| | |
|--|--|
| FEMA Flood Zone | City of Edina Mapping Inundation 10-year |
| A, AE, AE,ADMINISTRATIVE FLOODWAY, AE,FLOODWAY, AH, AO, X,0.2 PCT ANNUAL CHANCE FLOOD HAZARD | Edina Pilot Model Area |
| | 10-year Inundation (FT) |
| | Less than 0.25 |
| | 0.251 - 1.00 |
| | 1.01 - 2.00 |
| | 2.01 - 3.00 |
| | 3.01 - 10.00 |



Modeled High-Water Levels

| | |
|---------------------------|-----------------|
| Weber Pond (A) | 10-year: 865.3 |
| | 100-year: 867.4 |
| Townes Rd - West (B) | 10-year: 870.8 |
| | 100-year: 872.6 |
| West 51st Street (C) | 10-year: 880.5 |
| | 100-year: 881.6 |
| Arden and 50th Street (D) | 10-year: 884.4 |
| | 100-year: 885.5 |

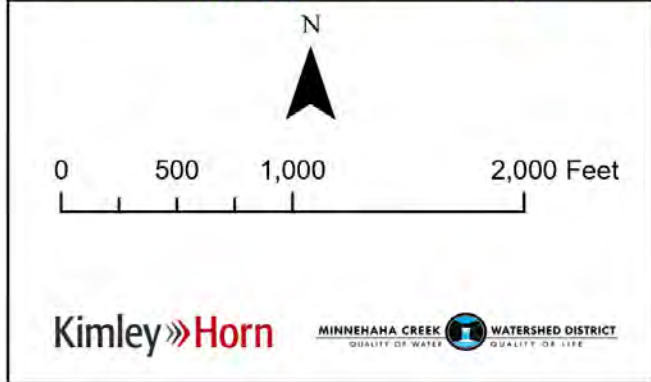


Exhibit 1B

ICM - Flood Inundation Comparison

100-year, 24-hour Rainfall Event

04/26/2023

Legend

| | |
|--------------------------------------|--------------------------|
| FEMA Flood Zone | Edina Pilot Model Area |
| A, | City of Edina Mapping |
| AE, | Inundation 100-year |
| AE,ADMINISTRATIVE FLOODWAY | 100-year Inundation (FT) |
| AE,FLOODWAY | <0.25 |
| AH, | 0.251 - 1.000 |
| AO, | 1.001 - 2.000 |
| X,0.2 PCT ANNUAL CHANCE FLOOD HAZARD | 2.001 - 3.000 |
| | 3.001 - 10.000 |

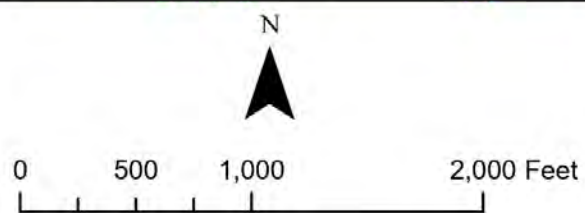
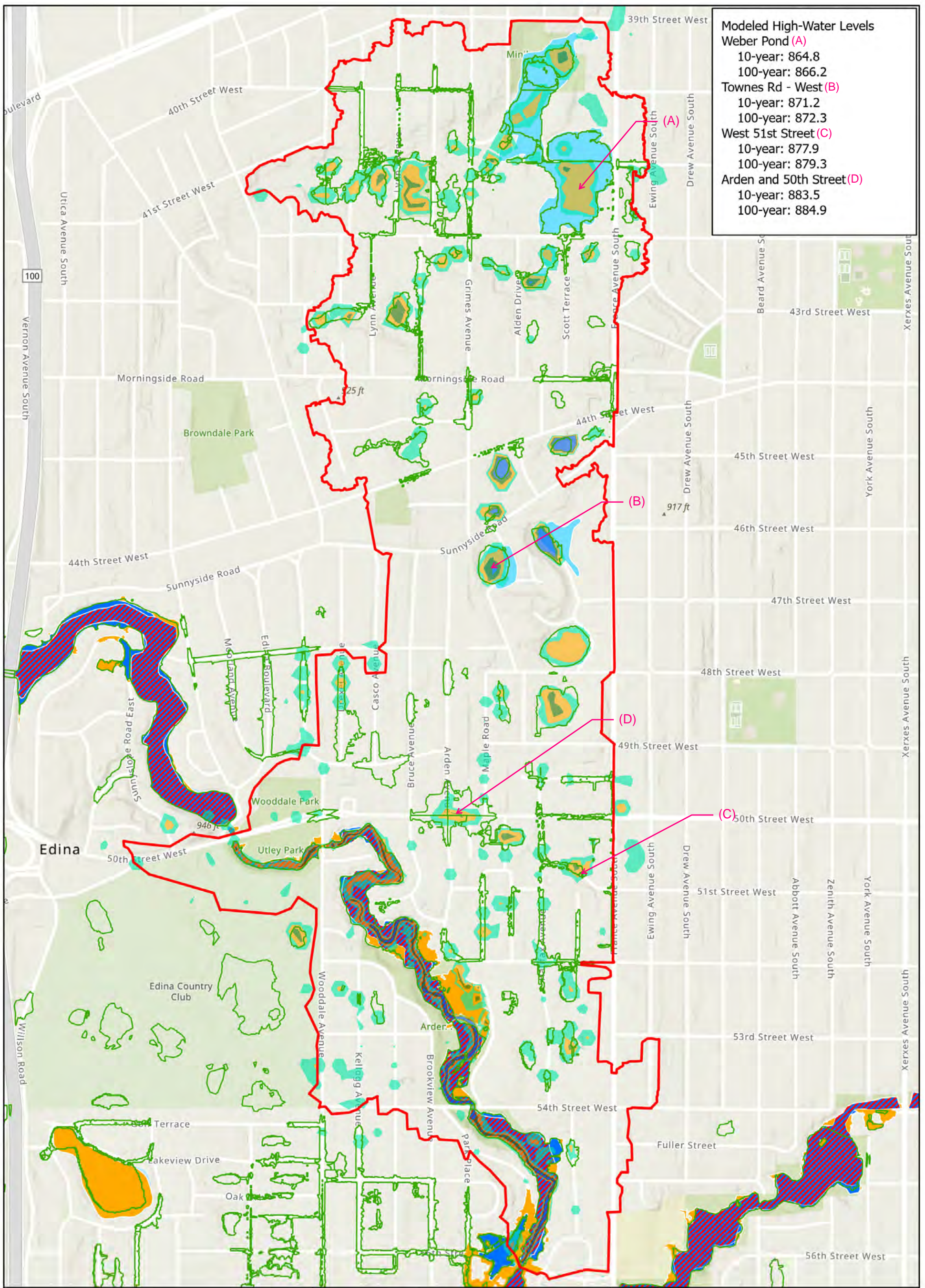


Exhibit 2A

ICPR - Flood Inundation Comparison

10-year, 24-hour Rainfall Event
04/26/2023

| FEMA Flood Zone | | Edina Pilot Model Area | |
|--------------------------------------|-------------|--|--|
| A, | AE, | | City of Edina Mapping Inundation 10-year |
| AE,ADMINISTRATIVE FLOODWAY | AE,FLOODWAY | | 10-year Inundation (FT) |
| AH, | AO, | | <0.25 |
| X,0.2 PCT ANNUAL CHANCE FLOOD HAZARD | | | 0.251 - 1.000 |
| | | | 1.001 - 2.000 |
| | | | 2.001 - 3.000 |
| | | | 3.001 - 13.000 |

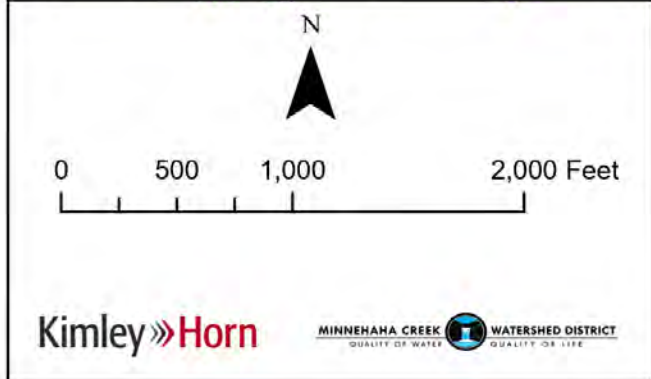
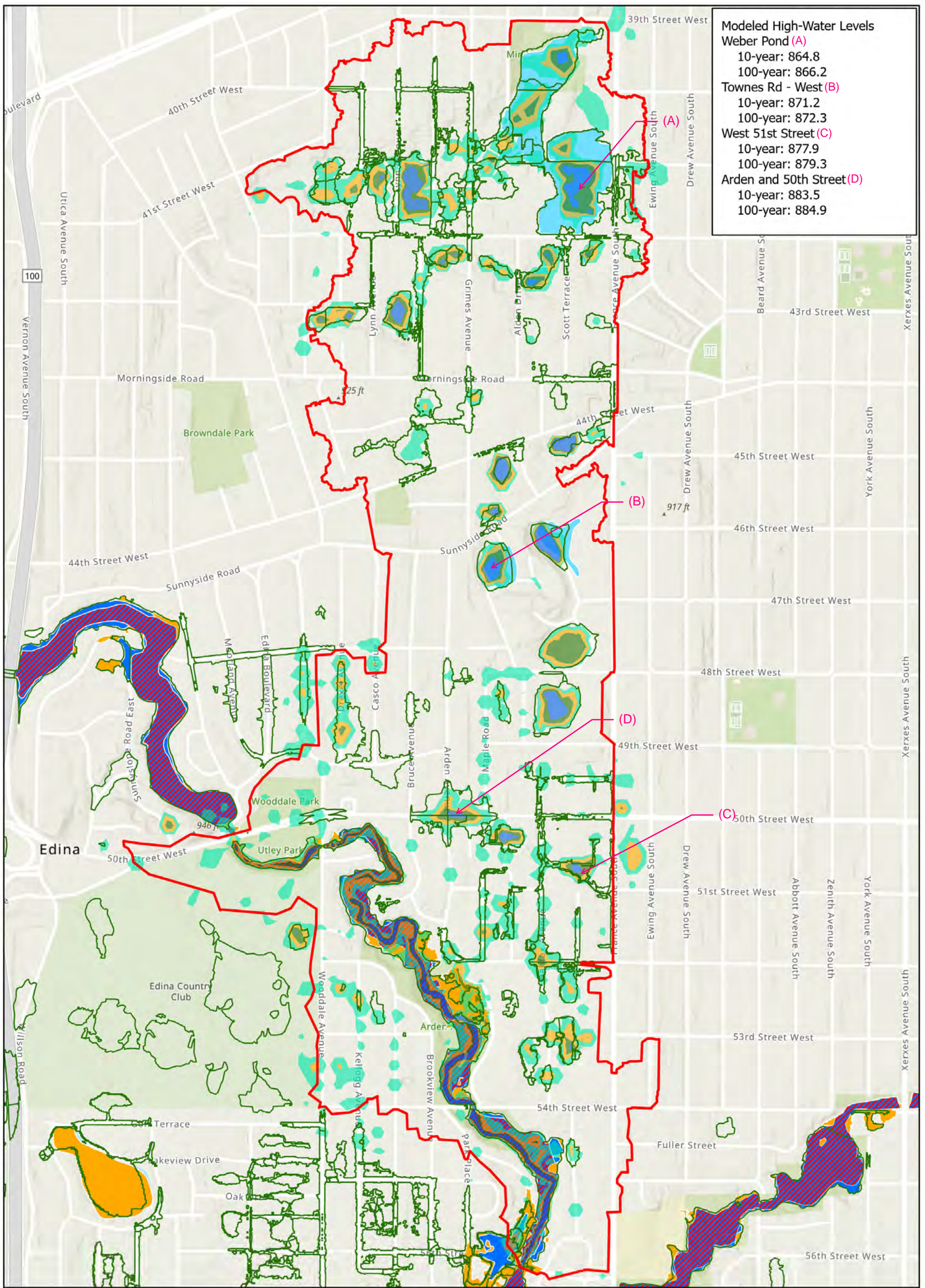
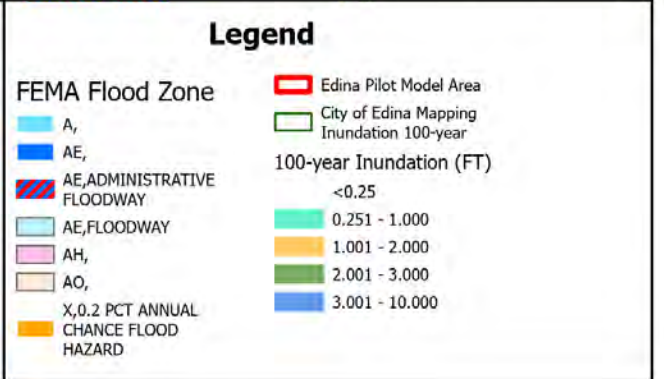


Exhibit 2B

ICPR - Flood Inundation Comparison

100-year, 24-hour Rainfall Event

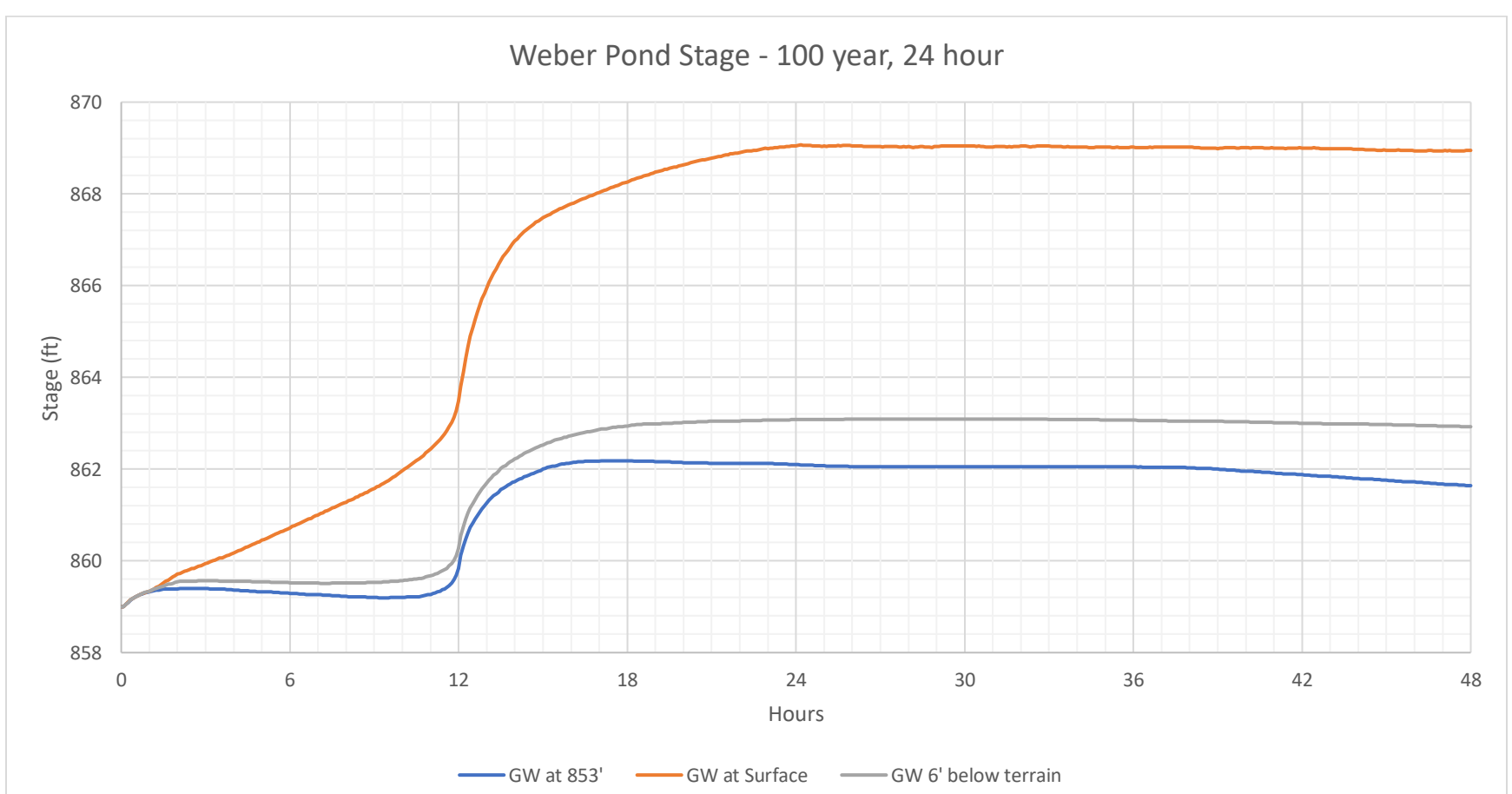
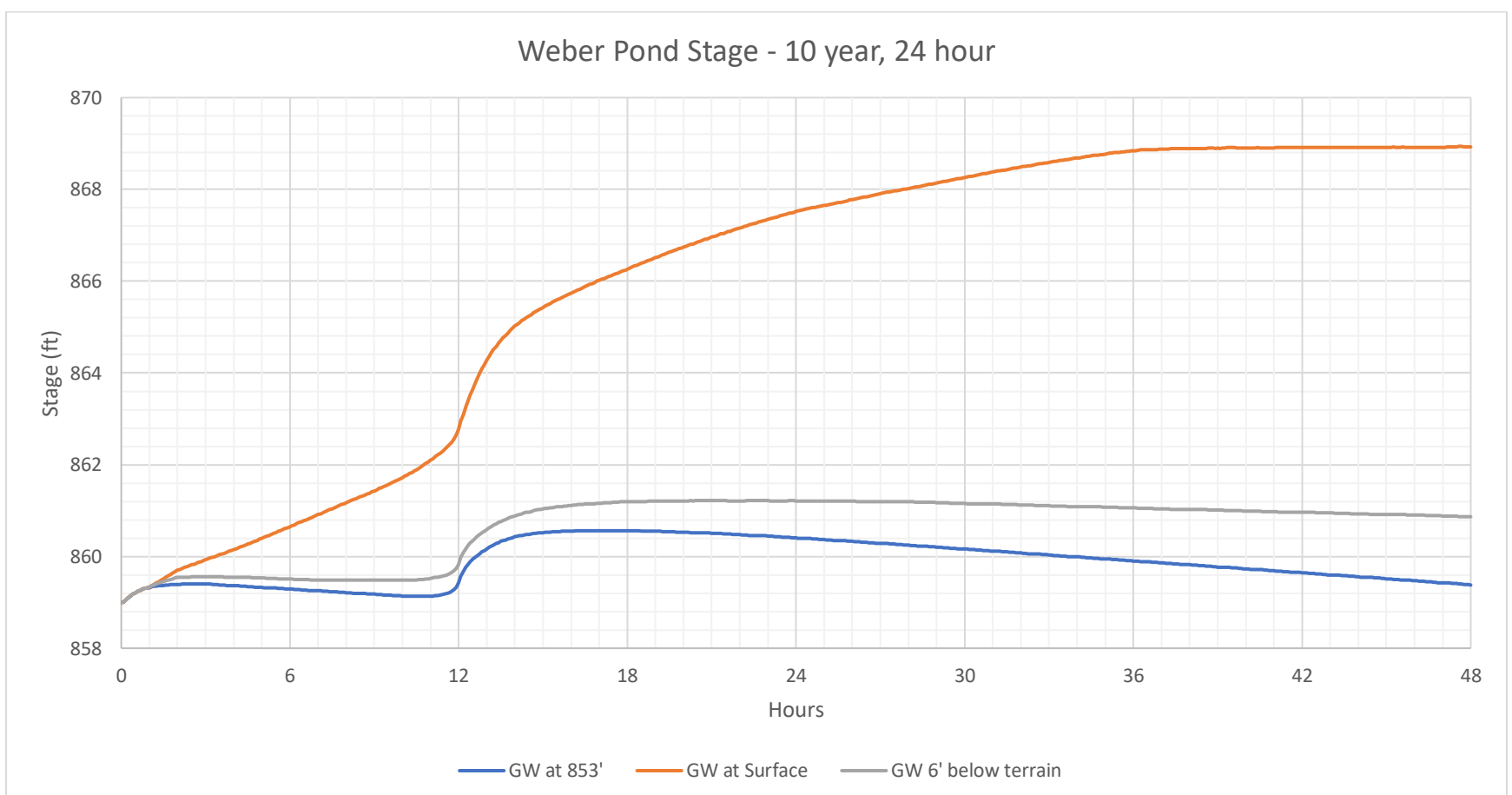
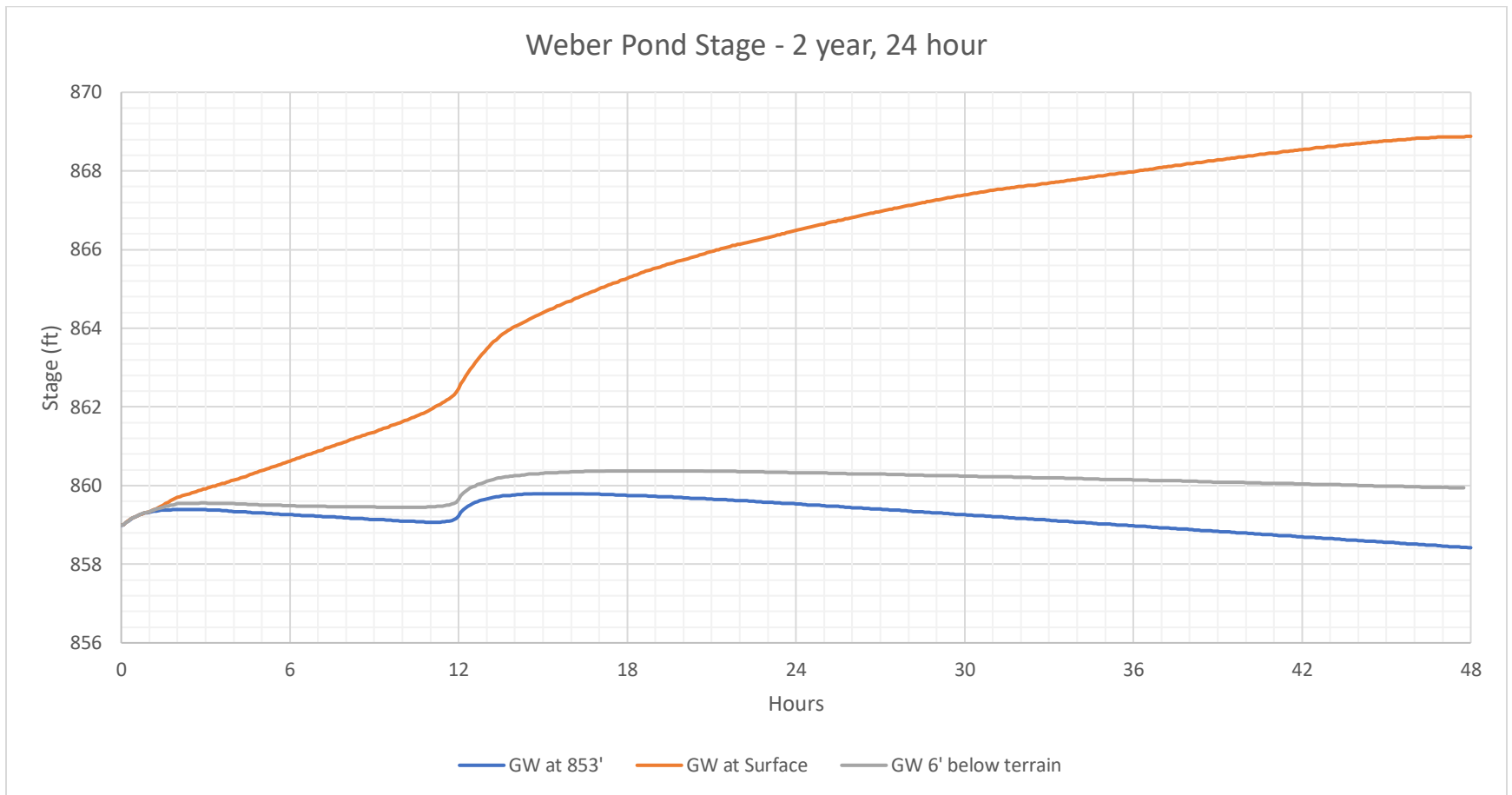
04/26/2023



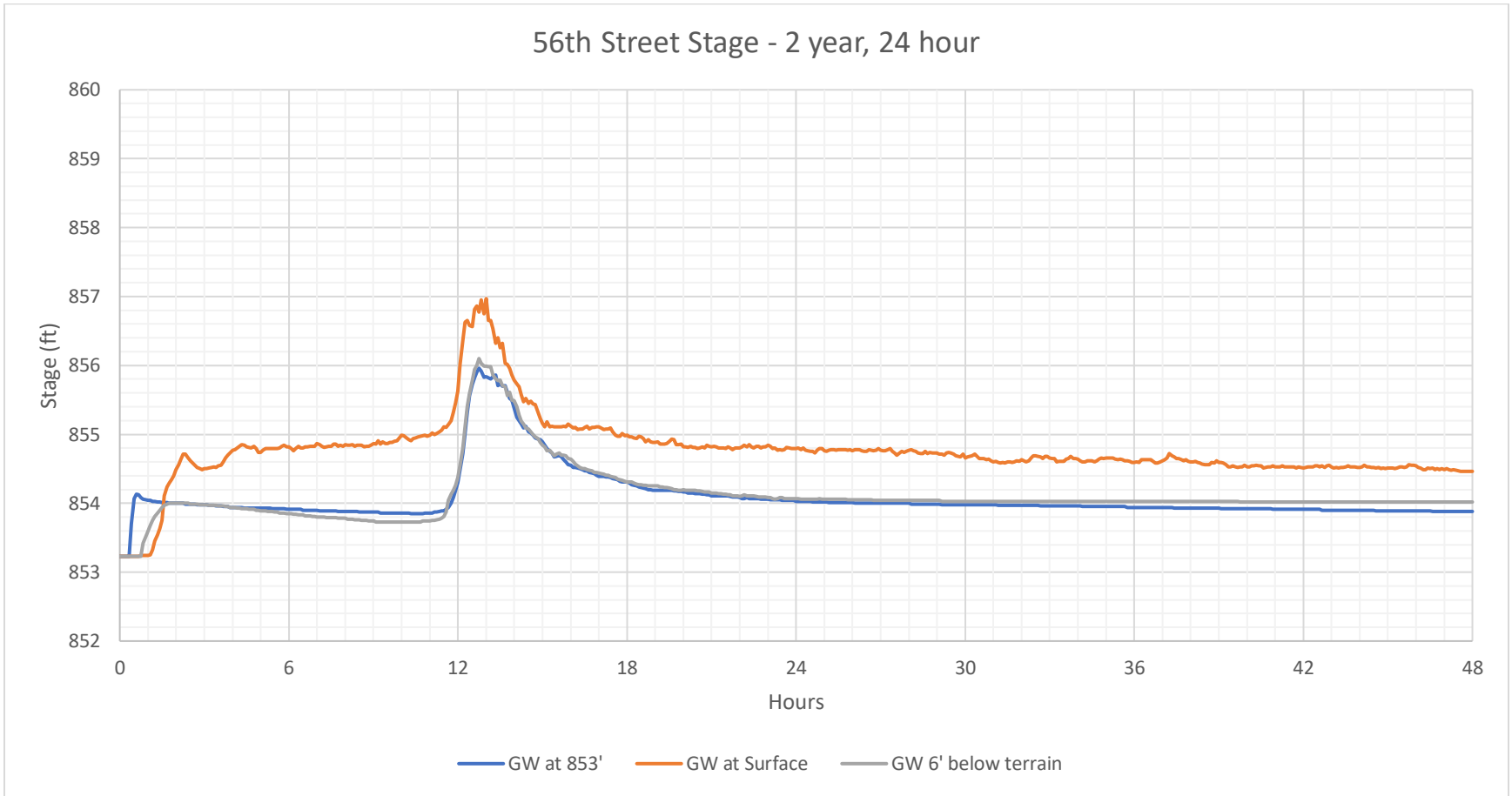
APPENDIX B

GROUNDWATER INFLUENCE EXHIBITS

APPENDIX B – GROUNDWATER INFLUENCE



APPENDIX B – GROUNDWATER INFLUENCE



APPENDIX F – EVALUATION FRAMEWORK MEMORANDUM

MEMORANDUM

To: Kailey Cermak, Project Manager | Minnehaha Creek Watershed District
From: Ron Leaf, Project Manager | Kimley-Horn
Date: March 2022 (Revised May 2023)
Subject: 2D Pilot Model Build - Evaluation Framework Memo

BACKGROUND

The Minnehaha Creek Watershed District's (MCWD or District) current modeling tools are dated and do not provide the granularity and features necessary for the District to effectively manage and adapt to climate change. District staff identified the need to develop a new modeling tool that has greater granularity that can characterize and quantify the impacts of climate change and evaluate a range of scenarios to shape climate adaptation strategies, programmatic policies, and specific projects.

MCWD began the model selection process by conducting a cursory assessment of several two-dimensional modeling software packages. This screening-level assessment (included as an appendix), along with vendor information sessions and consultation with agency experts, led the District to narrow their focus to ICPR and ICM. Both software packages were selected for evaluation through two distinct subwatershed areas during the pilot model build analysis, giving the District an opportunity to comprehensively compare the two. The District chose to pursue a pilot model build, ahead of the full watershed-wide build, to mitigate for the relational and technical risk that is often associated with large-scale, high-resolution models such as selecting the right software for the intended use.

A key objective of the pilot model build project and the first task within the scope of work is to establish a clear, comprehensive evaluation framework that the District can ultimately use as a resource to:

- 1) Inform which of the two models is best suited to meet the District's current needs; and
- 2) Understand the operational considerations and challenges of scaling the model watershed-wide.

This memorandum outlines key components of the evaluation framework and how the information will be used by the District in support of making an informed decision on which modeling platform to advance to the watershed wide model build phase of their initiative.

FRAMEWORK OVERVIEW

The purpose of this portion of the memo is to describe how the framework was developed and to provide an overview of its structure. It was important that the evaluation framework serves the District for its current model evaluation process and can be re-used in the future as the District's modeling needs change and new tools need to be evaluated. To accomplish this, the project team concluded that the framework must evaluate models based on a combination of model use criteria and model operational criteria. This approach provides a broad view of each model's range of capabilities evaluated against the District's primary intended use cases.

The framework structure and definitions are as follows:

MCWD Model Uses

The first step in a model evaluation process is defining clear objectives since each software package provides unique features and capabilities. While the District relies on modeling for a variety of uses, the upcoming watershed-wide model's primary purpose is to serve climate adaptation planning work. It's important that emphasis gets placed on modeling features that directly support the District's primary use. Therefore, there are two categories within this section, which include:

- 1) *Primary Uses*: The primary model uses section lists what the MCWD has identified as priority needs and uses from the upcoming watershed-wide model. Emphasis will be placed on this category during model selection.
- 2) *Secondary Uses*: The secondary model uses section lists capabilities from which the District and its partners would benefit, although these metrics will not drive model selection.

Model Operations

The ability to efficiently build, run, and export data are also important considerations when developing a model. Having a thorough understanding of these operational-level metrics will shape how the District chooses to scale and maintain the upcoming watershed-wide model. The categories within this section include:

- 1) *Data Processing*: This category includes metrics related to how raw datasets are processed into model-build ready datasets. While this category is important to evaluate if either model has any significantly different level or format of data import needs, the end result of those differences would likely relate more to how much effort a user may need to expend during the pre-model build process to ready the data for import into the model. The other differentiator relates to how effectively the scripting process can process the raw datasets into a model-build ready format.
- 2) *Model Build Processes (Including Calibration and Validation)*: This category is intended to assess the metrics that impact the ability of the automated model build scripts to create a functioning model. Important differences in the two models will relate to how well each accepts and connects the surface grid interaction with the subsurface conduit data through the automated processes. Two model maintenance metrics are included to differentiate between the ease of automated model updates and the more manual maintenance process for model version and security. Significant attention was placed on the calibration process and validation process during the pilot study.
- 3) *Model Function and Results (Scenario Analyses)*: This category focuses on metrics relating to obtaining reliable and repeatable model results, comparing output capabilities of the two models and how well each model is suited to a range of scenario planning situations. This category is the most directly related to each model's ability to meet the District's primary modeling goals.
- 4) *Software Specifics*: This category captures a range of metrics that do not fit directly within any of the previous categories. In general, these are more administrative type considerations that are not likely to directly influence model selection. Instead, these metrics are more likely to inform the District on considerations that will impact costs of owning the model, watershed-wide scale-up issues and/or long-term maintenance and administration of the model. As the project and evaluation process progresses, additional categories may be carved out of this section.

EVALUATION PROCESS

Approach

The matrix metrics were intentionally established at the beginning of the project, so the project team would have a designated place to track observations and data points about each model. The general approach was to provide space for qualitative assessments, while also being able to record quantitative study results to provide an objective comparison of the two models. The final version of the matrix presents the following assessment information:

- 1) ICM/ICPR Evaluation Process Comments / Observations. Comments and observations included here represent a combination of the pilot modeling team input from the pilot study experiences and, in some areas, input directly from the model developer has been included.
- 2) Summary / Comparison. Based on the full body of comments from the pilot modeling team and comments provided by the model developers, this column includes a comparison of where the two models differ or a statement that both models are capable in this area and there is no apparent difference based on the scope and results of the pilot study.
- 3) Rating. The rating format assigns a 0-1-2 level for each Evaluation Factor based on the model's relative capability for that factor. A "0" rating defines the model as not capable or having only weak capabilities; a "1" rating defines the model as being proficient for that factor; and a "2" rating defines the model as having strong capabilities for that factor. Where one model rated higher than the other, that box has been highlighted either blue (for ICM) or green (for ICPR), to better emphasize where model differences were observed. Scores were not totaled only for the matrix as a whole and not for each category. The total score should be considered a data point that confirms both models have wide capabilities and should not be considered an absolute numerical ranking of each model.

A description of how each section of the matrix was populated and scored is provided below.

Model Use Metrics Assessment

The primary and secondary model uses represent two categories of metrics within this evaluation framework. These two categories of factors include some of the most common model capabilities the District currently uses and some additional model functions that the District see as the providing the greatest program related value in the future. The evaluation of these model use factors or metrics is designed to provide an overview of how the two models differ in their abilities as it relates to known upcoming uses. The lead ICM/ICPR modelers, in coordination with model Quality Control (QC) leads, populated these sections prior to commencement of the model build and then revisited each of the responses after completing each subsequent step in the model build and scenario analysis phases of work. These updates were intended to confirm whether the initial assessment is still accurate and to supplement observations based on the completed pilot study phase.

Operational Metrics Assessment

Similar to the Model Use metrics, a qualitative assessment of each of the operational metrics was initially completed by the respective ICM and ICPR modeling leads and GIS/Software experts during the initial phases of work to process data needed for model builds, build and calibrate the models and ultimately complete the scenario analyses. The goal with these factors is to define the specific capabilities of each model related to a given metric and draw attention to any unique or significant benefits or drawbacks.

Upon completion of distinct model build phases, the ICM and ICPR modeling leads and GIS/Software experts revisited the initial comments and observations.

MODEL EVALUATION

The original intent of the evaluation matrix process was to provide an objective comparison framework for the two models. The initial framework included a numeric rating process that was removed in favor of the current 0-1-2 rating format to provide a more representative direct assessment of the model capability for each factor. This approach supports the goal of the matrix being to inform the decision by the District, but not to produce a decision based purely on the final score.

At the outset of the pilot study, District staff were not anticipating that the evaluation process would reveal a clear and easy choice and desired a broad range of evaluation factors within the evaluation matrix to refer back to as the final assessment of both models took place. Each model will likely deliver on some primary uses better than the other and each model's overall capabilities comes with its own unique operational considerations and challenges. The District is prepared to prioritize selecting a model based on its ability to meet its primary uses, even if it means it will be operationally more difficult.

A summary of the findings from this evaluation process and insight on each model's ability to scale and deliver on the District's primary uses will be discussed in the final project report.

ATTACHMENTS

1. Evaluation Metric Descriptions
2. Evaluation Framework Matrix
3. Aquaveo Screening Study

EVALUATION METRIC DESCRIPTIONS

Primary Model Uses

1. *Produce channel and localized flood inundation maps*
 - a. The ability to present the occurrence of flooding in a particular location within the model and through export of model result to GIS or other platforms for producing maps to illustrate flooding extents and/or depths.
2. *Run long-term back-to-back extreme wet or dry years to evaluate groundwater-surface water interactions*
 - a. The does model have the ability to discern results related to impact of ground water table and antecedent moisture content on infiltration rates in existing and proposed stormwater features. Is there capability to assess ground water-surface water interaction during the model run.
3. *Evaluate impacts of current and alternative regulation/policies on surface water quantity*
 - a. Review results and impacts to water levels in surface water features based on changes in landuse from development and changes in infiltration rules applied on a project scale or land area scale, for example.
4. *Quantify the impact of regional volume management strategies (Projects/BMPs)*
 - a. Ability to extract additional results information at specific surface water features throughout the watershed based on applying a combined developed condition with volume control requirements applied.

Secondary Model Uses

5. *Short-term channel and localized flood forecasting (all seasons, consider snowmelt)*
 - a. Ability to use future projections of rainfall and snowmelt conditions to efficiently predict flow and runoff results throughout the watershed and within creek channels.
6. *Characterize water quality changes / impacts*
 - a. Ability to determine impacts of additional development on water quality parameters and the presence and movement of contaminants through the watershed.
7. *Provide boundary conditions for other models*
 - a. Set and/or extract boundary conditions at edges of subwatershed-scale model boundaries to benefit partner model development.
8. *Establish updated FEMA certified flood maps*
 - a. Determine the impacts to riverine flooding conditions based upon proposed/future modifications to the watershed and have the model results accepted by FEMA as the official basis for base flood elevation inundation mapping.

Data Processing

9. *Accepted file formats of input datasets*
 - a. Diversity of import dataset types and formats.
10. *Repeatability of data process to model build ready data*
 - a. Is the process of data preparation repeatable? And easily repeatable?
 - b. Scale of preprocessing effort to efficiently build a baseline model of a subwatershed or the entire watershed build out.

11. *Manual processing effort to get model input data ready for model import. Infrastructure data and geospatial data.*
 - a. Additional manual processing effort for import of standard infrastructure and surface features (e.g., gravity pipes, landuse delineations, inlets)
12. *Manual data processing feedback loops. Ability to export manually adjusted data to external geodatabase.*
 - a. Import/export of manual data changes for future tracking and to reduce of duplication of effort and potential for missing updated data.

Model Build Processes

13. *Model node limitations (scale capabilities)*
 - a. Is there a model node limit, actual or practical? Is there a large change in process to build the model when adding additional nodes/links within model?
14. *Default hydrology method and processing.*
 - a. *What is or are the available default hydrology methods and are they applicable to District needs and uses.*
15. *Watershed-wide construction considerations. Single watershed wide model versus multiple smaller model areas.*
 - a. Ability to efficiently develop the baseline watershed-wide model and does the model format allow for variations and detail between single area and watershed-wide builds.
16. *Ability to carve out smaller sections of the model.*
 - a. Ability to efficiently increase model level of detail in areas where finer results are required. Does the finer model build area/version allow for automated update to the base model.
17. *Model resolution required to support primary uses*
 - a. Baseline model resolution in terms of 2D grid sizes to support the primary uses. Is the mesh a constant or does it allow for variable mesh size to capture greater detail in areas of interest such as where steep grades are present.
18. *2D overland mesh methodology*
 - a. What is the mesh creation methodology and what is the resulting need for additional hand edits to the mesh for baseline model development.
19. *1D-2D Connection Points.*
 - a. *What is the models approach and options for connecting the 2D surface mesh with the 1D features such as pipes and inlets to the pipes.*
20. *Pump system functions/capabilities*
 - a. Ability to model existing and proposed pump stations and forcemains within the watershed.
21. *Method/approach to calibration*
 - a. Steps to review model results and multiple scenarios in terms of adjusting inputs/parameters for calibration to one or more known events/observed results.
 - b. Steps to review baseline model build results with respect to observed events and results. Specifically, the ability to relate model results to other previously calibrated models and calibrated results.

Model Function and Results

22. *Ease and Options for BMP Evaluation*
 - a. Input process for proposed BMPs, any unique parameters and simulating the resulting effects of the modifications.
23. *Ease of Land-Use Change Scenarios*
 - a. Effort and process to swap out land-use delineations or parameters in a given area.
24. *Model runtime (common processing system)*
 - a. Overall model run times on common computer comparison.

- 25. *Results quality and output format*
 - a. Quality of results data, broadness of results in model and in exports.
 - b. Exported results resolution and assessment of consistency of exported results (i.e., exported data the same as viewed in the model)?
- 26. *Export process and format*
 - a. The ease of exporting results for use in outside programs and analysis.
 - b. The default format for exporting results, and any additional options for exporting results.

Software Specifics

- 27. *Sharing model versions*
 - a. Formats for sharing models or portions of a model.
 - b. Issues in sharing models, sending to others (internally and externally).
- 28. *Local versus network – processing ease*
 - a. Options for performing model simulation runs on a local device or network and the change in model run times and model saving considerations.
- 29. *License type and cost*
 - a. License cost, types, features.
- 30. *Model maintenance (version management, security, technical support)*
 - a. How does model maintenance work? Is the software updated often? New features in the works? Is the technical support provided easily accessible and responsive? Are there any unique security issues related to model storage and sharing.
- 31. *User Community*
 - a. Size and extent of users across the region/country.



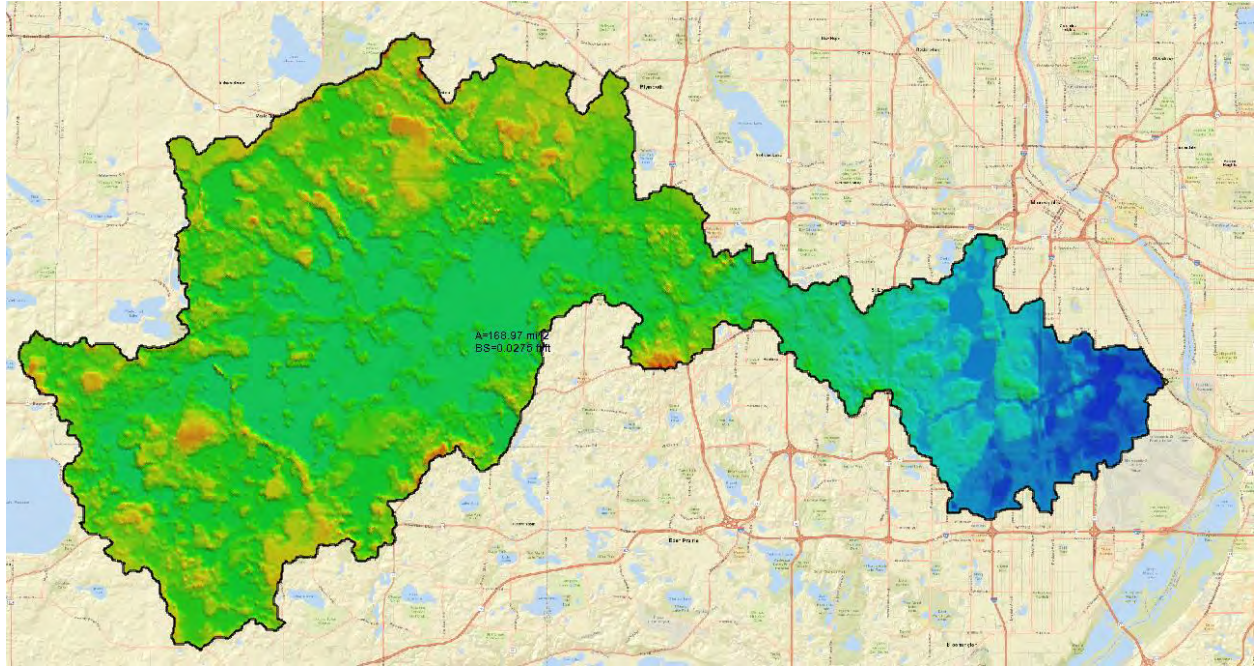
EVALUATION FRAMEWORK MATRIX

MCWD 2D PILOT MODEL BUILD
EVALUATION FRAMEWORK MATRIX
31-Jul-23

| Evaluation Category | Line ID | Evaluation Factor / Description | Evaluation Process Comments / Observations | | Summary / Comparison | Rating (0 - not capable or weak; 1 - proficient; 2 - strong) | |
|--|--|---|---|--|---|--|------|
| | | | ICM | ICPR | | ICM | ICPR |
| | | | | | | | |
| MCWD Primary Model Uses | 1 | Produce channel and localized flood inundation maps | Capabilities to view flood animation results within the model (inundation depth, velocity). Fully feasible to produce results and maps to depict out of bank and localized inundation areas within the model or through export to other tools. 2D results elements (points, line, polygons) can also be used to review flooding and simulation results along and within common locations over multiple model simulations. | Capabilities to view flood animation results within the model (inundation depth, velocity). Feasible to produce results and maps to depict out of bank and localized inundation areas within the model or through export to other tools. | Both models fully capable to produce inundation mapping. | 2 | 2 |
| | 2 | Run long-term back to back extreme wet or dry years to evaluate groundwater-surface water interactions | Not feasible. Model does not account for two-way surface water/groundwater interaction. Can mimic antecedent conditions through soil moisture at the beginning of the simulation. Uses Green-Amp parameters to mimic antecedent soil moisture at beginning of simulation. | Model can dynamically model groundwater via 2D network or simpler methods, and multiple scenarios are possible. | ICPR tracks horizontal groundwater movement and ICM does not. ICM simulations are based on a starting GW condition and do not vary throughout a long-term simulation, like ICPR. | 0 | 2 |
| | 3 | Evaluate impacts of current and alternative regulation/policies on surface water quantity | Options to spatially update land-use through hand edits or re-import of land use delineations. Land-use delineations and parameters can be varied by scenario to allow for additional flexibility. | Able to manipulate through land use map layers via spatial update, or via tabular property update, for entire study area on the fly. | Both models fully capable and have wide capabilities. | 2 | 2 |
| | 4 | Quantify impact of regional volume management strategies on surface water quantity (projects/BMPs) | Use the 2D results elements to review infiltration rates and volumes for proposed regional BMPs or specific policies. | Somewhat limited in ability to quantify smaller BMP impacts en masse, but smaller elements can be implemented manually. ICPR can interact on multiple model scales within a single model. | Both models fully capable and have wide capabilities. | 2 | 2 |
| MCWD Secondary Model Uses | 5 | Short-term channel and localized flood forecasting (consider snowmelt) | Feasible | Feasible; snow melt can be further varied via baseflow input parameters, likely manual edit within the software. | Both models fully capable and have wide capabilities. ICM refers to a different snowmelt analysis than what is current practice in MN. | 2 | 2 |
| | 6 | Characterize water quality changes/impacts | Not directly evaluated during pilot study. Software developer input and model documentation refers to multiple functions and capabilities to model multiple parameters, including sediment, salt, colloids, phosphorus and many others. | Not directly evaluated during pilot study. Software developer input and model documentation refers to 1D water quality modeling in ICPRA Pro, and plans to release a new water quality module in late 2023. | ICM has capabilities to model TP and other parameters of interest to the District. ICPR does not currently have any capabilities to track nutrients or pollutants. | 1 | 0 |
| | 7 | Provide boundary conditions for other models | Feasible to export and use stage and flow hydrographs at any location. | Feasible to export and use stage and flow hydrographs at any location. | Both models fully capable. | 1 | 1 |
| | 8 | Establish updated FEMA certified flood maps | Not currently approved nationally by FEMA. Has been locally approved in selected locations. Developer is actively seeking national approval from FEMA. | Not currently approved nationally by FEMA. Has been locally approved in selected locations. | Neither is nationally accepted currently. Both have examples of being accepted regionally/locally. ICM may likely be accepted sooner as they are actively seeking | 0 | 0 |
| Data Processing | 9 | Accepted file formats of input datasets | Shapefiles, Geodatabases, CAD, Text | Shapefiles, CAD (.dxf), CSV tables | Both models fully capable and have wide capabilities. | 2 | 2 |
| | 10 | Repeatability of data process to model build ready data | Use of MGS and Configuration files for consistent import of infrastructure data. | Use of MGS and Configuration files for consistent import of infrastructure data. | Both models equally capable/functional. | 1 | 1 |
| | 11 | Manual processing effort to get model input data ready for model import. Infrastructure data and geospatial data. | Manual effort required for the following cleanup steps: US/DS node name for pipe, artificially high pipes due to DEM fill of invert. Non-snapping of nodes to DEM in model space, node elevations needs to be pre-defined for pipe outlets to match the pipe invert. | Manual effort required for US/DS nodes on some pipes missed, DEM invert reversions, repeat pipe names. 1D connections for catchbasin (inlets) starting elevation need to be overwritten and assigned pipe invert elevation or additional water introduced to system at beginning of simulation. Nodes cannot be easily swapped out for 1D connections (live in separate buckets: 1D object vs. 2D feature) | For 1D infrastructure elements - no significant difference. For 2D elements ICPR takes additional effort to set groundwater layer for functionality. ICM does not have. | 1 | 1 |
| | 12 | Manual data processing feedback loop. Ability to export manually adjusted data to external geodatabase. | Allows for export of pipe/junction parameters along with flags to denote original value or updated value in export attribute table. | ICPR has a "difference" tool that can compare and report on additions, deletions, and changes between scenarios for 1D input data. | Both models equally capable/functional. | 2 | 2 |
| Model Build Processes (Including Calibration and Validation) | 13 | Model node limitations (scale capabilities) | There are no limits to the number of nodes (unlimited license). | No limits, may have practical limit when compared to run times. Groundwater mesh practical limit around 38,000 nodes due to matrix solver. Can get around this by creating multiple groundwater meshes. | No significant differences in capabilities. ICPR has some recommended approaches related to addition of the 2D groundwater mesh. | 2 | 2 |
| | 14 | Default hydrology method and processing | Green-Amp, Horton, Constant Rate, Fixed Rate | SCS Curve Number, Green-Amp, Vertical Layers Green-Amp | No significant difference. Both have options for varied methods. | 1 | 1 |
| | 15 | Watershed-wide construction considerations. | Ability to introduce various levels of detail through scenario manager. Feasible to build a single overall watershed-wide model with multiple scenarios with additional data recommended. Also feasible to build several subwatershed-specific models. | Ability to incorporate various levels of detail through model build. Feasible to build single model for entire watershed possible while considering the developer's recommended practical limits on the groundwater mesh size. Ability to scale groundwater module watershed-wide, but may reduce resolution. Multiple smaller models would allow for higher resolution in groundwater mesh. Combined SW and GW mesh will require additional attention and careful construction of watershed wide build. | No significant difference for surface water. Groundwater capability for ICPR requires additional effort and adjustment/calibration iterations to get it built well and running efficiently for single model or for multiple smaller models. | 2 | 1 |
| | 16 | Ability to carve out smaller sections of the model. | Capability to create new sub-model to allow multiple scenario runs of smaller areas. Cannot push sub-model edits back to full model. Changes to base model are automatically updated in sub-model. | Cannot push model edits back to full model. | Both have a scenario manager function/option that allows for separate/discrete model areas to be created and saved. ICM retains connection of sub-models to base model. | 2 | 1 |
| | 17 | Model resolution required to support primary uses | Automated variable mesh generation tool allows for development of mesh with sufficient detail to meet primary use needs (exception of groundwater). | Automated variable mesh resolution results in flexibility when only overland flow module is being used. To couple overland flow and groundwater, the mesh must be manually created using breakpoint offsets. Mesh must be transferred from overland flow to groundwater mesh. | Both models equally capable/functional. | 1 | 1 |
| | 18 | 2D overland mesh methodology | Triangular elements represent the average elevation within the element. Breaklines, void areas and walls can also be included in the mesh development to simulate hydraulically significant features. | Three elements; triangles represent links, hexagons represent storage/land use parameters, diamonds connect hexagon edges. Breaklines must be enforced along flow paths to allow water to move freely. Breakpoints are recommended. All overland mesh breaklines must be transferred to the groundwater region to ensure alignment between meshes. | Both require edits for breakline refinement of 2D mesh. ICPR requires more breaklines to define accurate flowpaths. ICPR requires additional refinement of GW mesh. | 2 | 1 |
| | 19 | 1D-2D Connection Points | 1D connections are specified through the standard node input. Using either the 2D, Gully 2D, or Inlet 2D flood pipe allows user input of inlet capacity. Can modify/inlet types and parameters after creation, also allowed for removal of inlet capacity (manhole). | 1D connections utilize the mesh elevation data to determine starting WSEL. User must specify alternate starting elevation when modeling 1D connection to storm sewer (e.g., catchbasin). 1D connections allow direct connection between overland flow region and 1D hydraulics, do not allow for inlet capacity limitation. To model inlet capacity, weir link required between 1D connection and stage/area node (manhole). | Both models equally capable/functional. | 1 | 1 |
| | 20 | Pump system functions/capabilities | Pumps can be included as part of the development. Additional user input to include pump curve and others needed to effectively model pump. Multiple pump types available. Allows for specific pump information and is fairly intuitive for users familiar with pump system modeling. | Pumps can be included as part of the development. Additional user input to include pump curve and others needed to effectively model pump. Multiple pump types available. Input and review of results required to accurately inform pump curve selection. | ICM may be slightly easier to use for pump systems, but the vast majority of pump pc scenarios will be essentially the same. | 1 | 1 |
| | 21 | Method/approach to calibration | Calibration was achieved through the adjustment of surface roughness changes and breakline delineation. No adjustments were required to reduce error during calibration. | Calibration was achieved through the adjustments of surface roughness changes, breakline delineation, breakpoint addition, and impervious value increases. Adjustments to the breakline placement were required to reduce model error to acceptable ranges. The groundwater mesh region required an additional step to add breaklines to the GW region that matched the SW region. Adjustment of breaklines in one region must be transferred to other region. | No significant difference in surface water only. ICM produced a lower error than ICPR with comparable calibration effort. ICPR requires additional adjustments and setups to achieve acceptable calibration related to added groundwater surface. | 1 | 1 |
| | Model Function and Results (Scenario Analyses) | 22 | Ease and options for BMP evaluation | BMP evaluation can be achieved through 1D element creation or 2D mesh manipulation depending on underlying level of data for BMPs. | BMP evaluation can be achieved through 1D element creation or 2D mesh manipulation depending on underlying level of data for BMPs. | Both models equally capable/functional. | 1 |
| 23 | | Ease of land-use change scenarios | Ability to change delineations and roughness values within scenario manager. | Straightforward via land use map layer or underlying data table manipulation. | Both models equally capable/functional. | 1 | 1 |
| 24 | | Model runtime (common processing system) | Design storm runs for both watersheds take less than 5 minutes to run. 100 year, 10-day event (30 day simulation) takes 2 hours to complete run. ICM generally runs fast but the modeler should review the 2D time step during the simulation run for issues. 1D hydraulic time step can have an effect on overall simulation run time. | Model run times are affected by overall stability of model and the use of 1D elements for lake/pond areas and creek channel areas. Varying the Freeball timestep parameters can achieve small reductions in model run times. When groundwater is actively passing water back to overland module, the simulation slows down. Increasing groundwater timestep allows for faster processing. | ICM is faster at comparable resolution sometimes significantly faster. | 2 | 1 |
| 25 | | Results quality and output format | Extensive results data review can be completed within the software. Most (if not all) results that vary with time can be reviewed on table or graph format (e.g., stage, velocity, infiltration rate, saturation). Easy to review changes to parameters and affect on output. E.g. changes in effective infiltration rate, flow direction from individual cells | Standard results can be viewed within the software (stage, flow arrow, depth to groundwater). Cannot easily view individual cell outputs outside of standard outputs. Output reports can be saved (2D Node Selection) for duplication over various scenarios. | ICM has better visual results (less noise) and more stable hydrographs for the more automated 2D build under this pilot study. ICPR stability may improve by going to 1D channels and ponds, for example. | 2 | 1 |
| 26 | | Export process and format | Ability to export individual timestep results and maximum results. Graphs and tables available for export. Use of selection lists to export key data/monitoring locations. MapInfo MIF, MapInfo TAB, shapefile, ESR geodatabase, CSV. The shapefile export includes the triangular mesh as a polygon shapefile. | Results from 1D network is exported from the report section of the model. Results for 2D inundation are exported from the Animation tab in the model Export reports can be saved to reuse in future. Text files, graphics, animations, DEMs from 2D. | Both models equally capable/functional. | 2 | 2 |
| Software Specifics | 27 | Sharing model versions | Model files can be stored in a multuser format assuming shared network location, or model can be packaged using a transportable database and sent to external user for modification. | Must keep track of versions when sharing model files. There is no multuser format. | ICM transportable database is more portable from a file size transfer standpoint compared to copying a full folder for ICPR. | 2 | 1 |
| | 28 | Local versus network - processing ease | Can process locally on computer or on a cloud computer, can save results locally or on a server location. New version has ability to be run on innoveye server to give faster run times. | Can process locally on computer or on a cloud computer, can save results locally or on a server location | Both models equally capable/functional. | 1 | 1 |
| | 29 | License type and cost | Subscription, \$18,000/yr for unlimited nodes and mesh elements (price may have changed with new subscription model from Autodesk). 2023 \$54,000 for 3 year subscription. Flex option may also be considered for limited use approach. | Subscription, \$2,400/yr/simultaneous user | ICPR lower annual license cost. | 1 | 2 |
| | 30 | Model maintenance (version management, security, technical support) | Currently model version updated each year with new version. innoveye seems to be very responsive to questions and solving problems that arise. | ICPR is updated periodically. Streamline Technologies has been very responsive to questions and solving problems that arise. | Both models equally capable and responsive technical support. | 1 | 1 |
| | 31 | User Community | Over 400 consultants/utilities using ICM in the US based on developer records. | Used widely in Florida by consultants and utilities. Based on developer input, also currently working with roughly a dozen universities for research and teaching purposes. | Minnesota/Midwest starting to see some ICM uses. Nothing notable for ICPR in this area. | 2 | 1 |
| | | | | | | 44 | 39 |



AQUAVEO SCREENING STUDY



1D/2D MODEL COMPARISON

Performed for Minnehaha Creek Watershed District

Submitted by:

Aquaveo, LLC

3210 N. Canyon Rd. Suite 300

Provo, Utah 84604

Contact: Tony Melcher

Email: tmelcher@aquaveo.com

Background and Description

Aquaveo was tasked with assessing the hydrologic and hydraulic modeling needs of the Minnehaha Creek Watershed District (District) and to provide a comparison of multiple numerical models. The purpose of this comparison is to provide the District with the information necessary for selecting a model to replace an existing XP-SWMM model of the Minnehaha Creek watershed. This legacy XP-SWMM model uses a lumped parameter sub-catchment representation of hydrology and rainfall/runoff processes. The District is looking to replace the model with a coupled 1D/2D hydrologic model that takes advantage of high-resolution elevation data. The model will be used to analyze urban flooding events and assess vulnerabilities.

The tasks that Aquaveo was assigned to complete are the following:

Task 1. Review the District's modeling needs assessment summary

Task 2. Prepare a comparison matrix of applicable model options.

Task 3. Prepare a short presentation summarizing findings and recommendations

Task 4. Provide project technical memorandum and finalized comparison matrix

Tasks 1 – 3 were completed prior to the preparation of this report. The document represents the deliverable for Task 4.

Model Comparison and Methods

The District's provided work order ("Aquaveo_modelreview_WorkOrder_Final.pdf") states that the model comparison must include the following models: TUFLOW, HEC-RAS (1D/2D), InfoWorks ICM, SRH-2D, xP2d/XP-SWMM, and MIKE FLOOD. The GSSHA and ICPR4 models were added to the list at the request of Aquaveo and the District. The criteria by which each model was evaluated are listed below:

- Model Input Formats
 - Shapefiles
 - Geodatabases
 - CAD
 - Text files
- Editing Model Inputs
 - Methods for editing inputs
 - Ease of editing inputs
 - Ease of modifying model scenarios and BMPs
- Model Output Format
- Groundwater Representation
 - Groundwater or Infiltration included
 - Method for representing groundwater or infiltration
 - Groundwater/Streamflow interaction

- Modeling Capabilities
 - Hydrology
 - Hydraulics
- Parallelization
 - Included
 - What method is used if included
- User Community
 - Publications
 - Model is well documented online
 - When was the last update of the model?
- Cost
 - Single cost or subscription
 - Cost of licensing method

The model comparison matrix is given in below.

Table 1. Model Comparison Matrix

| Modeling Software | Shapefiles | Geodatabases | CAD | Text files | Method for editing | Ease of Modification | Ease of Implementing BMPs | Format | Included | Method | Surface water exchange? | Hydrology | Hydraulics | Allowed | Method | Publications | Well Documented | Last updated | Single or subscription | Cost for License |
|-----------------------|--------------------|--------------|------------|------------|--|--|---|---|--|---|--|--|---|------------------|---|--|---|--|------------------------|---|
| | Model Input Format | | | | Model Inputs | | | Model Output | Groundwater | | | Modeling Capabilities | | Parallellization | | User Community | | | Cost | |
| Tuflow (classic, HPC) | Yes | Yes | Yes | Yes | Multiple Interfaces/ Programmatically | Some text editing, Easy with GIS software | BMPs and LIDs are not built-in. Roughness can be modified to represent land use change and hydrograph attenuation | Output depends on the interface selected, includes ASCII, XMDf | Infiltration and Groundwater | Green-Ampt, Horton | Once groundwater level reaches surface, no more infiltration is allowed. | Can specify rainfall and infiltration for basin areas | Yes, primarily a 1D (ESTRY)/2D hydraulic model | Yes | If purchasing TUFLOW HPC, must also purchase GPU option | 47 in 2021 (Google Scholar) | Online manual, support, forum | Software released in 10/2020, manual released in 3/2018 | Subscription | \$6,000 plus annual fees |
| HEC-RAS 2D | Yes | No | Yes | Yes | HEC-RAS interface/ Programmatically with text files. | Depends on familiarity with HEC-RAS software. Some editing can be done using text files. | BMPs and LIDs are not built-in. Roughness can be modified to represent land use change and hydrograph attenuation | .dss, .tif file for 2D results | No | No | No | No, however, recent update (v6.0) allows for spatial precipitation and infiltration). | Yes, 1D and 2D, does not allow for piped networks. | Yes | Built-in OpenMP parallelization (cpu) | 788 in 2021 (Google Scholar) | Online manual, large user community | Dec-20 | Single | Free |
| InfoWorks ICMOne | Yes | Yes | Yes | Yes | InfoWorks ICMOne interface/ Ruby scripting | Depends on familiarity with InfoWorks interface | Multiple LID options and processes. Applied to polygons and subcatchments. Can be edited programmatically | Mapinfo MIF, Mapinfo TAB, Shapefile, ESRI geodatabase, CSV | Yes | Uses SWMM infiltration methods for subcatchments (i.e., GA, Horton) | Groundwater is not connected to surface water features. Performs soil moisture accounting. | Yes, 2D mesh based overland flow. | 1D pipe hydraulics, 2D river hydraulics/dam breach analysis | Yes | Claims to be "multi-core and multi-processor aware." Can take advantage of GPU for 2D analysis | 45 in 2021 (Google Scholar) | Online videos and tutorials. User manual appears to only be available to paying customers. | 2021 | Subscription | \$30,000/yr for unlimited nodes and mesh elements |
| SRH-2D | Yes | No | Yes | Yes | SMS Interface/ programmatically | Depends on familiarity with SMS interface | BMPs and LIDs are not built-in. Roughness can be modified to represent land use change and hydrograph attenuation | ASCII, XMDf, text formats | No | No | No | No | Yes, 2D depth averaged St Venant | No | No | 21 in 2021 (Google Scholar), endorsed and funded by FHWA | Aquaveo/FHWA provides training courses, tutorials, videos, and wiki page | 2021 | Single | \$5,115 plus annual fees |
| xP2d/XPSWMM* | Yes | Yes | Yes | Yes | Inputs edited within XPSWMM software | Requires a familiarity of XPSWMM | Multiple LID options and processes. Applied to polygons and subcatchments | ASCII and text output | Infiltration and Groundwater | Green-Ampt, Horton | Once groundwater level reaches surface, no more infiltration is allowed. | Can specify rainfall and infiltration for basin areas | Yes, primarily a 1D/2D hydraulic model | Yes | Allows for GPU parallelization | 31 in 2021 (Google Scholar) | Not well documented online | Couldn't find information online | No | No |
| MIKE FLOOD | Yes | Yes | Yes | Yes | Offers two interfaces/ programmatically | Requires a familiarity with DHI or ArcGIS software | BMPs and LIDs are not built-in. Roughness can be modified to represent land use change and hydrograph attenuation | Common raster formats and ASCII text files | Infiltration but groundwater is limited. | Not a super detailed soil moisture model. | Yes this is allowed. | Yes | Yes | Yes | Allows for GPU parallelization | 89 in 2021 (Google Scholar) | Aquaveo provides tutorials, videos, training courses, software suites | 2021 | Subscription | \$11,000/yr |
| GSSHA | Yes | No | Yes | Yes | WMS interface/ programmatically editing .prj file | Requires a familiarity of WMS. | LIDs and BMPs can be implemented by modifying infiltration parameters and roughness values. | ASCII, shapefiles | Yes | GA, Richard's | Yes | Yes, 2D grid overland flow, solves two-dimensional manning's equation. | 1D pipe hydraulics, 1D river hydraulics | Yes | Built-in OpenMP parallelization (cpu) | 27 in 2021 (Google Scholar) | Aquaveo provides tutorials, videos, training courses, GSSHAWiki page | 2020 | Single | Single - \$6,500 plus annual fees |
| ICPR4 Expert | Yes | No | Yes (.dxf) | Yes | ICPR4 interface. | Requires a familiarity with the ICPR4 interface. | Multiple ponds, wetlands, infiltration, percolation, french drains available. | Textfiles, graphics, animations, DEMs from animations. Not much information online about output file formats. | Yes | 2D Surficial aquifer groundwater flow | Yes, designed to model surface water body and surficial aquifer interactions. | Yes, 2D overland flow using the finite volume methods of the St. Venant equations. Spatially variable rainfall and evapotranspiration. | 1D pipes and canals. | Yes | Not much information is offered online. Website states "ICPR4 includes parallel processing, so multiple cores is advantageous." | 14 in 2021 (Google Scholar) | Limited online documentation. Help is provided through technical support and tutorials installed with the software. | Called Streamline Technologies and they said version 4.07.08 was released 2/9/2021 | Subscription | \$2,400/year/simu ltaneous user |

* Notified by Innovize that XP software would be discontinued

Conclusions and Recommendations

The model comparison matrix was completed according to previous knowledge of the model, online documentation, and speaking with company representatives. In this section, we will provide our conclusions from the study and our recommendations.

Based on our discussions with the District and information provided in the “Modeling Needs” document, it is apparent that the District is seeking a model that offers 2-dimensional overland flow and 1-dimensional piped stormwater network representation. This requirement eliminates some of the models compared in this study, such as HEC-RAS and SRH-2D as neither of those models offers the 1D piped network component. Additionally, we were notified by Innovyze that xP2d/XPSWMM would likely be discontinued sometime in the next few years. For that reason, it will be difficult to continue to find online support or documentation for the model.

Each of the remaining models (TUFLOW, InfoWorks ICMOne, MIKE FLOOD, GSSHA, and ICPR4) have their strengths and weaknesses. InfoWorks ICMOne appears to offer all the modeling capabilities the District needs, however, the cost of \$30,000/yr may be the deciding factor. MIKE FLOOD also offers many of the components that are required by the District, however, the cost of \$11,000/yr may be a factor. Additionally, MIKE FLOOD doesn’t perform the groundwater/infiltration calculations that the other models do. GSSHA is much less expensive (\$6,500 one-time cost plus annual fees) and provides the hydrologic and hydraulic components required, however, the user community for GSSHA is much smaller than for some of the other models. As a result, features such as the piped network and groundwater flow haven’t been tested as extensively and may contain bugs. The ICPR4 model, developed by Streamline Technologies, offers the ability to simulation 2D overland flow, 2D groundwater flow, and 1D piped networks. The cost is also much more reasonable at \$2,400/year. However, it’s not clear whether ICPR4 offers GPU parallelization, and the online documentation is extremely limited! For these reasons, our recommendation is that the District considers InfoWorks ICMOne, the less expensive TUFLOW model, or the ICPR4 model (with discretion due to the lack of online documentation) to meet their modeling needs.

Exhibit C: List of curated datasets currently available for this project

Subwatersheds: Minor subwatersheds delineated as part of the MCWD's XP-SWMM H&H development

Geologic Atlas: 2019 Hennepin County Geologic Atlas including the groundwater atlas and bedrock surface.

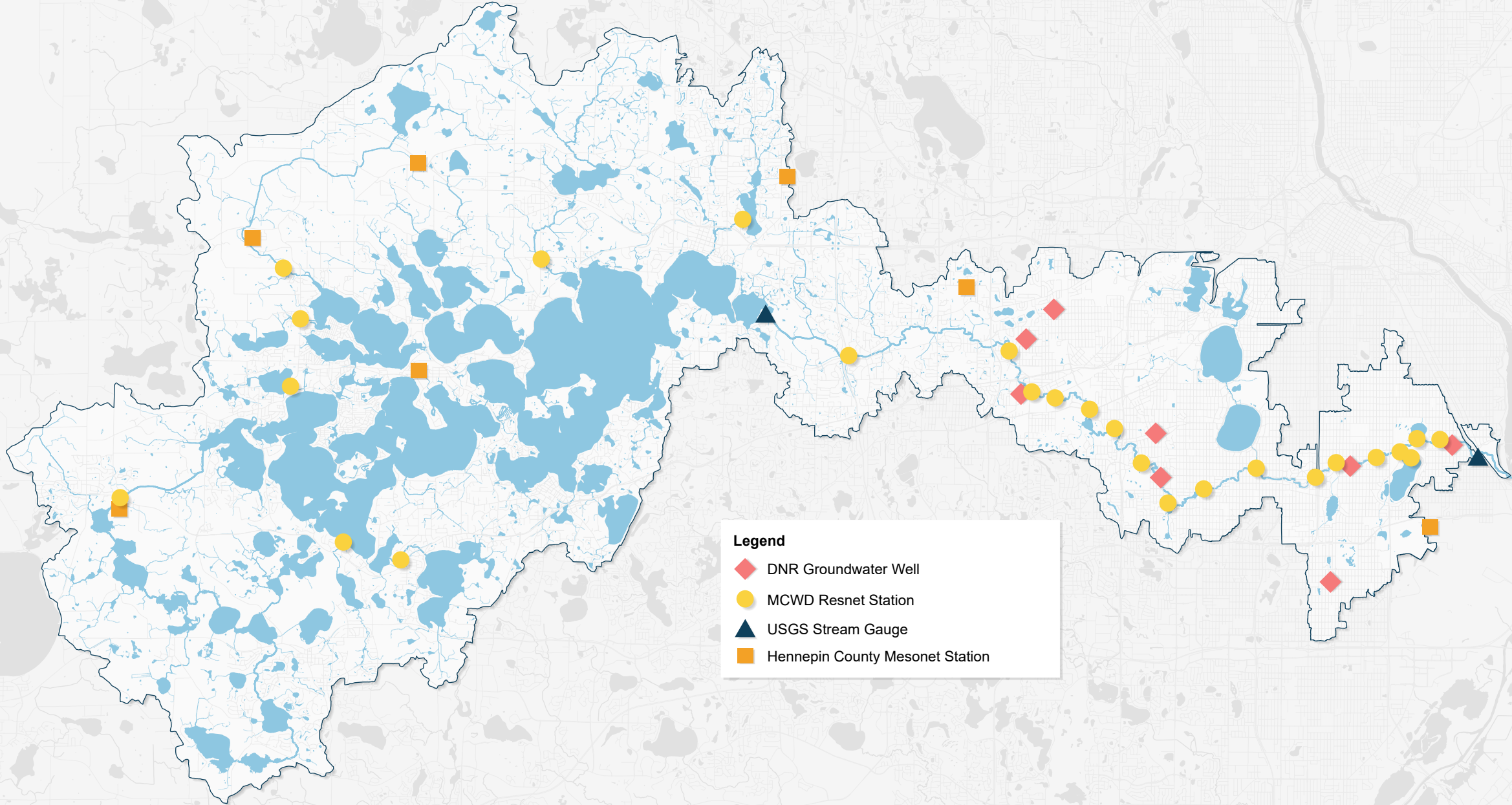
Soils: USDA SSURGO database for Hennepin County and Carver County.

Land Use: 2020 Landuse with calculated Mannings N and percent impervious based on MCWD's Pilot Model Python Scripting.

LiDAR: Digital Elevation Model and point cloud tiles that cover MCWD from 2011 State of Minnesota LiDAR.

Stormwater Infrastructure: Standardized stormwater network datasets, in MetroGIS Stormwater Geodata Project Standard (.gdb). This includes individual agency geodatabases and the watershed wide geodatabase.

Exhibit D: Map of Monitoring Locations



Legend

- ◆ DNR Groundwater Well
- MCWD Resnet Station
- ▲ USGS Stream Gauge
- Hennepin County Mesonet Station

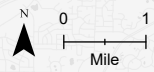


Exhibit E: LCCMR Grant Agreement

**STATE OF MINNESOTA
GRANT CONTRACT AGREEMENT
Environment and Natural Resources Trust Fund**

This grant contract agreement is between the State of Minnesota, acting through its Commissioner of Natural Resources ("State") and Minnehaha Creek Watershed District, 15320 Minnetonka Blvd, Minnetonka, MN, 55345 ("Grantee").

Recitals

1. Under [Minn. Stat. §84.026](#) the Department of Natural Resources is empowered to enter into grant agreements.
2. Under M.L. 2023, Chp. 60, Art. 2, Sec. 2, Subd. 041, Leveraging Innovations in Data Analytics for Project Implementation, \$738,000 the first year is from the trust fund to the commissioner of natural resources for an agreement with Minnehaha Creek Watershed District to integrate local and statewide data sets into a high-resolution planning tool that forecasts the impacts of changing precipitation patterns and quantitatively compares cost effectiveness and outcomes for water quality, ecological integrity, and flood prevention projects in the district. Minnehaha Creek Watershed District may license third parties to use products developed with this appropriation without further approval from the legislature or the Legislative-Citizen Commission on Minnesota Resources, provided the licensing does not generate income. This appropriation is subject to Minnesota Statutes, section 116P.10.
3. The State awards to the Grantee for the purpose of conducting the program entitled Leveraging Innovations in Data Analytics for Project Implementation in the manner described in the Grantee's approved Work Plan.
4. The Grantee represents that it is duly qualified and agrees to perform all services described in this grant contract agreement and Minn. Stat. Ch. 116P. Pursuant to [Minn.Stat. §16B.98, Subd.1](#), the Grantee agrees to minimize administrative costs as a condition of this grant contract agreement.

Grant Contract Agreement

1 Term of Grant Contract Agreement

1.1 Effective date: July 1, 2023

Per Minnesota Statutes Section 16B.98, subdivision 7, no payments will be made to the Grantee until this grant contract agreement is fully executed.

1.2 Expiration Date: June 30, 2026

1.2.1 The appropriation is available until June 30, 2026 by which time the project must be completed and final products delivered. For acquisition of real property, the appropriation is available for an additional fiscal year if a binding contract for acquisition of the real property is entered into before the expiration date of the appropriation. If a project receives a federal award, the period of the appropriation is extended to equal the federal award period to a maximum trust fund appropriation length of six years.

1.2.2 In the event this grant contract agreement is continued by way of amendment or new agreement, the date the amendment or new agreement is fully executed, is the end date. Notwithstanding the foregoing, in the event an amendment or new agreement is not fully executed within 60 calendar days of the stated expiration date, this grant agreement will expire on August 30, 2026.

1.3 Survival of Terms

The following clauses survive the expiration or cancellation of this grant contract agreement: 10 Liability; 11 State Audits; 12 Government Data Practices and Intellectual Property; 15 Acknowledgment and Endorsement; 16 Governing Law, Jurisdiction, and Venue; 18 Data Disclosure; 23 Monitoring; and 26 Program Requirements.

1.4 Incur Expenses

Notwithstanding Minnesota Statutes, section 16A.41, expenditures made on or after July 1, 2023, or the date the Work Plan is approved, whichever is later, are eligible for reimbursement unless otherwise provided in [M.L. 2023, Chapter 60, Article 2](#).

2 Grantee's Duties

The Grantee, who is not a state employee, will:

- 2.1 See Attachment A, approved work plan, which is incorporated and made a part of this agreement. If applicable, the Grantee shall provide the State's Authorized Representative a copy of the revised work plan and the corresponding ENRTF approval letter within one week of any ENRTF approved changes to the work plan.
- 2.2 The Grantee will comply with required grants management policies and procedures set forth through Minn. Stat. §16B.97, subd 4 (a)(1), Minn. Stat. Ch.116P, and [M.L. 2023, Chapter 60, Article 2](#).
- 2.3 The Grantee agrees to complete the program in accordance with the approved budget to the extent practicable and within the program period specified in the grant contract agreement. Any material change in the grant contract agreement shall require an amendment by the State (see Section 8.2).
- 2.4 The Grantee shall be responsible for the administration, supervision, management, record keeping, and program oversight required for the work performed under this agreement.
- 2.5 The Grantee is responsible for maintaining an adequate conflict of interest policy. Throughout the term of this agreement, the Grantee shall monitor and report any actual, potential, or perceived conflicts of interest to the State's Authorized Representative.

3 Time

The Grantee must comply with all the time requirements described in this grant contract agreement.

4 Consideration and Payment

Consideration for all services performed by Grantee pursuant to this grant contract agreement shall be paid by the State as follows:

4.1 *Consideration.*

4.1.1 *Compensation*

Compensation in an amount not to exceed \$738,000 based on the following computation: See Attachment A for project budget.

4.1.2 *Matching requirements*

Grantee certifies that the following matching requirement for the grant will be met by GRANTEE. The total project cost is \$738,000. Grantee agrees to match at least \$0.00 of this project cost.

4.1.3 *Total Obligation*

The total obligation of the State for all compensation and reimbursements to the Grantee under this grant contract agreement will not exceed Seven Hundred Thirty Eight Thousand Dollars and No Cents dollars.

- 4.1.4 Funds made available pursuant to this Agreement shall be used only for expenses incurred in performing and accomplishing the purposes and activities specified herein. Notwithstanding all other provisions of this Agreement, it is understood that any reduction or termination of funds allocated to the State may result in a like reduction to the Grantee.

4.2 *Payment*

4.2.1 *Payment*

The State shall disburse funds to the Grantee pursuant to this agreement on a reimbursement basis. The Grantee shall submit payment requests with required expenditure documentation, as defined in the current Reimbursement Manual, to the State for review and approval. The Grantee shall submit payment requests on a regular basis (i.e. quarterly).

If necessary, advance payments on grants shall be negotiated between the State and Grantee on a case-by-case basis. In order to make advance payments, the Grantee must prepare and submit a written justification to the State for approval that details the specific need to utilize advance payments. A copy of the signed justification must be maintained in the grant file. All advance payments on grants over \$50,000 must be reconciled within 12 months of issuance or within 60 days of the end of the grant period.

4.2.2 *Retainage*

The final reimbursement will be paid out when the State determines that the Grantee has satisfactorily fulfilled all the terms of this agreement, unless otherwise excluded by the State in writing.

5 Use of Funds

- 5.1 The Grantee shall use the proceeds of this agreement only for the eligible direct expenditures of the program as described in the approved work plan.
- 5.2 The Grantee may provide portions of the proceeds of this agreement to the State. Work done by the State must be so specified in the approved work plan. A letter shall be submitted to the State's Authorized Representative and include: work to be accomplished; the specific area of the work plan authorizing the work; the portion of the proceeds to be used by the State; and the name, title, address, phone number and e-mail address for the State's staff member assigned to accomplish the work. The State will have the opportunity to review the letter and approve the work prior to accepting the funds. The Grantee's proceeds available under clause 4, Consideration and Payment, of this agreement shall be reduced by the amount provided for State use. In return, the State agrees to report back to the Grantee as to how appropriation funds were spent once the work is completed.

6 Conditions of Payment

All services provided by the Grantee under this grant contract agreement must be performed to the State's satisfaction, as determined at the sole discretion of the State's Authorized Representative and in accordance with all applicable federal, state, and local laws, ordinances, rules, and regulations. The Grantee will not receive payment for work found by the State to be unsatisfactory or performed in violation of federal, state, or local law. The Grantee will be bound to the most recent Reimbursement Manual, as provided by the State each state fiscal year.

7 Authorized Representative

The State's Authorized Representative is Katherine Sherman-Hoehn, Grants Manager, (651) 259-5533, katherine.sherman-hoehn@state.mn.us, or his/her successor, and has the responsibility to monitor the Grantee's performance and the authority to accept the services provided under this grant contract agreement. If the services are satisfactory, the State's Authorized Representative will certify acceptance on each invoice submitted for payment.

The Grantee's Authorized Representative is Brian Beck, 15320 Minnetonka Blvd, Minnetonka, MN, 55345, bbeck@minnehahacreek.org, or his/her successor. If the Grantee's Authorized Representative changes at any time during this grant contract agreement, the Grantee must immediately notify the State.

8 Assignment Amendments, Waiver, and Grant Contract Agreement Complete

8.1 Assignment

The Grantee shall neither assign nor transfer any rights or obligations under this grant contract agreement without the prior written consent of the State, approved by the same parties who executed and approved this grant contract agreement, or their successors in office.

8.2 Amendments

Any amendments to this grant contract agreement must be in writing and will not be effective until it has been executed and approved by the same parties who executed and approved the original grant contract agreement, or their successors in office.

8.3 Waiver

If the State fails to enforce any provision of this grant contract agreement, that failure does not waive the provision or the State's right to enforce it.

8.4 Grant Contract Agreement Complete

This grant contract agreement contains all negotiations and agreements between the State and the Grantee. No other understanding regarding this grant contract agreement, whether written or oral, may be used to bind either party.

9 Subcontractors, Contracting, and Bidding Requirements

- 9.1 The Grantee agrees that if it subcontracts any portion of this project to another entity, the agreement with the subcontractor will contain all applicable provisions of the agreement with the State. The Grantee will refer to the Subcontractors section in the current Reimbursement Manual, as provided by the State.
- 9.2 Per Minn. Stat. §471.345, grantees that are Municipalities as defined in Subd. 1 must follow the law.
- 9.2.1 The Grantee must maintain support documentation of the purchasing and/or bidding process utilized

to contract services in their financial records, including support documentation justifying a single/sole source bid, if applicable.

9.2.2 For projects that include construction work of \$25,000 or more, prevailing wage rules apply per [Minnesota Statue 177.41](#) through [177.44](#). Consequently, the bid request must state the project is subject to prevailing wage. These rules require that the wages of laborers and workers should be comparable to wages paid for similar work in the community as a whole. A prevailing wage form should accompany these bid submittals.

9.2.3 The grantee must not contract with vendors who are suspended or debarred in MN: [Link to Suspend/Debarred Vendor Report \(https://mn.gov/admin/osp/government/suspended-debarred/index2.jsp\)](#)

9.3 *Nongovernmental Organizations*

Must follow the below requirements or submit a copy of their organization's contracting policies via Attachment B for review and possible approval by the State's Authorized Representative. If the thresholds change during the life of the grant, the Grantee must follow the most current Reimbursement Manual guidelines.

9.3.1 Any services and/or materials that are expected to cost \$100,000 or more must undergo formal public notice and solicitation process.

9.3.2 Any services and/or materials that are expected to cost between \$25,000 and \$99,999 must be based on three (3) verbal quotes or bids.

9.3.3 Any services and/or materials that are expected to cost between \$10,000 and \$24,999 must be competitively based on a minimum of two (2) verbal quotes or bids or awarded to a targeted vendor.

9.3.4 The grantee must take all necessary affirmative steps to assure that targeted vendors from businesses with active certifications through these entities are used when possible:

- [State Department of Administration's Certified Targeted Group, Economically Disadvantaged and Veteran-Owned Vendor List.](#)
- Metropolitan Council's Targeted Vendor list: [Minnesota Unified Certification Program](#)
- Small Business Certification Program through Hennepin County, Ramsey County, and City of St. Paul: [Central Certification Program](#)

9.3.5 The Grantee must maintain written standards of conduct covering conflicts of interest and governing the actions of its employees engaged in the selection, award and administration of contracts. See Attachment C: Conflict of Interest Disclosure

9.3.6 The Grantee must maintain support documentation of the purchasing and/or bidding process utilized to contract services in their financial records, including support documentation justifying a single/sole source bid, if applicable.

9.3.7 Notwithstanding 1- 3 above, the State may waive bidding process requirements when it is determined there is only one legitimate or practical source for such materials or services and that grantee has established a fair and reasonable price.

9.3.8 For projects that include construction work of \$25,000 or more, prevailing wage rules apply per [Minn. Stat. §177.41](#) through [177.44](#). These rules require that the wages of laborers and workers should be comparable to wages paid for similar work in the community as a whole. A prevailing wage form should accompany these bid submittals.

9.3.9 The grantee must not contract with vendors who are suspended or debarred in MN: [Link to Suspend/Debarred Vendor Report \(https://mn.gov/admin/osp/government/suspended-debarred/index2.jsp\)](#)

10 **Liability**

The Grantee must indemnify, save, and hold the State, its agents, and employees harmless from any claims or causes of action, including attorney's fees incurred by the State, arising from the performance of this grant contract agreement by the Grantee or the Grantee's agents or employees. This clause will not be construed to bar any legal remedies the Grantee may have for the State's failure to fulfill its obligations under this grant contract agreement.

11 **State Audits**

Under [Minn. Stat. §16B.98, Subd.8](#), the Grantee's books, records, documents, and accounting procedures and practices of the Grantee or other party relevant to this grant contract agreement or transaction are subject to

examination by the State and/or the State Auditor or Legislative Auditor, as appropriate, for a minimum of six years from the end of this grant contract agreement, receipt and approval of all final reports, or the required period of time to satisfy all state and program retention requirements, whichever is later.

12 Government Data Practices and Intellectual Property Rights

12.1 *Government Data Practices*

The Grantee and State must comply with the Minnesota Government Data Practices Act, [Minn. Stat. Ch. 13](#), as it applies to all data provided by the State under this grant contract agreement, and as it applies to all data created, collected, received, stored, used, maintained, or disseminated by the Grantee under this grant contract agreement. The civil remedies of [Minn. Stat. §13.08](#) apply to the release of the data referred to in this clause by either the Grantee or the State. If the Grantee receives a request to release the data referred to in this Clause, the Grantee must immediately notify the State. The State will give the Grantee instructions concerning the release of the data to the requesting party before the data is released. The Grantee's response to the request shall comply with applicable law.

12.2 As cited in M.L. 2023, Chp. 60, Art. 2, Sec. 2, Subd. 04l Minnehaha Creek Watershed District may license third parties to use products developed with this appropriation without further approval from the legislature or the Legislative-Citizen Commission on Minnesota Resources, provided the licensing does not generate income. This appropriation is subject to Minnesota Statutes, section 116P.10.

12.3 *Intellectual Property Rights (if applicable)*

The Grantee will comply with Minnesota Statutes, Chapter 116P.10.

13 Criminal Background Checks

A recipient of an appropriation that is receiving funding to conduct children's services, as defined in Minnesota Statutes, section 299C.61, subdivision 7, must certify to the Legislative-Citizen Commission on Minnesota Resources, as part of the required work plan, that it performs criminal background checks for background check crimes, as defined in Minnesota Statutes, section 299C.61, subdivision 2, on all employees, contractors, and volunteers that have or may have access to a child to whom the recipient provides children's services using the appropriation.

14 Workers Compensation

The Grantee certifies that it is in compliance with [Minn. Stat. §176.181](#), Subd. 2, pertaining to workers' compensation insurance coverage. The Grantee's employees and agents will not be considered State employees. Any claims that may arise under the Minnesota Workers' Compensation Act on behalf of these employees and any claims made by any third party as a consequence of any act or omission on the part of these employees are in no way the State's obligation or responsibility.

15 Acknowledgment and Endorsement

15.1 *Acknowledgment*

The Grantee must acknowledge financial support from the Minnesota Environment and Natural Resources Trust Fund in project publications, signage and other public communication and outreach related to work completed using the appropriation. Acknowledgment may occur, as appropriate, through use of the fund logo or inclusion of language attributing support from the trust fund.

15.2 *Endorsement*

The Grantee must not claim that the State endorses its products or services.

16 Governing Law, Jurisdiction, and Venue

Minnesota law, without regard to its choice-of-law provisions, governs this grant contract agreement. Venue for all legal proceedings out of this grant contract agreement, or its breach, must be in the appropriate state or federal court with competent jurisdiction in Ramsey County, Minnesota.

17 Termination

17.1 *Termination by the State*

The State may immediately terminate this grant contract agreement with or without cause, upon 30 days' written notice to the Grantee. Upon termination, the Grantee will be entitled to payment, determined on a pro rata basis, for services satisfactorily performed.

17.2 Termination by the Commissioner of Administration

The Commissioner of Administration may unilaterally cancel this grant contract agreement if further performance under the agreement would not serve agency purposes or is not in the best interest of the State.

18 Data Disclosure

Under [Minn. Stat. § 270C.65](#), Subd. 3, and other applicable law, the Grantee consents to disclosure of its social security number, federal employer tax identification number, and/or Minnesota tax identification number, already provided to the State, to federal and state tax agencies and state personnel involved in the payment of state obligations. These identification numbers may be used in the enforcement of federal and state tax laws which could result in action requiring the Grantee to file state tax returns and pay delinquent state tax liabilities, if any.

19 Use of Funds as Match to Other Grants or Programs

The Grantee must inform the State's Authorized Representative whenever the grant funds will be used as match or for reimbursement for any other grant or program.

- a. The Grantee must inform the State's Authorized Representative or their grant specialist of the following information: grant program, grant name, the amount of grant or match funds to be used, location where funds were or will be used, activity the funds will support, and current landowner (if applicable).
- b. The Grantee must also inform the State's Authorized Representative before work begins if the new grant or program will add any encumbrances to state land where grant or match funds will be spent.

20 American Disabilities Act

The Grantee must comply with the 2010 American Disabilities Act Standards for Accessible Design.

21 Non-Discrimination Requirements

No person in the United States must, on the ground of race, color, national origin, handicap, age, religion, or sex, be excluded from participation in, be denied the benefits of, or be subject to discrimination under, any program or activity receiving Federal financial assistance. Including but not limited to:

- a. Title VI of the Civil Rights Act of 1964 (42 U.S.C. § 2000d et seq.) and DOC implementing regulations published at 15 C.F.R. Part 8 prohibiting discrimination on the grounds of race, color, or national origin under programs or activities receiving Federal financial assistance; Title IX of the Education Amendments of 1972 (20 U.S.C. § 1681 et seq.) prohibiting discrimination on the basis of sex under Federally assisted education programs or activities;
- b. Section 504 of the Rehabilitation Act of 1973, as amended (29 U.S.C. § 794), and DOC implementing regulations published at 15 C.F.R. Part 8b prohibiting discrimination on the basis of handicap under any program or activity receiving or benefiting from Federal assistance.
- c. The Age Discrimination Act of 1975, as amended (42 U.S.C. § 6101 et seq.), and DOC implementing regulations published at 15 C.F.R. Part 20 prohibiting discrimination on the basis of age in programs or activities receiving Federal financial assistance;
- d. Title II of the Americans with Disabilities Act (ADA) of 1990 which prohibits discrimination against qualified individuals with disabilities in services, programs, and activities of public entities.
- e. Any other applicable non-discrimination law(s).

22 Reporting Requirements

The Grantee is bound to reporting requirements in [Minn. Stat. §116P](#), [M.L. 2023, Chapter 60, Article 2](#), Attachment A, as well as Attachments D and E (if applicable).

23 Monitoring

The State shall be allowed at any time to conduct periodic site visits and inspections to ensure work progress in accordance with this grant contract agreement, including a final inspection upon program completion. At least one monitoring visit per grant period on all state grants of over \$50,000 will be conducted and at least annual monitoring visits on grants of over \$250,000.

Following closure of the program, the State's authorized representatives shall be allowed to conduct post-completion inspections of the site to ensure that the site is being properly operated and maintained and that no conversion of use has occurred.

24 Invasive Species Prevention

Grantees must follow Minnesota DNR's Operational Order 113, which requires preventing or limiting the introduction, establishment and spread of invasive species during activities on public waters and DNR-administered lands. This applies to all activities performed on all lands under this grant contract agreement and is not limited to lands under DNR control or public waters. Duties are listed under Sections II and III (p. 5-8) of Operational Order 113 which may be found here: [Link to Operational Order 113](http://files.dnr.state.mn.us/assistance/grants/habitat/heritage/oporder_113.pdf) (http://files.dnr.state.mn.us/assistance/grants/habitat/heritage/oporder_113.pdf)

25 Pollinator Best Management Practices

Habitat restorations and enhancements conducted on DNR lands and prairie restorations on state lands or on any lands using state funds are subject to pollinator best management practices and habitat restoration guidelines pursuant to [Minnesota Statutes, section 84.973](#). Practices and guidelines ensure an appropriate diversity of native species to provide habitat for pollinators through the growing season. Current specific practices and guidelines to be followed for contract and grant work can be found here: [Link to Specific Pollinator Best Management Practices for DNR Grants and Contracts](#) (http://files.dnr.state.mn.us/natural_resources/npc/bmp_contract_language.pdf).

26 Program Requirements

The grantee must comply with the most current versions of Attachments A, B, C, D, and E as attached and incorporated into this grant contract agreement.

Attachments

- A. Current Work Plan with ENRTF Approval Letter
- B. Non-governmental Organization Subcontracting (if applicable)
- C. Conflict of Interest Disclosure
- D. Reimbursement Manual
- E. Land Acquisition Reporting Procedures (if applicable)

1. STATE ENCUMBRANCE VERIFICATION

Individual certifies that funds have been encumbered as required by Minn. Stat.

16A.15.

DocuSigned by:

Debra Lohmeyer

8235A2805B1B40F...

Signed: _____

Date: November 9, 2023

SWIFT Contract/PO No(s). 239227/3000243237

2. GRANTEE

The Grantee certifies that the appropriate person(s) have executed the grant contract on behalf of the Grantee as required by applicable articles, bylaws, resolutions, or ordinances.

DocuSigned by:

James Wisker

2EE16732846C433...

By: _____

Title: Administrator

Date: November 15, 2023

By: _____

Title: _____

Date: _____

3. STATE AGENCY

DocuSigned by:

Mary Robison

24FD40E480A84C0...

By: _____

(with delegated authority)

Title: Chief Financial Officer

Date: November 16, 2023

DS


DS
BB

Distribution:

Agency

Grantee

State's Authorized Representative



LEGISLATIVE-CITIZEN COMMISSION ON MINNESOTA RESOURCES
100 Rev. Dr. Martin Luther King Jr. Blvd., Room 65
St. Paul, MN 55155-1201

Phone: (651) 296-2406

Email: lccmr@lccmr.mn.gov Web: www.lccmr.mn.gov

June 28, 2023

Dear 2023 ENRTF project managers,

I am happy to report that at the LCCMR's June 22, 2023 and June 28, 2023 meeting, the commission approved the work plans listed on the attached spreadsheet. The work plan, together with the conditions provided in the bill language, statute, your forthcoming grant agreement, and other policies and guidelines of ENRTF funding, will guide the use of funds for your project. We will be sending a note to your agency heads momentarily to notify them that you now have authority to begin spending these funds starting July 1. We have also changed the status of your workplan in the online system to "Final Work Plan Approved" and will be posting an approved version of your work plan on the [M.L. 2023 appropriations page](#) of LCCMR's website. Please see below for additional information.

[Authority to Spend; Conditions](#)

As a non-state entity, your appropriation will be jointly managed throughout the life of your project by the LCCMR and the Department of Natural Resources (DNR) Grants Unit.

You may begin spending on your project starting July 1, however you must enter into a grant agreement contract with the DNR before any reimbursement for these costs can occur. We encourage caution in your spending until you are familiar with these contract terms, including state bidding and prevailing wage requirements, to ensure they will be eligible for reimbursement. You will be contacted by the DNR Grants Unit within the next month with financial compliance instructions, [reimbursement request instructions](#), and a draft of the required grant agreement contract for "pass-through grants."

[LCCMR & DNR Grants Unit: Joint Oversight](#)

The LCCMR's role throughout your project will primarily involve overseeing your work plan and budget, which will be done through review of your progress/spending reports submitted in the online system every 6 months according to the schedule shown on the Tab 4 (Narrative) of your workplan. For most of you, your first update won't be due until April 1. DNR's role will primarily involve reimbursement payment to you and financial compliance. The DNR cannot reimburse you without LCCMR approval of your status updates and your submission of a reimbursement request(s) to the DNR.

[Questions?](#)

If you have questions about your workplan or budget, please feel free to contact your LCCMR lead staff, who is listed at the top of Tab 10 (Review and Submit Page) of your work plan.

If you have questions about your grant agreement contract with DNR, contact Katherine Sherman-Hoehn, Grants Manager at katherine.sherman-hoehn@state.mn.us or visit the [DNR Pass-Through Grants website](#).

Congratulations on approval of your funding, and best of luck to you all as you get your projects launched! We look forward to seeing them come to fruition.

Becca

Becca Nash
Director
Legislative-Citizen Commission on Minnesota Resources
65 State Office Building
100 Rev. Dr. Martin Luther King Jr. Blvd.
St. Paul, Minnesota 55155
Phone: 651 296-6264
Email: Becca.Nash@lccmr.mn.gov

Environment and Natural Resources Trust Fund
Final Work Plan Review and Approvals - M.L. 2023 (FY 2024) as of June 28, 2023 - Pass Through

| Line # | Direct or Pass Through | Organization | First Name | Last Name | Subd. | Proposal ID | Title | Funding Amount Appropriated | Work Plan Approval Notes and Contingencies | Project Impact Area | Classified Staff | Fee Title Acquisition | Conservation Easement | Capital Expenditures | Peer Review Complete | Potential Revenue | Work Plan Approval Date |
|--------|------------------------|---|------------|---------------|-------|-------------|--|-----------------------------|---|---------------------|------------------|-----------------------|-----------------------|----------------------|----------------------|-------------------|-------------------------|
| 1 | Pass Through | Pheasants Forever Inc | Josh | Pommier | 08a | 2023-006 | Minnesota Bee and Beneficial Species Habitat Enhancement II | \$876,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |
| 2 | Pass Through | Friends of the Boundary Waters Wilderness | Alison | Nyenhuis | 05a | 2023-008 | Fostering Conservation by Connecting Students to the BWCA | \$1,080,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 3 | Pass Through | Three Rivers Park District | John | Moriarty | 08b | 2023-010 | Karner Blue Butterfly Insurance Population Establishment in Minnesota | \$405,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |
| 4 | Pass Through | ServeMinnesota | Sharon | Delcambre | 07a | 2023-013 | Community Forestry AmeriCorps | \$1,500,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 5 | Pass Through | Red River Basin Flood Damage Reduction Work Group | Andrew | Graham | 04b | 2023-022 | Assessment of Red River Basin Project Outcomes | \$920,000 | | NW | No | No | No | No | n/a | No | 6/22/2023 |
| 6 | Pass Through | Eagle Bluff Environmental Learning Center | Colleen | Foehrenbacher | 08c | 2023-025 | Root River Habitat Restoration at Eagle Bluff | \$866,000 | | Root River | No | No | No | No | n/a | No | 6/22/2023 |
| 7 | Pass Through | City of Frazee | Stephanie | Poegel | 09b | 2023-032 | Wannigan Regional Park Land Acquisition | \$727,000 | | NW | No | Yes | No | No | n/a | No | 6/22/2023 |
| 8 | Pass Through | Great River Greening | Todd | Rexine | 07b | 2023-043 | Biochar Implementation in Habitat Restoration: A Pilot | \$185,000 | | Central, Metro, SE | No | No | No | No | n/a | No | 6/22/2023 |
| 9 | Pass Through | Friends of the Mississippi River | Alex | Roth | 03a | 2023-044 | Assessing Restorations for Rusty-Patched and Other Bumblebee Habitat | \$75,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |
| 10 | Pass Through | Pioneer PBS | Cindy | Dorn | 05b | 2023-051 | Statewide Environmental Education via PBS Outdoor Series | \$391,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 11 | Pass Through | Audubon Minnesota | Dale | Gentry | 03c | 2023-072 | Mapping Migratory Bird Pitstops in Minnesota | \$340,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |
| 12 | Pass Through | City of Wayzata | Nick | Kieser | 08f | 2023-080 | Panoway on Wayzata Bay Shoreline Restoration Project | \$200,000 | The recipient must report to the Legislative-Citizen Commission on Minnesota Resources on the effectiveness of any new methods tested while conducting the project and may use a portion of the appropriation to prepare that report. | Metro | No | No | No | No | Yes | No | 6/22/2023 |
| 13 | Pass Through | City of St. Joseph | Nate | Keller | 09f | 2023-091 | Construction of East Park | \$700,000 | | Central | No | No | No | No | n/a | No | 6/22/2023 |
| 14 | Pass Through | Great River Greening | Rebecca | Tucker | 08g | 2023-105 | Pollinator Central III: Habitat Improvement with Community Monitoring | \$190,000 | | Metro | No | No | No | No | n/a | No | 6/22/2023 |
| 15 | Pass Through | University of St. Thomas | Kristine | Wammer | 04f | 2023-107 | Ecotoxicological Impacts of Quinone Outside Inhibitor (QOI) Fungicides | \$279,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |

Environment and Natural Resources Trust Fund
Final Work Plan Review and Approvals - M.L. 2023 (FY 2024) as of June 28, 2023 - Pass Through

| Line # | Direct or Pass Through | Organization | First Name | Last Name | Subd. | Proposal ID | Title | Funding Amount Appropriated | Work Plan Approval Notes and Contingencies | Project Impact Area | Classified Staff | Fee Title Acquisition | Conservation Easement | Capital Expenditures | Peer Review Complete | Potential Revenue | Work Plan Approval Date |
|--------|------------------------|--|------------|------------|-------|-------------|--|-----------------------------|--|---------------------|------------------|-----------------------|-----------------------|----------------------|----------------------|-------------------|-------------------------|
| 16 | Pass Through | City of Scandia | Kyle | Morell | 09g | 2023-110 | Scandia Gateway Trail to William O'Brien State Park | \$2,689,000 | This project must be designed and constructed in accordance with Department of Natural Resources state trail standards. Engineering and construction plans must be approved by the commissioner of natural resources before construction may commence. | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 17 | Pass Through | Great River Greening | Brad | Gordon | 08h | 2023-117 | Restoring Forests and Savannas Using Silvopasture - Phase II | \$674,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 18 | Pass Through | Fillmore County Soil and Water Conservation District | Riley | Buley | 04g | 2023-129 | Brightsdale Dam Channel Restoration | \$1,004,000 | | Root River | No | No | No | No | n/a | No | 6/22/2023 |
| 19 | Pass Through | The Trust for Public Land | Eric | Weiss | 08i | 2023-135 | Minnesota Community Schoolyards | \$1,433,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 20 | Pass Through | Alexandria Lake Area Sanitary District (ALASD) | Scott | Gilbertson | 04i | 2023-137 | ALASD's Chloride Source Reduction Pilot Program | \$764,000 | | Central | No | No | No | No | n/a | No | 6/22/2023 |
| 21 | Pass Through | Pheasants Forever Inc | Tanner | Bruse | 08k | 2023-142 | Conservation Cooperative for Working Lands | \$2,611,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 22 | Pass Through | Superior Cycling Association | Paul | Nordlund | 09h | 2023-147 | Grand Marais Mountain Bike Trail Rehabilitation-Phase II | \$200,000 | | NE | No | No | No | No | n/a | No | 6/22/2023 |
| 23 | Pass Through | City of Duluth | Cliff | Knettel | 09j | 2023-172 | St. Louis River Re-Connect - Phase II | \$1,375,000 | | NE | No | No | No | Yes | n/a | No | 6/28/2023 |
| 24 | Pass Through | Superior Hiking Trail Association | Lisa | Luukkala | 08m | 2023-181 | Renewing Access to an Iconic North Shore Vista | \$197,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 25 | Pass Through | Superior Hiking Trail Association | Lisa | Luukkala | 08n | 2023-189 | Addressing Erosion Along High Use River Loops | \$368,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 26 | Pass Through | Sugarloaf The North Shore Stewardship Association | Molly | Thompson | 05f | 2023-201 | North Shore Private Forestry Outreach and Implementation | \$375,000 | | NE | No | No | No | No | n/a | No | 6/22/2023 |
| 27 | Pass Through | City of Biwabik | Jeff | Jacobson | 09k | 2023-207 | City of Biwabik Recreation | \$1,306,000 | | NE | No | No | No | No | n/a | No | 6/22/2023 |
| 28 | Pass Through | City of Silver Bay | Lana | Fralich | 09l | 2023-210 | Silver Bay Multimodal Trailhead Project | \$1,970,000 | Before any construction costs are incurred, the city must demonstrate that all funding to complete the project are secured. | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 29 | Pass Through | Minneapolis Park and Recreation Board | Adam | Arvidson | 08p | 2023-212 | Enhancing Habitat Connectivity within the Urban Mississippi Flyway | \$190,000 | | Metro | No | No | No | Yes | n/a | No | 6/22/2023 |

Environment and Natural Resources Trust Fund
Final Work Plan Review and Approvals - M.L. 2023 (FY 2024) as of June 28, 2023 - Pass Through

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|--------|------------------------|--|------------|------------|-------|-------------|---|-----------------------------|---|---------------------|------------------|-----------------------|-----------------------|----------------------|----------------------|-------------------|-------------------------|
| 30 | Pass Through | Minneapolis Park and Recreation Board | Adam | Arvidson | 09m | 2023-213 | Above the Falls Park Restoration Planning and Acquisition | \$1,376,000 | This appropriation may not be used to purchase habitable residential structures. Before the acquisition, a phase 1 environmental assessment must be completed and the Minneapolis Park and Recreation Board must not accept any liability for previous contamination of lands acquired with this appropriation. | Metro | No | Yes | No | No | n/a | No | 6/22/2023 |
| 31 | Pass Through | Hawk Ridge Bird Observatory | Janelle | Long | 03o | 2023-217 | Linking Breeding and Migratory Bird Populations in Minnesota | \$199,000 | | Statewide | No | No | No | No | Yes | No | 6/22/2023 |
| 32 | Pass Through | Minnesota Trout Unlimited | John | Lenczewski | 05g | 2023-223 | Teaching Students about Watersheds through Outdoor Science | \$290,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 33 | Pass Through | Minneapolis Park and Recreation Board | MaryLynn | Pulscher | 05h | 2023-229 | Bioblitz Urban Parks: Engaging Communities in Scientific Efforts | \$198,000 | | Minneapolis | No | No | No | No | n/a | No | 6/22/2023 |
| 34 | Pass Through | Minnesota Discovery Center | Donna | Johnson | 09n | 2023-231 | Redhead Mountain Bike Park | \$1,666,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 35 | Pass Through | The Nature Conservancy | David | Ruff | 03r | 2023-232 | Community Response Monitoring for Adaptive Management | \$483,000 | | Statewide | No | No | No | No | n/a | No | 6/22/2023 |
| 36 | Pass Through | Minnehaha Creek Watershed District | Brian | Beck | 04l | 2023-238 | Leveraging Data Analytics Innovations for Watershed District Planning | \$738,000 | Minnehaha Creek Watershed District may license third parties to use products developed with this appropriation without further approval from the legislature or the Legislative-Citizen Commission on Minnesota Resources, provided the licensing does not generate income. | Statewide | No | No | No | No | n/a | Yes | 6/22/2023 |
| 37 | Pass Through | White Earth Band of Minnesota Chippewa Indians | Jamie | Konopacky | 04m | 2023-247 | Protecting Minnesota's Headwaters of the Mississippi/Pineland Sands | \$1,693,000 | ENRTF funds will be used for educational purposes but not for any lobbying for recommended policy changes. | Central | No | No | No | No | Yes | No | 6/22/2023 |

Environment and Natural Resources Trust Fund
 Final Work Plan Review and Approvals - M.L. 2023 (FY 2024) as of June 28, 2023 - Pass Through

| Line # | Direct or Pass Through | Organization | First Name | Last Name | Subd. | Proposal ID | Title | Funding Amount Appropriated | Work Plan Approval Notes and Contingencies | Project Impact Area | Classified Staff | Fee Title Acquisition | Conservation Easement | Capital Expenditures | Peer Review Complete | Potential Revenue | Work Plan Approval Date |
|--------|------------------------|-------------------|------------|-----------|-------|-------------|---|-----------------------------|--|---------------------|------------------|-----------------------|-----------------------|----------------------|----------------------|-------------------|-------------------------|
| 38 | Pass Through | Otter Tail County | Kevin | Fellbaum | 09o | 2023-249 | Maplewood State Park Trail Segment | \$2,514,000 | This project must be designed and constructed in accordance with Department of Natural Resources state trail standards. Engineering and construction plans must be approved by the commissioner of natural resources before construction may commence. | Central | No | No | No | No | n/a | No | 6/22/2023 |
| 39 | Pass Through | Otter Tail County | Kevin | Fellbaum | 08r | 2023-250 | Phelps Mill Wetland and Prairie Restoration | \$974,000 | Up to \$322,000 of this appropriation may be used to plan, engineer, and construct a boardwalk, viewing platforms, and soft trails within the park. | Central | No | No | No | No | n/a | No | 6/22/2023 |



Environment and Natural Resources Trust Fund

M.L. 2023 Approved Work Plan

General Information

ID Number: 2023-238

Staff Lead: Michael Varien

Date this document submitted to LCCMR: June 15, 2023

Project Title: Leveraging Innovations in Data Analytics for Project Implementation

Project Budget: \$738,000

Project Manager Information

Name: Brian Beck

Organization: Minnehaha Creek Watershed District

Office Telephone: (952) 471-8306

Email: bbeck@minnehahacreek.org

Web Address: <https://www.minnehahacreek.org/>

Project Reporting

Date Work Plan Approved by LCCMR: June 22, 2023

Reporting Schedule: April 1 / October 1 of each year.

Project Completion: July 31, 2025

Final Report Due Date: September 14, 2025

Legal Information

Legal Citation: M.L. 2023, Chp. 60, Art. 2, Sec. 2, Subd. 04I

Appropriation Language: \$738,000 the first year is from the trust fund to the commissioner of natural resources for an agreement with Minnehaha Creek Watershed District to integrate local and statewide data sets into a high-resolution planning tool that forecasts the impacts of changing precipitation patterns and quantitatively compares cost effectiveness and outcomes for water quality, ecological integrity, and flood prevention projects in the district. Minnehaha Creek Watershed District may license third parties to use products developed with this appropriation without further approval from the legislature or the Legislative-Citizen Commission on Minnesota Resources, provided the licensing does not generate income. This appropriation is subject to Minnesota Statutes, section 116P.10.

Appropriation End Date: June 30, 2026

Narrative

Project Summary: Integrating local and statewide datasets into a 21st-century planning tool, widely called for by our communities, that forecasts the impacts of changing precipitation patterns and quantitatively compares cost-effective solutions.

Describe the opportunity or problem your proposal seeks to address. Include any relevant background information.

Water systems throughout Minnesota were built for stable climate patterns that no longer exist. Extreme swings in precipitation are stressing our natural and built environments, impacting pollutant loading, stream erosion, wetland function, surface and groundwater interactions, habitat, and the safety of homes, public infrastructure, and businesses.

Watershed managers must help communities understand and adapt to these changes. However, the ability to do so is hampered by sparse and static historic data sets, which make it difficult to predict how specific areas will be impacted and quantitatively compare potential solutions.

Fortunately, advances in data science have made it affordable to collect exponentially more data and analyze it in more sophisticated ways. These advances allow water planners around the world to understand and predict changes with unprecedented accuracy and detail, allowing for more effective use of scarce public investment to address these issues. In Minnesota, data collection has outpaced the tools used to make sense of the information. Realizing the full potential of these advances requires new systems to integrate this data to identify existing issues, forecast future ones, and guide local decisions.

What is your proposed solution to the problem or opportunity discussed above? Introduce us to the work you are seeking funding to do. You will be asked to expand on this proposed solution in Activities & Milestones.

In partnership with the DNR, USGS, and Hennepin County, and with formal support from 14 federal, state, and regional agencies and local communities, the Minnehaha Creek Watershed District (MCWD) is proposing a pioneering program to maximize the value of recent public investments in data collection.

For example, MCWD has created a remote sensing network that collects more than 1 million data points per year about surface water levels, shallow groundwater levels, and pollutant loading. State leaders have invested in mapping the detailed topography of the state. Municipal partners have digitized data about their storm sewer systems.

MCWD will use funding from LCCMR to develop a reproducible process that brings these disparate data sets together into a quantitative planning tool. Using advances in 2-dimensional modeling, these tools will be able to pinpoint, quantitatively evaluate and drive decisions on climate adaptation projects and policies.

Such a tool will be critical to the climate adaptation planning efforts as watershed managers and communities begin to understand the impact of changing precipitation patterns on our built and natural systems. The result will be more effective green and gray infrastructure solutions that protect and conserve the watershed's iconic water resources.

What are the specific project outcomes as they relate to the public purpose of protection, conservation, preservation, and enhancement of the state's natural resources?

- A single, continuously-updated tool that integrates previously-siloed public data sets to quantitatively compare proposed natural resource projects
- A high-resolution understanding of the balance of all surface and groundwater inputs and outputs in the system, to identify natural resources and public assets in need of protection
- Improved ability to predict the impact of changes in precipitation and land use, to enhance infrastructure

planning

- Improved ability to quantify and compare the cost-effectiveness of potential conservation projects needed to address predicted impacts

Project Location

What is the best scale for describing where your work will take place?

Watershed(s): Mississippi River - Twin Cities

What is the best scale to describe the area impacted by your work?

Statewide

When will the work impact occur?

During the Project and In the Future

Activities and Milestones

Activity 1: Building the External Data Information Processing System

Activity Budget: \$361,000

Activity Description:

Because land use and stormwater infrastructure are constantly changing, watershed managers face the recurring challenge of using tools that are not based on up-to-date information. Historically, the process of updating watershed models has been a time-intensive endeavor because all data collection and processing has been done manually.

However, recent advances in data science have resulted in frameworks that automate complex data processing, which will dramatically reduce the cost of future model updates for MCWD and other public agencies throughout the state that could use this process as a template for enhancing and automating their own watershed model development.

MCWD will develop a reproducible data processing system that can incorporate publicly available datasets into a watershed modeling framework that can be used as a template for other local or regional agencies to efficiently build water resource models. Then, MCWD will work with technical experts to plan and build a GIS system that automatically updates based on changing landuse and infrastructure datasets to ensure the watershed model used for natural resource project identification is using the most current landscape and infrastructure information. The goal of this activity is to build a template that makes modeling much more efficient for all public agencies.

Activity Milestones:

| Description | Approximate Completion Date |
|---|-----------------------------|
| Develop Request for Proposals | April 30, 2023 |
| Request Authorization to Release Project Request for Proposals | July 31, 2023 |
| Request MCWD Board of Manager Authorization to Select Vendor for Model Input Refinement System | September 30, 2023 |
| Meet with Municipalities and Partner Agencies to Provide Project Overview and Coordinate Stormwater Infrastructure Data | October 31, 2023 |
| Develop Automated Process to Convert Regional Datasets into Standardized Geodatabase | November 30, 2023 |
| Collect Initial Wetland, Stream Channel, and Bridge Data | November 30, 2023 |
| Develop Automated Process that Converts the Standardized Geodatabase into Model Ready Format | January 31, 2024 |
| Develop Documentation for Automated Geospatial Processing Steps | April 30, 2024 |

Activity 2: Building the 2D Watershed Model for Natural Resource Climate Adaptation Planning

Activity Budget: \$377,000

Activity Description:

MCWD will incorporate the data produced from the automated processing system developed in activity 1 into a high-resolution watershed model that can predict, in unprecedented detail, how water and pollutants will move through the system under current and predicted scenarios. The outcome from building the watershed model will be a tool that can help watershed managers meet their water quality, water quantity, and ecologic improvement goals.

Building this model will involve an iterative process to ensure that the automated processes developed in activity 1 can be incorporated into a high-resolution watershed planning tool. In addition, the consultant will use streamflow data collected by MCWD staff to calibrate the model to ensure it can accurately predict how water moves through the built and natural environment.

MCWD and the consultant will meet with local municipalities and engineers to communicate the use cases for the model to ensure it can be used by other entities to identify water quality, natural resource, and flood reduction projects.

Activity Milestones:

| Description | Approximate Completion Date |
|--|-----------------------------|
| Request Authorization to Release Project Request for Proposals | September 30, 2023 |
| Request MCWD Board of Manager Authorization to Select Vendor to Build Watershed Model | December 31, 2023 |
| Load Model Ready Datasets from Automated Processes into Watershed Model to Test Completeness of Datasets | February 28, 2024 |
| Refine Automated Processes to Incorporate Fixes and Issues Based on Model Testing | May 31, 2024 |
| Load Updated Datasets Based on Refined Automated Process into Model | July 31, 2024 |
| Collect Missing Field Data Identified During Model Testing | August 31, 2024 |
| Calibrate and Validate 2D Watershed Model Upstream of Grays Bay Dam and Create Documentation | December 31, 2024 |
| Calibrate and Validate 2D Watershed Model Upstream of Grays Bay Dam and Create Documentation | February 28, 2025 |
| Develop Final Documentation of Model Based on Calibrated Model and Automated Process | May 31, 2025 |

Project Partners and Collaborators

| Name | Organization | Role | Receiving Funds |
|-------------|---|--|-----------------|
| Tim Cowdery | U.S. Geological Survey | Assist with identifying groundwater well monitoring locations, conducting groundwater data analysis, suggesting methods for incorporating groundwater data into the 2D model, and providing oversight on 2D model build. | No |
| Dan Lais | Minnesota Department of Natural Resources | Collect and analyze additional groundwater and surface water interactions to integrate this data, along with other datasets, into the development of a high-resolution two-dimensional (2D) watershed model. | No |
| John Evans | Hennepin County | Hennepin County will assist with data collection, assessing climate impacts, and providing input on the tools needed to effectively plan and adapt to changing hydrology, in partnership with our communities. | No |

Dissemination

Describe your plans for dissemination, presentation, documentation, or sharing of data, results, samples, physical collections, and other products and how they will follow ENRTF Acknowledgement Requirements and Guidelines.

There are multiple ways that the Minnehaha Creek Watershed District (District or MCWD) will disseminate the results from this project. Those dissemination efforts include:

- 1) Frequent communication with cities, townships, counties, regional agencies, and state agencies within MCWD to ensure that the model scenarios inform how each entity can improve its ability to manage water resources at a system scale.
- 2) Publishing a technical report on the Minnehaha Creek Watershed District website.
- 3) The MCWD staff will present at conferences about the model development process and the outputs from the model scenario analysis to increase awareness of the project.
- 5) The MCWD will provide acknowledgment of ENRTF, and include the ENRTF logo on reports and data submitted to the public and partner agencies.

Long-Term Implementation and Funding

Describe how the results will be implemented and how any ongoing effort will be funded. If not already addressed as part of the project, how will findings, results, and products developed be implemented after project completion? If additional work is needed, how will this work be funded?

This project will yield a sophisticated tool, drawing on state of the art data analytics, to enhance MCWD's organizational ability to partner with its member communities to identify, evaluate, and implement natural resource capital improvement projects that improve water quality, control water quantity, improve ecological integrity, and reduce flooding in the face of a changing climate. The products of LCCMR's investment are expected to help MCWD and its partners populate, focus, and prioritize capital improvement plans that will be funded locally. The long-term sustainment costs for maintaining the watershed tools will be borne by MCWD.

Budget Summary

| Category / Name | Subcategory or Type | Description | Purpose | Gen. Ineligible | % Benefits | # FTE | Classified Staff? | \$ Amount |
|---------------------------------------|--|--|---------|-----------------|------------|-------|-------------------|------------------|
| Personnel | | | | | | | | |
| | | | | | | | Sub Total | - |
| Contracts and Services | | | | | | | | |
| TBD through competitive bid | Professional or Technical Service Contract | The consultant will be responsible for developing computer programming that will convert municipal and regional agency geospatial data into a standardized format and model development, and report writing. They will also be responsible for collecting stream channel data, wetland data, and bridge data in cooperation with MCWD. | | | | 2.44 | | \$738,000 |
| | | | | | | | Sub Total | \$738,000 |
| Equipment, Tools, and Supplies | | | | | | | | |
| | | | | | | | Sub Total | - |
| Capital Expenditures | | | | | | | | |
| | | | | | | | Sub Total | - |
| Acquisitions and Stewardship | | | | | | | | |
| | | | | | | | Sub Total | - |
| Travel In Minnesota | | | | | | | | |
| | | | | | | | Sub Total | - |
| Travel Outside Minnesota | | | | | | | | |
| | | | | | | | Sub Total | - |

| | | | | | | | | |
|---------------------------------|--|--|--|--|--|--|--------------------|------------------|
| Printing and Publication | | | | | | | | |
| | | | | | | | Sub Total | - |
| Other Expenses | | | | | | | | |
| | | | | | | | Sub Total | - |
| | | | | | | | Grand Total | \$738,000 |

Classified Staff or Generally Ineligible Expenses

| Category/Name | Subcategory or Type | Description | Justification Ineligible Expense or Classified Staff Request |
|---------------|---------------------|-------------|--|
|---------------|---------------------|-------------|--|

Non ENRTF Funds

| Category | Specific Source | Use | Status | \$ Amount |
|------------------|---|--|----------------------------|------------------|
| State | | | | |
| | | | State Sub Total | - |
| Non-State | | | | |
| In-Kind | Minnehaha Creek Watershed District Tax Levy | Minnehaha Creek Watershed District staff will be contributing a total of 3790 hours of in-kind support for the project. This work will include coordinating with cities to obtain data, communicating model need and purpose with cities, collecting field data, installing groundwater and surface water well sensors, reviewing automated computer code, and reviewing model develop. MCWD will also develop portions of the automated data intake code for automatically pulling weather data from the National Weather Service to calibrate the watershed model. | Secured | \$203,100 |
| Cash | Minnehaha Creek Watershed District Tax Levy | MCWD will contract with the USGS to oversee the selection of groundwater sensor monitoring locations and the development of the 2D watershed model | Secured | \$10,000 |
| | | | Non State Sub Total | \$213,100 |
| | | | Funds Total | \$213,100 |

Attachments

Required Attachments

Visual Component

File: [61f1f181-ad1.pdf](#)

Alternate Text for Visual Component

The attached graphic demonstrates how the proposed tools will convert a variety of disparate data sources into usable information to inform natural resource management decisions. It demonstrates how data sources about our built and natural environment – soils, topography, wetlands, hydrology, groundwater, precipitation, land cover, future land use, and storm sewer – will be integrated into a 2-dimensional model that will predict how water moves through the landscape under a variety of scenari...

Board Resolution or Letter

| Title | File |
|---|----------------------------------|
| MCWD Board Resolution Placeholder for LCCMR Application | 14569b43-45c.pdf |

Optional Attachments

Support Letter, Photos, Media, Other

| Title | File |
|---|----------------------------------|
| USGS Letter of Support | de56a77b-280.pdf |
| MN DNR Letter of Support | 2f72934d-205.pdf |
| Met Council Letter of Support | b0553570-10b.pdf |
| Minnesota Cities Stormwater Coalition Letter of Support | 69c682c6-fb2.pdf |
| Hennepin County Letter of Support | 306c750a-ab4.pdf |
| EQB Letter of Support | da7c900a-a25.pdf |
| Minneapolis Park and Recreation Board Letter of Support | 0d2f38c5-2d0.pdf |
| City of Edina Letter of Support | c31c2f7f-880.pdf |
| City of Minneapolis Letter of Support | b03378ab-d1d.pdf |
| City of Minnetonka Letter of Support | 08497ccb-20c.pdf |
| City of Mound Letter of Support | 667fb967-295.pdf |
| City of St. Louis Park Letter of Support | 35e72525-f56.pdf |
| City of Victoria Letter of Support | 8164d10e-d55.pdf |
| City of Wayzata Letter of Support | 10977cbe-a91.pdf |
| Background Check Certification Form | d5791bba-c6a.pdf |

Difference between Proposal and Work Plan

Describe changes from Proposal to Work Plan Stage

I updated the proposal based on the comments provided by LCCMR staff. I want to add a clarifying comment about the primary purpose of our proposal. The major advancement is developing a reproducible system that converts publically available data into a model-ready format to make watershed model development more efficient and higher resolution. All datasets for model development are currently available, however, they are all in unique formats, which makes model development a very labor-intensive process. However, the steps tend to be very repetitive and logical, which means that data conversion could be facilitated by developing computer scripts that automate the data conversion process. Furthermore, manual data incorporation results in lower resolution models, which is why most models at a watershed or regional scale are built at a low resolution which makes them difficult to use for planning purposes. Other agencies have already expressed interest and support for this system since they can use our process to build more efficiently

build models for their cities or regional agencies.

Please let me know if that doesn't come through in the work plan. The low word count limit makes it difficult to explain the purpose and the methods.

Additional Acknowledgements and Conditions:

The following are acknowledgements and conditions beyond those already included in the above workplan:

Do you understand and acknowledge the ENRTF repayment requirements if the use of capital equipment changes?

N/A

Do you agree travel expenses must follow the "Commissioner's Plan" promulgated by the Commissioner of Management of Budget or, for University of Minnesota projects, the University of Minnesota plan?

N/A

Does your project have potential for royalties, copyrights, patents, or sale of products and assets?

Yes

Do you understand and acknowledge IP and revenue-return and sharing requirements in 116P.10?

Yes

Do you wish to request reinvestment of any revenues into your project instead of returning revenue to the ENRTF?

No

Does your project include original, hypothesis-driven research?

No

Does the organization have a fiscal agent for this project?

No



ENRTF/OHF Pass-Through Grant Agreement Attachment C: Conflict of Interest Disclosure

Conflict of Interest:

As referenced in the Minnesota Department of Administrations Office of Grants Management's Policy 08-01, a conflict of interest, actual, potential, or perceived, occurs when a person has actual or apparent duty or loyalty to more than one organization and the competing duties or loyalties may result in actions which are adverse to one or both parties. A conflict of interest exists even if no unethical, improper or illegal act results from it.

Actual Conflict of Interest:

An actual conflict of interest occurs when a decision or action would compromise a duty to a party without taking immediate appropriate action to eliminate the conflict. Examples included but not limited to:

- One party uses his or her position to obtain special advantage, benefit, or access to the other party's time, services, facilities, equipment, supplies, badge, uniform, prestige, or influence.
- One party receives or accepts money (or anything else of value) from another party or has equity or a financial interest in or partial or whole ownership of the other party's organization.
- One party is an employee, board member or family member of the other party.

Potential Conflict of Interest:

A potential conflict of interest may exist if one party has a relationship, affiliation, or other interest that could create an inappropriate influence if the person is called on to make a decision or recommendation that would affect one or more of those relationships, affiliations, or interests. Examples included but not limited to:

- One party has a relationship, affiliation, or other interest that could create an inappropriate influence if one party is called on to make a decision or recommendation that would affect one or more of those relationships, affiliations, or interests. For example, when one party serves in a volunteer capacity for another party, it has the potential to, but does not necessarily create a conflict of interest, depending on the nature of the relationship between the two parties.

A disclosed potential conflict of interest warrants additional discussion in order to identify the nature of the relationship, affiliation, or other interest and take action to mitigate any potential conflicts.

Individual Conflict of Interest:

A conflict of interest that may benefit an individual employee is any situation in which their judgment, actions or non-action could be interpreted to be influenced by something that would benefit them directly or through indirect gain to an immediate family member, business, or organization with which they are involved.

A employee uses their status or position to obtain special advantage, benefit, or access to the grantee or grant applicant's time, services, facilities, equipment, supplies, badge, uniform, prestige, or influence

Organizational Conflict of Interest:

A conflict of interest can also occur with an organization that is a grant applicant or grantee of a state agency. Organizational conflicts of interest occur when:

- A grantee's objectivity in carrying out the grant is impaired or compromised due to competing duties or loyalties

- A grantee, potential grantee or grant applicant has an unfair competitive advantage through being furnished unauthorized proprietary information or source selection information that is not available to all competitors.

Particular attention should be paid to any proposed grant contract agreement requirements that provide for the rendering of planning, consultation, evaluation, or similar activities that may inform decisions on future grant awards.

This section to be completed by Grantee's Authorized Representative:

I certify that we will maintain an adequate Conflict of Interest Policy, and throughout the term of our agreement will report any actual, potential and perceived conflicts of interests by individual employees or are organization as a whole to the State's Authorized Representative.

Authorized Representative Signature: _____

DocuSigned by:

James Wisler

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November 15, 2023



Office of Management and Budget (OMBS)

Pass-Through Grants Reimbursement Manual

**Environment and Natural Resources Trust Fund (ENRTF)
Outdoor Heritage Fund (OHF)
Fiscal Year 2024 (July 1, 2023- June 30, 2024)**

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Changes In This Version

The Fiscal Year 2024 manual contains these changes from previous versions:

- Clarifies current monitoring requirements that any grantee with a grant over \$250,000 must complete an annual monitoring with their grant specialist before June 30 every year. If a grantee fails to complete their monitoring visit, they are out of compliance with the grant agreement, and further reimbursements requests will not be completed until after the monitoring requirements are completed.
- Clarifies that monitoring can be done by any agreed upon electronic meeting platform, phone, or in person.
- Clarifies the steps that the Grants Unit may take if it finds at monitoring that some costs that were reimbursed were not correctly incurred or documented.
- Adds that, if their address changes during the grant period, the grantee must update their address in the SWIFT portal before receiving additional reimbursements.
- Updates the links to the Dept. of Administration's Debarred Vendors list.

Introduction

The Grants Unit within the DNR Office of Management and Budget Services (OMBS) provides contract management services related to ENRTF and OHF pass-through grant projects. Contract management ensures oversight of reimbursement for project deliverables and meets the requirements of all state laws and policies including the Department of Administration's Office of Grants Management (OGM) procedures. Contract management helps recipients with financial compliance and ensures project consistency with appropriation law, state statute, grants policies, and approved work/accomplishment plans.

This manual was developed to help grantees administer their pass-through appropriation(s) and to provide instruction on how to obtain reimbursements for eligible project expenses. However, it will not be able to address all issues and potential problems that may arise during the completion of the project. **For questions regarding the grant agreement and amendments or reimbursement requests, please contact the State's Authorized Representative or your assigned Grants Specialist.** Contact information is at the end of this manual; it can also be found in the contract agreement and on the DNR pass-through grants [website](#).

For questions regarding your ENRTF work plan, please contact Legislative-Citizen Commission on Minnesota Resources ([LCCMR](#)) staff.

For questions regarding your OHF accomplishment plan, please contact Lessard-Sams Outdoor Heritage Council ([LSOHC](#)) staff.

Internal Controls

- The grantee is responsible for establishing and maintaining adequate financial internal control systems that follow generally accepted accounting and auditing principles.
- Grantees must establish a separate, non-interest bearing account for ENRTF/OHF funds.

- Any accounting issues not addressed in this manual are subject to state agency standards as interpreted by their internal auditors. All projects are subject to final audit.

Monitoring

- OGM [policy 08-10](#) requires one monitoring visit during the course of the grant period on projects valued at over \$50,000.
- Any grantee with a grant over \$250,000 must complete an annual monitoring with their DNR grant specialist before June 30th every year. If a grantee fails to complete their monitoring visit, they are out of compliance with their grant agreement, and further reimbursements requests will not be completed until after the monitoring requirements are completed. Grants from \$50,000-\$250,000 must do at least one monitoring during the grant period. If any costs are found that were reimbursed, but upon further review should not have been eligible during annual monitoring or other reviews during the grant period, repayment of those costs, or other corrective action, may be required.
- Monitoring can be done by any agreed upon electronic meeting platform, phone, or in person. . Grantees will be given adequate notice prior to monitoring.
- The focus of the visit will be financial controls, grants management, conflict of interest, and associated documentation and procedures.

State Accounting System (SWIFT) Requirements

The DNR processes project reimbursement payments through a system managed by Minnesota Management and Budget (MMB). The preferred method of payment is through the use of an electronic funds transfer (EFT) directly into the grantee's designated bank account. Electronic transfer reimbursements provide timely payments and prevent the loss of checks either in the mail or by misdirection. In order to set up the electronic transfer payment process, please contact MMB at 651-201-8106.

The grantee will also need to request a User ID to access the SWIFT e-Supplier portal to view payment information:

- Go to [Minnesota Supplier Portal](#)
 - Click on the 'Register for an Account' link.
 - Click the 'Register as a Supplier' button in the New Supplier section.
 - Complete the online form.
 - An email will be sent with the new User ID and password.
- Questions regarding this process can be sent to efthelpline.mmb@state.mn.us.
- If the address of the grantee changes during the grant period, the grantee must update their address in the SWIFT portal before receiving additional reimbursements.

Project Reimbursement

Pass-through grants are reimbursement based. The grantee must pay for project expenses prior to seeking reimbursement. Eligible expenses are then reimbursed under the terms of the agreement with the State of Minnesota.

Advance Payment

The DNR will only provide advance payment with prior approval as outlined in session law and the grantee's agreement.

- LCCMR and/or LSOHC will need to approve all advances for the project through the work/accomplishment plan.
- This does not apply to land acquisitions, where the Grants Unit can transfer funds to the grantee at least one business day prior to the closing date through an EFT.
- All pre-closing documentation must be submitted at least **fifteen business days** in advance of the closing date in order to allow the Grants Unit to review the paperwork, notify the grantee of any missing or incomplete land acquisition documentation, and process the request.

Reimbursement Timeframe

Grantees should expect to be reimbursed within 30 days of the DNR receiving a complete reimbursement request.

- If documentation to process the request is missing, or the request has discrepancies or incorrect information, the 30-day clock does not start until all necessary information has been submitted to the DNR.
- If work/accomplishment plan updates or progress reports are past due to either the LSOHC or the LCCMR, the DNR will withhold reimbursement payments for that project until the grantee is in compliance.

Frequency of Submission

Grantees must submit reimbursement requests at least annually while grant work is being done and expenses have been incurred. If the grantee has not yet incurred costs, no reimbursement request is required.

Final Reimbursement

- The final reimbursement will be paid out when the State determines that the Grantee has satisfactorily fulfilled all the terms of their grant agreement, unless otherwise excluded by the State in writing.
 - The State must complete a financial reconciliation on all grants over \$50,000 prior to approving the final reimbursement request. If a final reimbursement request is also the first request, this may delay payment.
 - The final report must be approved by the LCCMR or the LSOHC prior to payment of the final reimbursement request unless the grantee receives prior approval from the DNR in coordination with LCCMR/LSOHC staff to waive that requirement.

Reimbursement Payment Request Documents

The reimbursement payment request is comprised of four sections.

Project Reimbursement Payment Request Form

This form must be completed and signed by an individual who is authorized by the organization to submit payment requests. It is required for all payment requests, including land or conservation easement acquisitions.

Reimbursement Spreadsheet

The Reimbursement Spreadsheet provides information on the starting budget amounts, total reimbursements to date, current requested reimbursement amount, and the remaining balance of funds available.

- Each funding source has a different spreadsheet. Only use the ENRTF spreadsheet for LCCMR projects, and the OHF spreadsheet for all LSOHC projects.
- Line items (categories) on the spreadsheet must match the line items from the approved work /accomplishment plan budget.
- Only approved budget items (expenses) will be eligible for reimbursement.
- Please note the [guidance on allowable expenses documents](#) on the LCCMR's website and the [budget line item definitions](#) on the LSOHC's website.

Project Activity Summary Spreadsheet

The Project Activity Summary Spreadsheet provides a detailed summary of all expenses on the reimbursement payment request. The spreadsheet highlights the transaction date, description of the charges, the amount requested, and the approved budget categories for each expense.

Reimbursement Documentation

Each reimbursement payment request must include back-up documentation for all expenses. This documentation may include receipts, invoices, and time (payroll) records. The documentation should show that the expenses were allowable costs and happened within the time period of the payment request. Specific documentation is required for land acquisitions. Land acquisition reporting requirements are listed in the grant agreement, in Attachment E.

- All invoices must explicitly state the date(s) that the services were performed. The date must fall within the period of the reimbursement payment request.
- Documentation for salary expenses includes time or payroll records for the payment request period. All employees working on a project should track number of hours worked on the project.
 - Timesheet elements include the period worked (date range of work performed), name of the employee, rate of pay, hours worked, and benefit rate. The original time records must be available for review if requested.
 - All vacation (paid time off), sick, and holiday benefits are eligible for reimbursement on a proportional level. Please contact your assigned Grants Specialist for more information.
- The following information must be added to (or written on) the copies of receipts, invoices, time records or other documentation:

- Budget line item the expense is being posted to, such as personnel, equipment, travel, etc.
- Check number or payment number that was used to pay for the receipt, invoice, or payroll. This number should match up with payment documentation such as a bank statement or other proof of payment.
- If the documentation has non-project expenses on it, be sure to circle the expenses being posted to the project along with budget item.

Expedited Reimbursement Documentation

Grantees with a history of clean reimbursement requests and no findings on their most recent monitoring do not have to send in receipts for equipment, supplies, and travel with their payment requests. Grantees must continue to keep all receipts in your folders, for review at annual monitoring.

Grantees only send in invoices/receipts/other proof of expenses in these categories with payment requests:

- Contracts
- Land Acquisition
- Capital Equipment
- Expenses over the bidding threshold in all categories.

To access this policy, grantees must have:

- no findings in their most recent monitoring, and
- no questioned costs over their last four reimbursement requests, in a span at least six months.

Once grantees meet this standard, their grant specialist lets them know that they can stop sending in receipts. If a grantee has questioned costs or findings, the grant specialist may require full receipts on future payment requests.

Submission of Reimbursement Payment Requests

Please send one copy of the reimbursement request to your designated Grants Specialist, via e-mail. Starting in Fiscal Year 2021 we will only accept electronic submissions for all grant reimbursements. Where possible, please ensure electronic documentation is in an accessible format.

Documents to Be Kept on File

The grantee must maintain all project agreements, correspondence, and the records pertaining to project expenses requested for reimbursement in a separate project file. Project records are required for monitoring/audit purposes and must be readily available for review.

Retention Period

All records related to the project must be retained for a minimum of six (6) years from the grant agreement end date, or the receipt and approval of all final reports, whichever is later. For OHF grants that have restoration and enhancement end dates, grantees may contact their grant specialist to discuss the possibility of a close-out amendment to set the end date for purposes of retention.

Proof of Payment

The State requires proof of payment documentation to ensure that funds are being provided on a reimbursement basis.

The grantee must maintain proof of payment documentation and make it available when requested by the State. Proof of payment documentation may include:

- a copy of a bank statement with small photocopies of cleared checks
- an electronic bank statement
- a copy of cancelled check(s), or other certified financial records
- employee original time records and payroll documentation.

The State may conduct a proof of payment review during grant monitoring or for any reimbursement request submitted by the grantee throughout the grant period if necessary. No additional reimbursement requests for that project will be processed until the proof of payment for the request being reviewed is submitted and approved.

Background Checks

All ENRTF grantees that conduct a project that provides children's services, as defined in Minnesota Statutes Sec 299C.61 Subd.7, must perform criminal background checks for background check crimes, as defined in Minnesota Statutes, section 299C.61, Subd. 2, on all employees, contractors, and volunteers who have or may have access to a child to whom children's services are provided. Grantees should keep this documentation in the project file, for review at annual monitoring.

Vendors and Subcontractors

Subcontractors include other organizations and/or businesses that perform services identified in the work/accomplishment plan. Vendors provide supplies or materials to the project. Both must be selected based on contracting/purchasing procedures. Transparency, fiscal control, and accountability are key reasons why the State requires grantees to be thorough in the solicitation and selection of subcontractors and vendors.

Each executed subcontract must include the amount of the subcontract, the length of the subcontract, and all elements of the grantee's contract with the State.

Contracting and Bidding for Municipalities

Municipalities as defined in Minnesota statute 471.345, subdivision 1 must follow the [Uniform Municipal Contracting Law](#).

Contracting and Bidding for Non-Governmental Organizations

Non-governmental organizations may either:

- submit a copy of their contracting policies for review to the State's Authorized Representative, through the use of Attachment B (Non-Governmental Organization Subcontracting) to their grant agreement;
- or follow the contracting policies/procedures below.

Contracting and Bidding Thresholds and Process

- Any services and/or materials that are expected to cost \$100,000 or more must undergo a formal notice and bidding process.
- Services and/or materials that are expected to cost between \$25,000 and \$99,999 must be competitively awarded based on a minimum of three (3) verbal quotes or bids.
- Services and/or materials that are expected to cost between \$10,000 and \$24,999 must be competitively awarded based on a minimum of two (2) verbal quotes or bids or awarded to a targeted vendor.
- Grantees must use a Request for Proposal (RFP)/Request for Quote (RFQ) process to competitively select professional and technical services.
- The advertisement for bid processes must allow for fair competition among potential qualified bidders.

Debarment

Grantee must not contract with vendors/subcontractors who are on the on the [State Department of Administration's Debarred Vendors list](#):

Targeted Vendors

Grantees must take all necessary affirmative steps to assure that targeted vendors from businesses with active certifications through the entities below when possible. Please contact your grant specialist for assistance the first time you go through this process:

- [State Department of Administration's Certified Targeted Group, Economically Disadvantaged and Veteran-Owned Vendor List](#)
- Metropolitan Council's Targeted Vendor list: [Minnesota Unified Certification Program](#)
- Small Business Certification Program through Hennepin County, Ramsey County, and City of St. Paul: [Central Certification Program](#)

Conflict of Interest

The grantee must maintain written standards of conduct covering conflicts of interest and governing the actions of its employees or board members engaged in the selection, award and administration of contracts. This written standard may be requested during monitoring.

Single/Sole Source

The State may waive bidding process requirements when it is determined there is only one legitimate or practical source for such materials or services and that grantee has established a fair and reasonable price. Single and sole source grants or contracts are used when only one entity is reasonably able to meet a grant's intended purpose and objectives, due to their geographic location, specialized knowledge, relationships or specialized equipment.

To seek a single/sole source waiver, the grantee must complete a [Grant Single Source Justification Form](#), available on the pass-through grants website, and submit it to the State's Authorized Representative for signature and approval. If approved, the Grantee must keep the executed copy on file.

Supporting Documentation

The grantee must maintain support documentation of the purchasing and/or bidding process utilized to contract services in their financial records, including support documentation justifying a single/sole source bid, if applicable.

Grantees must retain, in the project file:

- copies of the executed subcontract agreements
- a copy of the bid tabulation (if applicable),
- written documentation that describes the rationale for selection of the subcontractor, and
- documentation of the contract/bid approval if required by grantee internal controls (such as meeting minutes.)

This documentation may be reviewed during the monitoring visit or when requested by the State.

Prevailing Wage

For projects that include construction work of \$25,000 or more, prevailing wage rules apply per [Minnesota Statute 177.41](#) through [177.44](#). Consequently, the bid request must state the project is subject to prevailing wage. These rules require that the wages of laborers and workers should be comparable to wages paid for similar work in the community as a whole. A prevailing wage form should accompany these bid submittals.

Grantees must retain documentation in the project file either of the prevailing wage forms, or a notice from the Department of Labor and Industry that the project is not subject to prevailing wage. Other prevailing wage information can be found at the [Minnesota Department of Labor and Industry](#).

Cost Category-Specific Requirements

Equipment

Records for grantee-owned equipment used on a project must include the time actually used for the project and the computation used to arrive at the charged use rate. Use rates are subject to review by DNR.

Capital Equipment

Capital equipment purchased with grant funds must be:

- listed in the approved work/accomplishment plan prior to purchase
- tagged, maintained in an up-to-date directory, and available for review.

Land Acquisitions

- Any project that is subject to the land acquisition reporting requirements of Attachment E (Land Acquisition and Reporting Procedures) will be held to the most current version of Attachment E.
- The Grants Unit can transfer funds to the grantee one to two business day prior to the closing date through an EFT.

- All pre-closing documentation must be submitted at least ten business days in advance of the closing date in order to allow the Grants Unit to review the paperwork, notify the grantee of any missing or incomplete land acquisition documentation, and process the request.

Fourth Year Land Acquisition

In order to be reimbursed for land acquisitions that occur in the fourth fiscal year, the grantee will notify the State that a binding purchase agreement has occurred before the end of the third fiscal year.

- Notify your grant specialist that you intend to have a binding agreement in place by June 30 of the third year.
- Provide proof of that agreement no later than July 15 of the fourth year.
- Failure to do so may significantly affect payment. In that case, notify your grant specialist as soon as possible of the binding agreement.

Use of Funds

For all acquisitions that require DNR services, the grantee must submit a Use of Funds letter, as outlined in their grant agreement, to pay for the DNR's services. Funding must be available in the "DNR Land Acquisition Costs" (OHF) or the "Other DNR acquisition, reporting, and management" (ENRTF) budget line item in the approved work/accomplishment plan to pay for these costs.

Please see Attachment E of the grant agreement for step-by-step land acquisition procedures and requirements.

Materials and Services

Materials and services are eligible expenses when they are purchased by the grantee to achieve outcomes/activities stated in the work/accomplishment plan and reflected in the approved budget. Typical examples of material/service purchases include hardware, paint, lumber, sand/gravel, concrete, landscape materials, and signs.

In order to request reimbursement for materials and services, the grantee must have an invoice from the vendor. The invoice and the copy sent in with the reimbursement payment request must be legible and include the following items:

- Name and address of the vendor;
- Date the item or service was purchased;
- Date the service was performed;
- Quantity of item(s) purchased or hours worked;
- Description of item(s) or services purchased;
- Unit price/Prorate;
- Total amount of the line item.
- Please also add the following information to the invoices:
 - The activity number that the expense is being posted to.

- If a portion of an expense is being posted to more than one activity or budget line items, please include that information on the invoice (ENRTF only).
- The budget line item (or category) the expense is being posted to. Examples include expenses identified as “travel”, “personnel”, “equipment”, etc.

Travel

Travel must be included in the approved work/accomplishment plan and budget in order to be eligible for reimbursement. Out of state travel is an ineligible expense for both ENRTF and OHF projects unless explicitly approved in the work/accomplishment plan.

Travel expenses must follow Commissioner’s Plan guidelines in order to be eligible for reimbursement. The [Commissioner’s Plan](#) includes mileage and meal reimbursement rates and guidelines in Chapter 15 – Expense Reimbursement. Information on travel expenses can also be found on the [Travel Reimbursement and Documentation Guide](#) available on the DNR Pass-Through grant website.

Contact Information

Minnesota Department of Natural Resources
Office of Management and Budget Services, Grants Unit
500 Lafayette Road St. Paul, MN 55155-4010
[Pass-Through Grants Website](#)

Katherine Sherman-Hoehn, Grants Manager (State Authorized Representative)
Phone: (651- 259-5533
[E-mail: katherine.sherman-hoehn@state.mn.us](mailto:katherine.sherman-hoehn@state.mn.us)

Karen Mueller, Grants Specialist Senior
Phone: 651- 259-5559
[E-mail: karen.cibuzar-mueller@state.mn.us](mailto:karen.cibuzar-mueller@state.mn.us)

Mandy Skypala, Grants Specialist Coordinator
Phone: 651-259-5543
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Jason Tidemann, Grants Specialist Coordinator
Phone: 651- 259-5534
[E-mail: jason.tidemann@state.mn.us](mailto:jason.tidemann@state.mn.us)

Monica Weber, Grants Specialist Sr.
Phone: (651) 259-5370
[email: monica.weber@state.mn.us](mailto:monica.weber@state.mn.us)

APPENDIX

Reimbursement Request Checklist

The checklist contains the items that must be included with the reimbursement request. Please use the checklist to ensure that the payment request is complete.

For all projects, the Grantee must submit the following:

1. **___ Section 1: Project Reimbursement Payment Request Form**
This document must be dated and signed by an appropriate representative for the grantee. Please complete the form and include the name of the project, the SWIFT purchase order number (300000XXXX), the sequence of the request (for example, the first request would be #1), and the period of time the request covers.
2. **___ Section 2: Reimbursement Spreadsheet**
The Reimbursement Spreadsheet will need to be customized to include the budget items and outcomes/activities from Attachment A, the approved work/accomplishment plan. This will help track budget line items to ensure funding is being expended by budget categories.
3. **___ Section 3: Project Activity Summary Spreadsheet**
The Project Activity Summary Spreadsheet should include the date range of reimbursable activity, the transaction date, the approved budget category for each charge and the amount requested, along with a brief description of the reimbursable items.
4. **___ Section 4: Reimbursement Documentation**
Submit copies of receipts, invoices, and time records (payroll). This information is necessary to determine if the expenses are eligible for reimbursement. Please write the activity number, budget item title and payment information (such as check number, bank statement) on the receipt/invoice document submitted. This information is needed to determine what part of the project the expenses are being directed to.
5. **___ Section 5: Acquisition Documents (if applicable)**
Specific documentation is required for land acquisitions. Please see your grant agreement to view your land acquisition reporting requirements.

Exhibit F: Contract Template

**AGREEMENT BETWEEN
MINNEHAHA CREEK WATERSHED DISTRICT and
[CONSULTANT]**

2-D Watershed-Wide Model

This agreement is entered into by the Minnehaha Creek Watershed District, a public body with powers set forth at Minnesota Statutes chapters 103B and 103D (MCWD), and [CONSULTANT], a Minnesota corporation (CONSULTANT). In consideration of the terms and conditions set forth herein and the mutual exchange of consideration, the sufficiency of which hereby is acknowledged, MCWD and CONSULTANT agree as follows:

1. Scope of Work

CONSULTANT will perform the work described in the [DATE] Scope of Services attached as Exhibit A (the Services). Exhibit A is incorporated into this agreement and its terms and schedules are binding on CONSULTANT as a term hereof. MCWD, at its discretion, in writing may at any time suspend work or amend the Services to delete any task or portion thereof. Authorized work by CONSULTANT on a task deleted or modified by MCWD will be compensated in accordance with paragraphs 5 and 6. Time is of the essence in the performance of the Services.

The work is supported by a grant of the Legislative-Citizen Commission on Minnesota Resources under the Environment and Natural Resources Trust Fund. The grant agreement and attachments (together, "Grant Agreement") are incorporated into this agreement. CONSULTANT will cooperate with MCWD as necessary for MCWD to fulfill the terms of the grant agreement.

2. Independent Contractor

CONSULTANT is an independent contractor under this agreement. CONSULTANT will select the means, method and manner of performing the Services. Nothing herein contained is intended or is to be construed to constitute CONSULTANT as the agent, representative or employee of MCWD in any manner. Personnel performing the Services on behalf of CONSULTANT will not be considered employees of MCWD and will not be entitled to any compensation, rights or benefits of any kind from MCWD.

3. Subcontract and Assignment

CONSULTANT will not assign, subcontract or transfer any obligation or interest in this agreement or any of the Services without the written consent of MCWD and pursuant to any conditions included in that consent. MCWD consent to any subcontracting does not relieve CONSULTANT of its responsibility to perform the Services or any part thereof, nor in any respect its duty of care, insurance obligations, or duty to hold harmless, defend and indemnify under this agreement.

4. Duty of Care; Indemnification

CONSULTANT will perform the Services with due care and in accordance with national standards of professional care. CONSULTANT will hold harmless and indemnify MCWD, its board members, employees and agents, from any and all actions, costs (including reasonable attorney fees), damages and liabilities of any nature arising from CONSULTANT's lack of professional due care,

and will defend, hold harmless, and indemnify MCWD, its board members, employees and agents from any and all actions, costs, damages and liabilities of any nature arising from CONSULTANT's negligent or otherwise wrongful act or omission, or breach of a specific contractual duty other than the duty of professional due care. For any claim subject to this paragraph by an employee of CONSULTANT, the indemnification obligation is not limited by a limitation on the amount or type of damages, compensation or benefits payable by or for CONSULTANT under workers' compensation acts, disability acts or other employee benefit acts.

5. Compensation

MCWD will compensate CONSULTANT for the Services on an hourly basis and reimburse for direct costs in accordance with Exhibit A. Invoices will be submitted monthly for work performed during the preceding month, and must conform to the requirements of the Grants Reimbursement Manual that are a part of the Grant Agreement. Payment for undisputed work will be due within 30 days of receipt of invoice. Direct costs not specified in Exhibit A will not be reimbursed except with prior written approval of the MCWD administrator.

The total payment for each task will not exceed the amount specified for that task in Exhibit A. The total payment for the Services will not exceed [\$ _____]. Total payment in each respect means all sums to be paid whatsoever, including but not limited to fees and reimbursement of direct costs, whether specified in this agreement or subsequently authorized by the administrator.

CONSULTANT will maintain all records pertaining to fees or costs incurred in connection with the Services for six years from the date of completion of the Services. CONSULTANT agrees that any authorized MCWD representative, the state auditor or the grant auditor may have access to and the right to examine, audit and copy any such records during normal business hours.

6. Termination; Continuation of Obligations

This agreement is effective when fully executed by the parties and will remain in force until [DATE] unless earlier terminated as set forth herein.

MCWD may terminate this agreement at its convenience, by a written termination notice stating specifically what prior authorized or additional tasks or services it requires CONSULTANT to complete. CONSULTANT will receive full compensation for all authorized work performed, except that CONSULTANT will not be compensated for any part performance of a specified task or service if termination is due to CONSULTANT's breach of this agreement.

Insurance obligations; duty of care; obligations to defend, indemnify and hold harmless; duty to cooperate in assignment of intellectual property; and document-retention requirements will survive the completion of the Services and the term of this agreement.

7. No Waiver

The failure of either party to insist on the strict performance by the other party of any provision or obligation under this agreement, or to exercise any option, remedy or right herein, will not waive or relinquish such party's rights in the future to insist on strict performance of any provision, condition or obligation, all of which will remain in full force and affect. The waiver of either party on one or more occasion of any provision or obligation of this agreement will not be construed as

a waiver of any subsequent breach of the same provision or obligation, and the consent or approval by either party to or of any act by the other requiring consent or approval will not render unnecessary such party's consent or approval to any subsequent similar act by the other.

Notwithstanding any other term of this agreement, MCWD waives no immunity in tort. This agreement creates no right in and waives no immunity, defense or liability limit with respect to any third party.

8. Insurance

At all times during the term of this Agreement, CONSULTANT will have and keep in force the following insurance coverages:

- A. General: \$1.5 million, each occurrence and aggregate, covering CONSULTANT's ongoing operations on an occurrence basis.
- B. Professional liability: \$1.5 million each claim and aggregate. Any deductible will be CONSULTANT's sole responsibility and may not exceed \$50,000. Coverage may be on a claims-made basis, in which case CONSULTANT must maintain the policy for, or obtain extended reporting period coverage extending, at least three (3) years from completion of the Services.
- C. Automobile liability: \$1.5 million combined single limit each occurrence coverage for bodily injury and property damage covering all vehicles on an occurrence basis.
- D. Workers' compensation: in accordance with legal requirements applicable to CONSULTANT.

CONSULTANT will not commence work until it has filed with MCWD a certificate of insurance clearly evidencing the required coverages and naming MCWD as an additional insured for general liability, along with a copy of the additional insured endorsement establishing coverage for CONSULTANT's ongoing operations as primary coverage on a noncontributory basis. The certificate will name MCWD as a holder and will state that MCWD will receive written notice before cancellation, nonrenewal or a change in the limit of any described policy under the same terms as CONSULTANT.

9. Compliance With Laws

CONSULTANT will comply with the laws and requirements of all federal, state, local and other governmental units in connection with performing the Services and will procure all licenses, permits and other rights necessary to perform the Services.

In performing the Services, CONSULTANT will ensure that no person is excluded from full employment rights or participation in or the benefits of any program, service or activity on the ground of race, color, creed, religion, age, sex, disability, marital status, sexual orientation, public assistance status or national origin; and no person who is protected by applicable federal or state laws, rules or regulations against discrimination otherwise will be subjected to discrimination.

Specifically but not exclusively, CONSULTANT will conform to the requirements of the Grant Agreement, §21 (“Non-Discrimination Requirements”), as applicable.

10. Data and Information

As between MCWD and CONSULTANT, all data and information obtained or generated by CONSULTANT in performing the Services, including documents in hard and electronic copy, software, and all other forms in which the data and information are contained, documented or memorialized (together, here and in sections 11 and 12, the “Materials”), are the property of MCWD. CONSULTANT hereby assigns and transfers to MCWD all right, title and interest in: (a) its copyright, if any, in the Materials; any registrations and copyright applications relating to the Materials; and any copyright renewals and extensions; (b) all works based on, derived from or incorporating the Materials; and (c) all income, royalties, damages, claims and payments now or hereafter due or payable with respect thereto, and all causes of action in law or equity for past, present or future infringement based on the copyrights. CONSULTANT agrees to execute all papers and to perform such other proper acts as MCWD may deem necessary to secure for MCWD or its assignee the rights herein assigned. [Exception for instruments of service and third-party licensed data, if any.] Intellectual property rights of the parties under this agreement further are subject to the rights of the State as set forth in the Grant Agreement.

MCWD may immediately inspect, copy or take possession of any Materials on written request to CONSULTANT. On termination of the agreement, CONSULTANT may maintain a copy of some or all of the Materials except for any Materials designated by MCWD as confidential or non-public under applicable law, a copy of which may be maintained by CONSULTANT only pursuant to written agreement with MCWD specifying terms.

11. Data Practices; Confidentiality

If CONSULTANT receives a request for data pursuant to the Data Practices Act, Minnesota Statutes chapter 13 (DPA), that may encompass data (as that term is defined in the DPA) CONSULTANT possesses or has created as a result of this agreement, it will inform MCWD immediately and transmit a copy of the request. If the request is addressed to MCWD, CONSULTANT will not provide any information or documents, but will direct the inquiry to MCWD. If the request is addressed to CONSULTANT, CONSULTANT will be responsible to determine whether it is legally required to respond to the request and otherwise what its legal obligations are but, before replying, will: (a) notify and consult with MCWD and its legal counsel; and (b) under the terms of the Grant Agreement, await any instructions from the State. Nothing in the preceding sentence supersedes CONSULTANT’s obligations under this agreement with respect to protection of MCWD data, property rights in data or confidentiality. Nothing in this section constitutes a determination that CONSULTANT is performing a governmental function within the meaning of Minnesota Statutes section 13.05, subdivision 11, or otherwise expands the applicability of the DPA beyond its scope under governing law.

CONSULTANT agrees that it will not disclose and will hold in confidence any and all proprietary Materials owned or possessed by MCWD and so denominated by MCWD. CONSULTANT will not use any such Materials for any purpose other than performance of the Services without MCWD written consent. This restriction does not apply to Materials already possessed by CONSULTANT or that CONSULTANT received on a non-confidential basis from MCWD or another party.

Consistent with the terms of sections 10 through 12 regarding use and protection of confidential and proprietary information, and regarding the intellectual property rights of the State, CONSULTANT retains a nonexclusive license to use the Materials and may publish or use the Materials in its professional activities. Any CONSULTANT duty of care under this agreement does not extend to any party other than MCWD or to any use of the Materials by MCWD other than for the purpose(s) for which CONSULTANT is compensated under this agreement. In any publication or public communication pertaining to its work under this agreement, CONSULTANT will acknowledge the financial support of the Minnesota Environment and Natural Resources Trust Fund as required by the Grant Agreement, §15.

12. MCWD Property

All property furnished to or for the use of CONSULTANT or a subcontractor by MCWD and not fully used in the performance of the Services, including but not limited to equipment, supplies, and Materials, will remain the property of MCWD and returned to MCWD at the conclusion of the performance of the Services, or sooner if requested by MCWD. CONSULTANT further agrees that any proprietary Materials are the exclusive property of MCWD and will assert no right, title or interest in the Materials. CONSULTANT will not disseminate, transfer or dispose of any proprietary Materials to any other person or entity unless specifically authorized in writing by MCWD.

Any property including but not limited to Materials supplied to CONSULTANT by MCWD or deriving from MCWD is supplied to and accepted by CONSULTANT as without representation or warranty including but not limited to a warranty of fitness, merchantability, accuracy or completeness. However, CONSULTANT's duty of professional care under paragraph 4, above, does not extend to Materials provided to CONSULTANT by MCWD or any portion of the Services that is inaccurate or incomplete as the result of CONSULTANT's reasonable reliance on those Materials.

13. Notices

Any written communication required under this agreement to be provided in writing will be directed to the other party as follows:

To MCWD:

Administrator
Minnehaha Creek Watershed District
15320 Minnetonka Boulevard
Minnetonka, MN 55345

To CONSULTANT:

[Authorized Representative
Organization
Address]

Either of the above individuals may in writing designate another individual to receive communications under this agreement.

14. Choice of Law; Venue

This agreement will be construed under and governed by the laws of the State of Minnesota. Venue for any action will lie in Hennepin County, except that venue in Ramsey County is permitted where the action properly is consolidated with an adjudication under the Grant Agreement.

15. Whole Agreement

The entire agreement between the two parties is contained herein and this agreement supersedes all oral agreements and negotiations relating to the subject matter hereof. Any modification of this agreement is valid only when reduced to writing as an amendment to the agreement and signed by the parties hereto. MCWD may amend this agreement only by action of the Board of Managers acting as a body.

IN WITNESS WHEREOF, intending to be legally bound, the parties hereto execute and deliver this agreement.

CONSULTANT

By _____ Date: _____
Its _____

Approved as to Form and Execution

MCWD Attorney

MINNEHAHA CREEK WATERSHED DISTRICT

By _____ Date: _____
Its _____

Exhibit A
Scope of Services

TEMPLATE